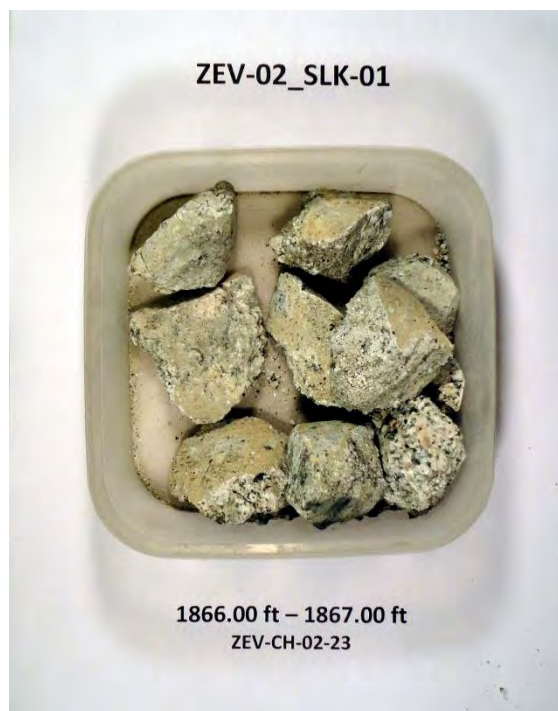


DOCKETED	
Docket Number:	21-AFC-02
Project Title:	Willow Rock Energy Storage Center
TN #:	254799
Document Title:	Willow Rock Energy Storage Center Supplemental AFC Volume II - Appendix 54A - Part VI
Description:	N/A
Filer:	Amanda Cooley
Organization:	Ellison Schneider Harris & Donlan LLP
Submitter Role:	Applicant Representative
Submission Date:	3/1/2024 4:46:44 PM
Docketed Date:	3/1/2024

APPENDIX J

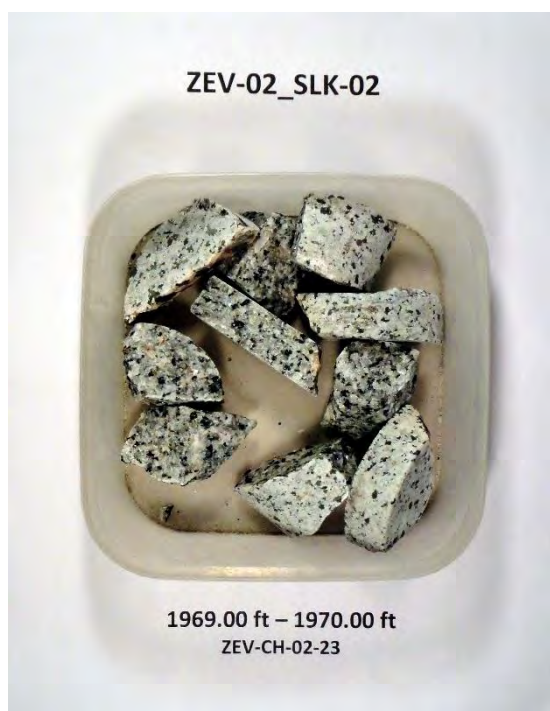
BEFORE AND AFTER PHOTOGRAPHS OF SLAKE DURABILITY TEST SPECIMENS



ZEV-02_SLK-01—Before



ZEV-02_SLK-01—After

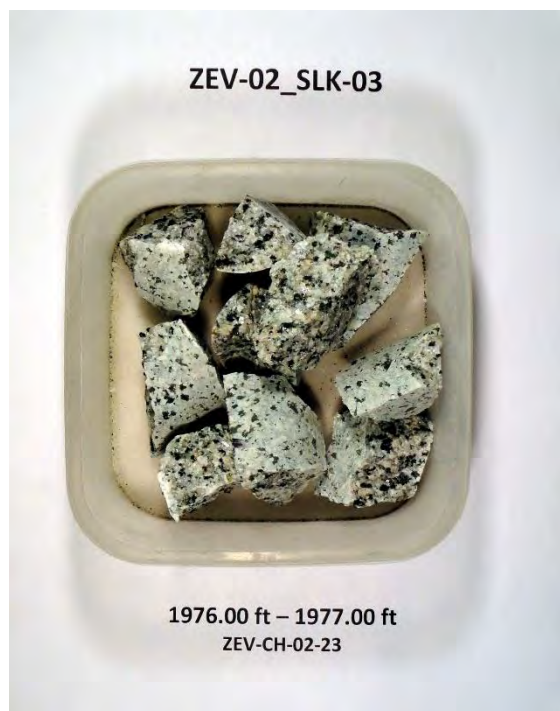


ZEV-02_SLK-02—Before



ZEV-02_SLK-02—After

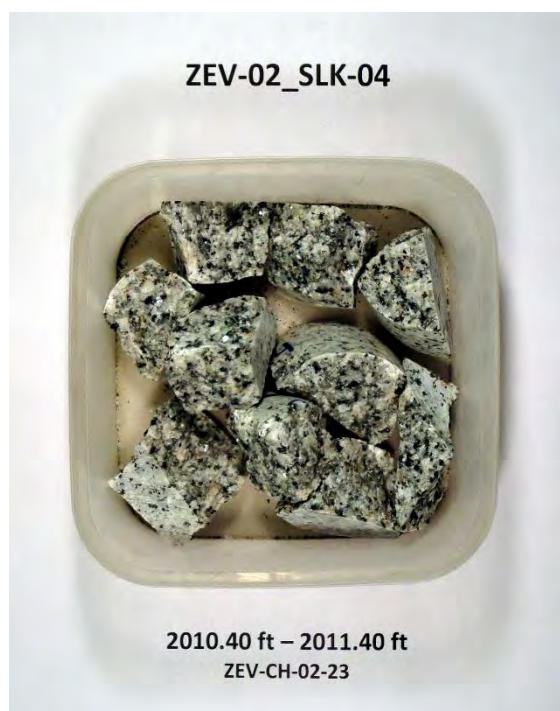
Figure J-1. Before and After Photographs of Slake Durability Test Specimens



ZEV-02_SLK-03—Before



ZEV-02_SLK-03—After

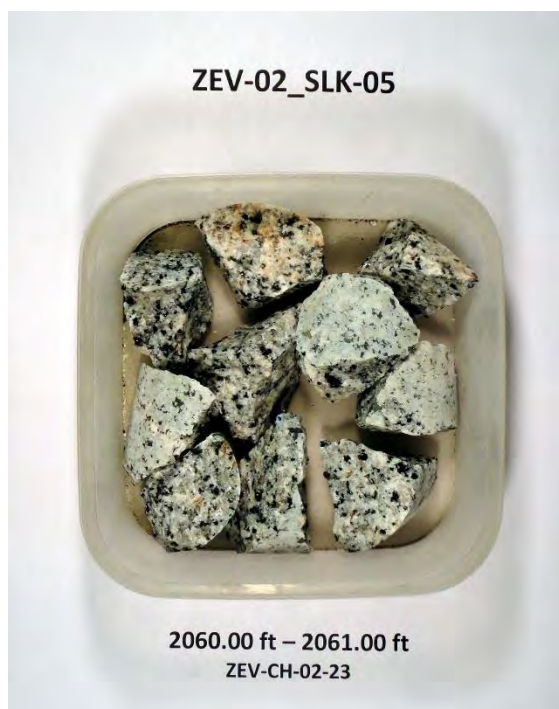


ZEV-02_SLK-04—Before



ZEV-02_SLK-04—After

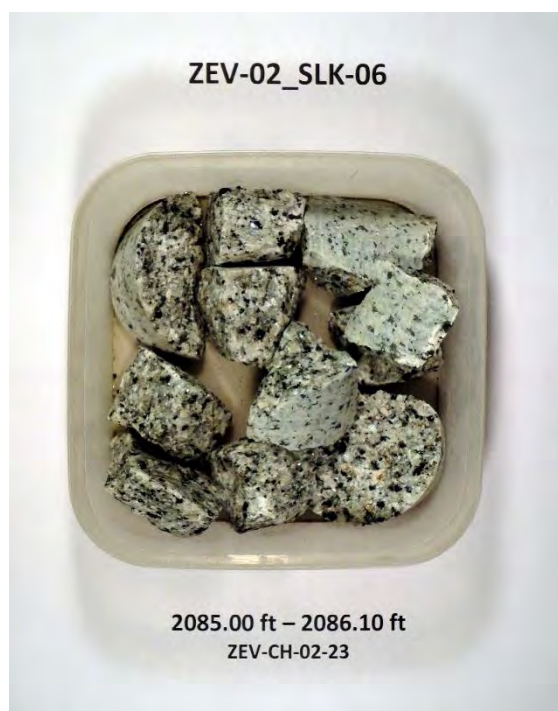
Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



ZEV-02_SLK-05—Before



ZEV-02_SLK-05—After



ZEV-02_SLK-06—Before



ZEV-02_SLK-06—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



ZEV-02_SLK-07—Before



ZEV-02_SLK-07—After

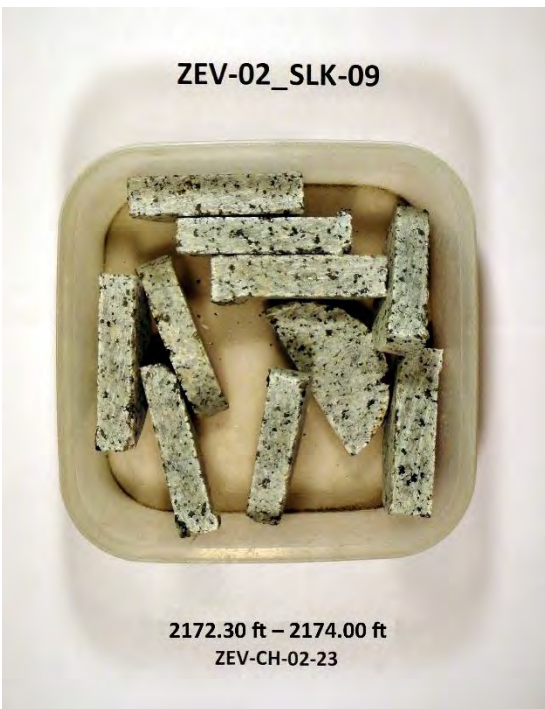


ZEV-02_SLK-08—Before

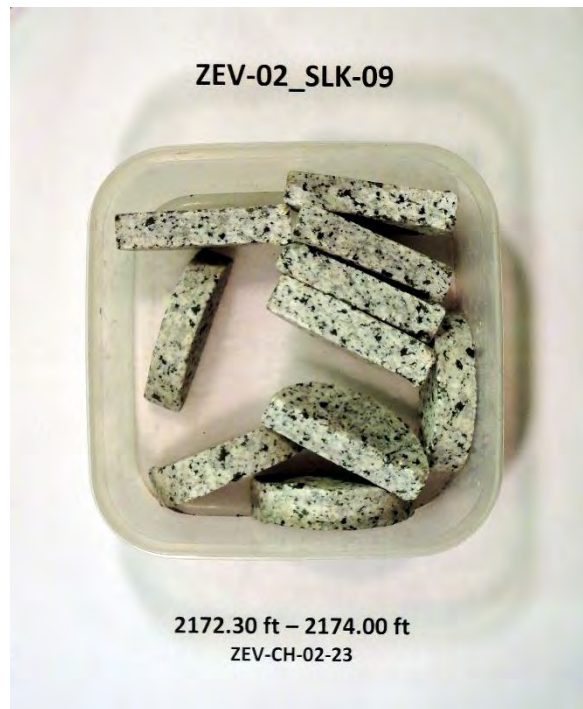


ZEV-02_SLK-08—After

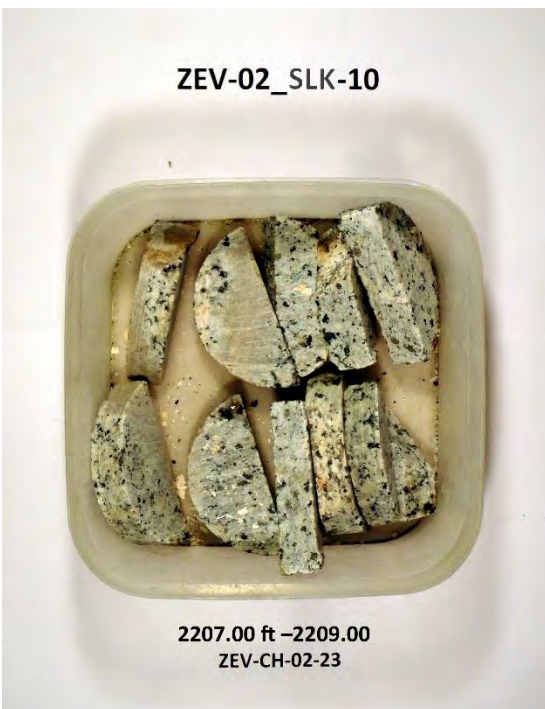
Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



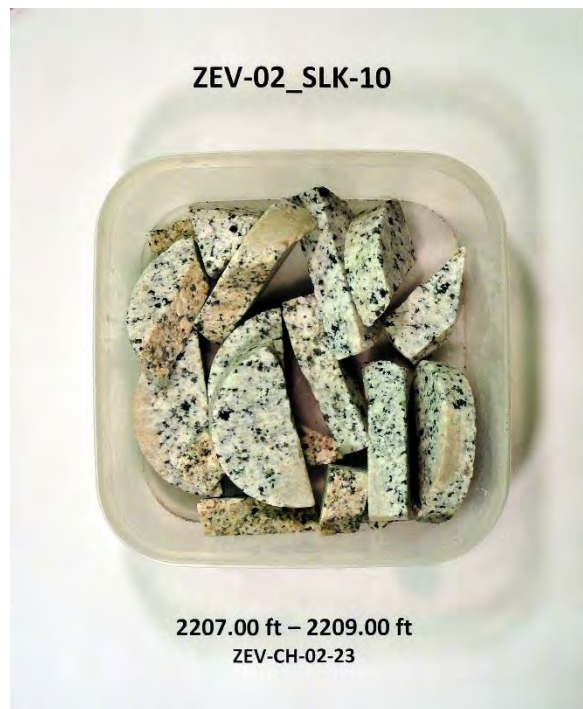
ZEV-02_SLK-09—Before



ZEV-02_SLK-09—After



ZEV-02_SLK-10—Before



ZEV-02_SLK-10—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



ZEV-02_SLK-11—Before



ZEV-02_SLK-11—After



ZEV-02_SLK-12—Before



ZEV-02_SLK-12—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



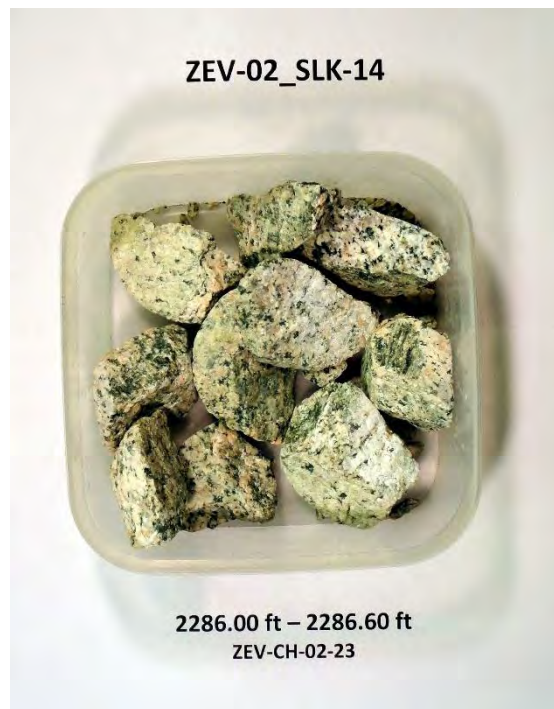
ZEV-02_SLK-13—Before



ZEV-02_SLK-13—After



ZEV-02_SLK-14—Before



ZEV-02_SLK-14—After

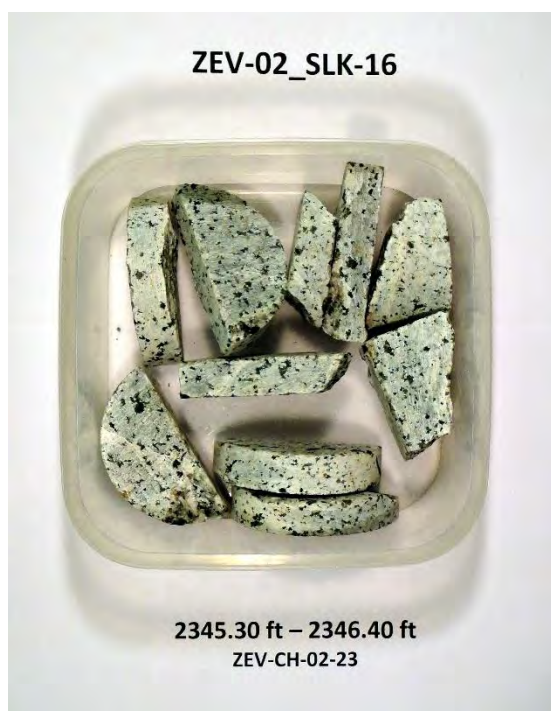
Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



ZEV-02_SLK-15—Before



ZEV-02_SLK-15—After



ZEV-02_SLK-16—Before



ZEV-02_SLK-16—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



ZEV-02_SLK-17—Before



ZEV-02_SLK-17—After



ZEV-02_SLK-18—Before



ZEV-02_SLK-18—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



ZEV-02_SLK-19—Before



ZEV-02_SLK-19—After



ZEV-02_SLK-20—Before



ZEV-02_SLK-20—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



ZEV-02_SLK-21—Before



ZEV-02_SLK-21—After



ZEV-02_SLK-22—Before

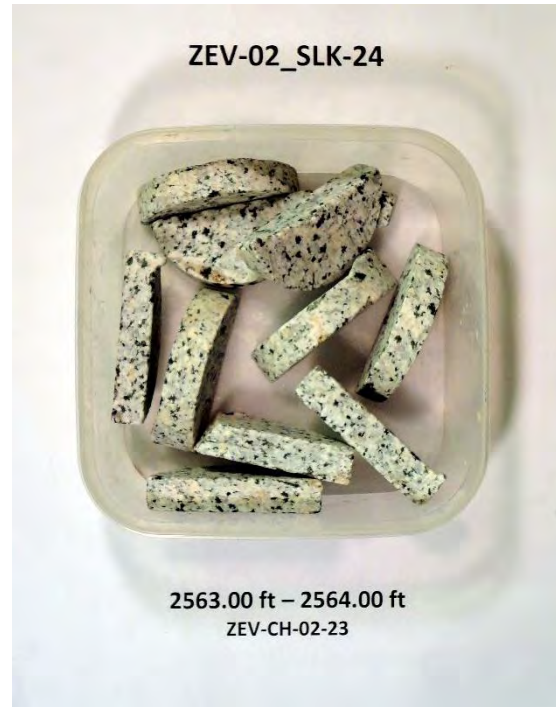


ZEV-02_SLK-22—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



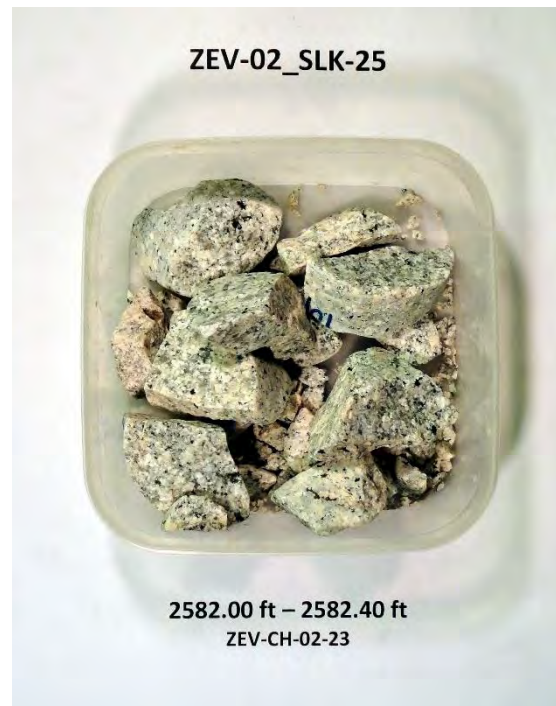
ZEV-02_SLK-24—Before



ZEV-02_SLK-24—After



ZEV-02_SLK-25—Before

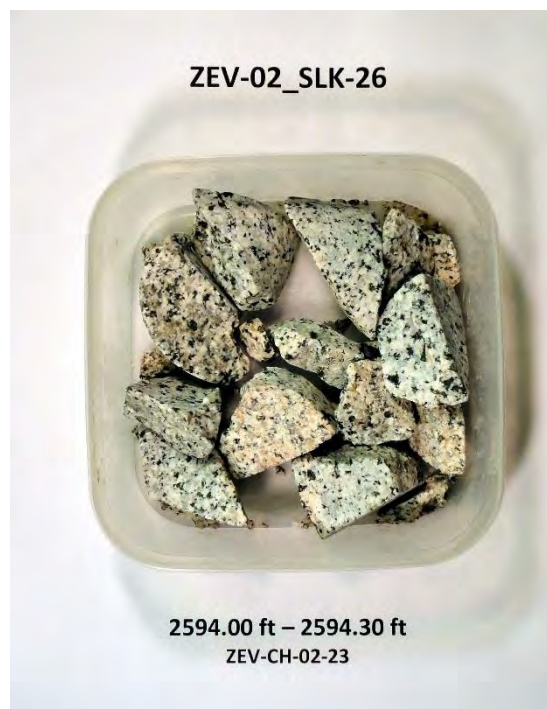


ZEV-02_SLK-25—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



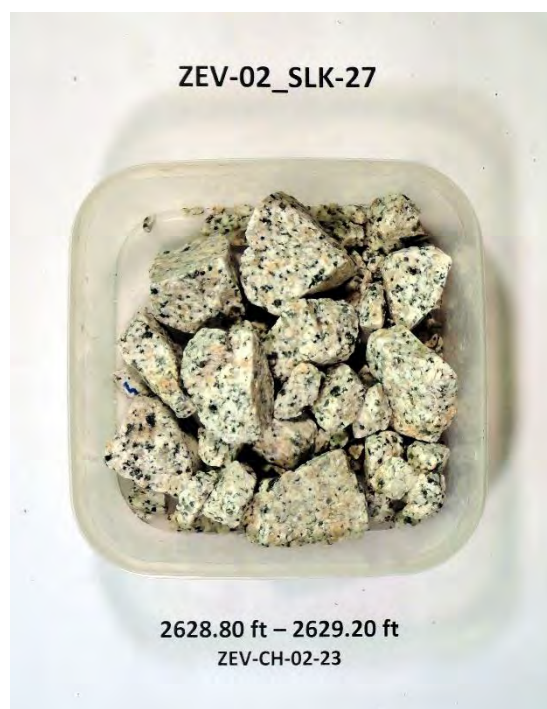
ZEV-02_SLK-26—Before



ZEV-02_SLK-26—After



ZEV-02_SLK-27—Before



ZEV-02_SLK-27—After

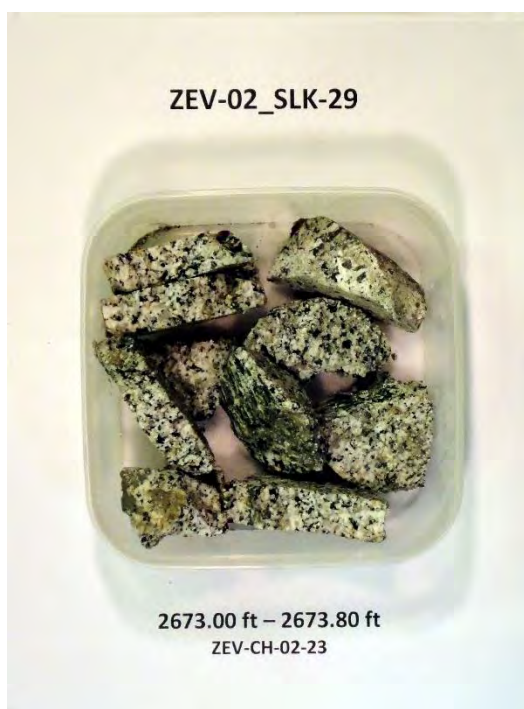
Figure J-1. Before and After Photographs of Slake Durability Test Specimens (continued)



ZEV-02_SLK-28—Before



ZEV-02_SLK-28—After

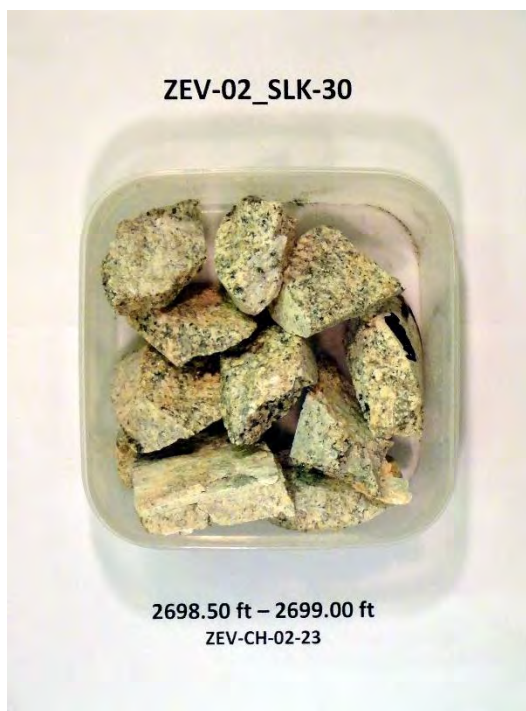


ZEV-02_SLK-29—Before

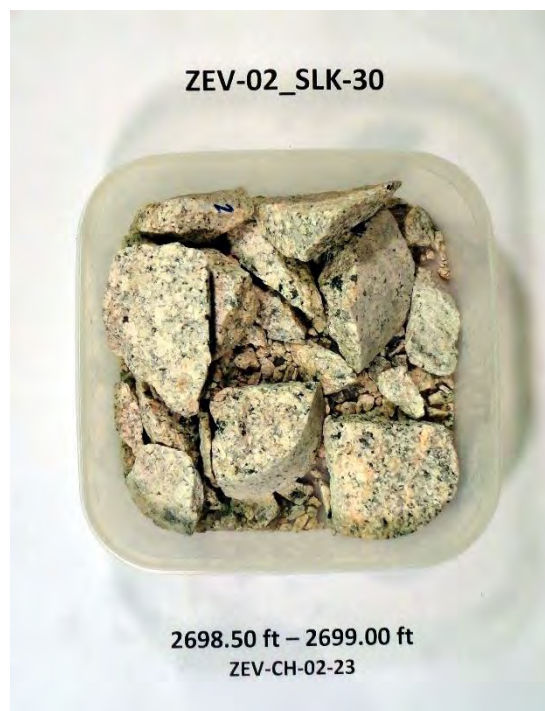


ZEV-02_SLK-29—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



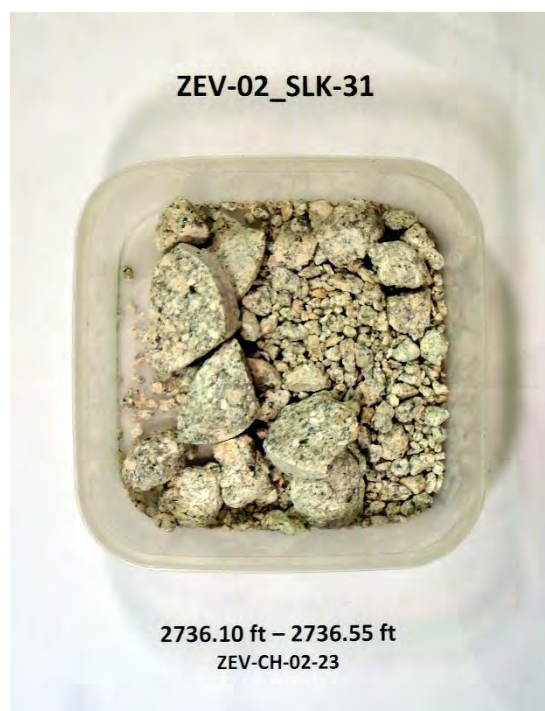
ZEV-02_SLK-30—Before



ZEV-02_SLK-30—After



ZEV-02_SLK-31—Before

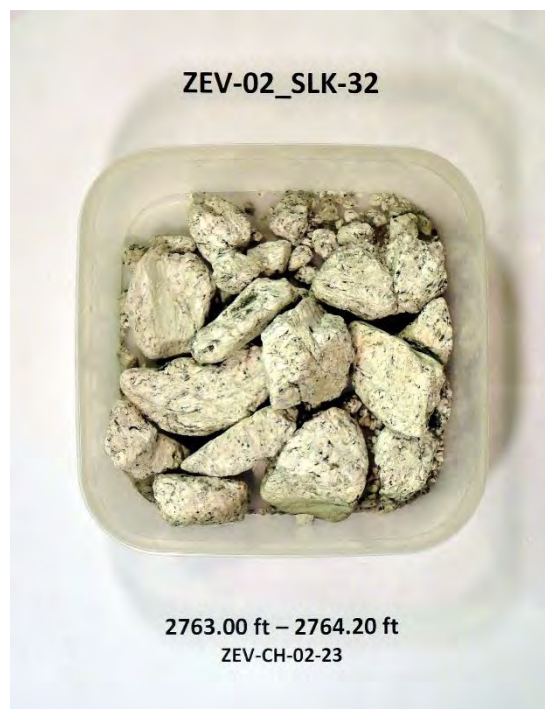


ZEV-02_SLK-31—After

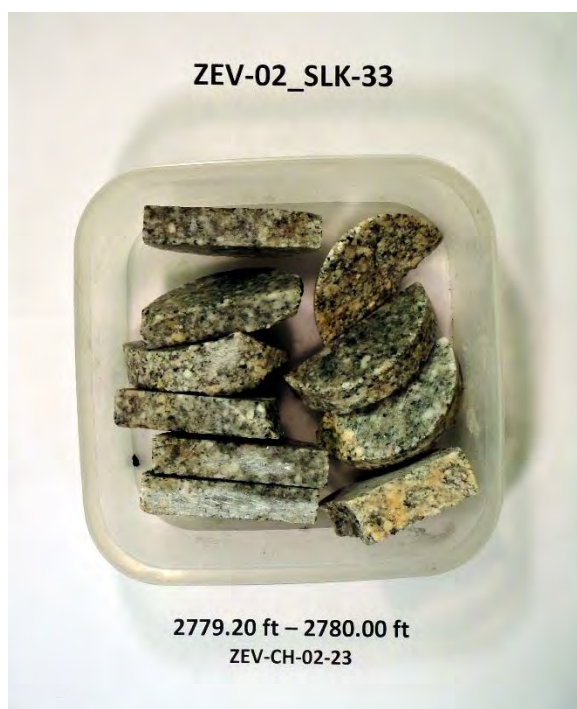
Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



ZEV-02_SLK-32—Before



ZEV-02_SLK-32—After

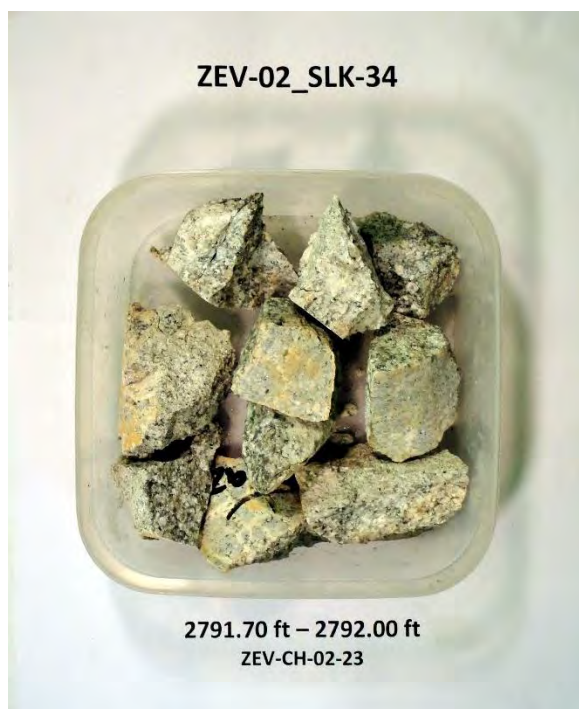


ZEV-02_SLK-33—Before

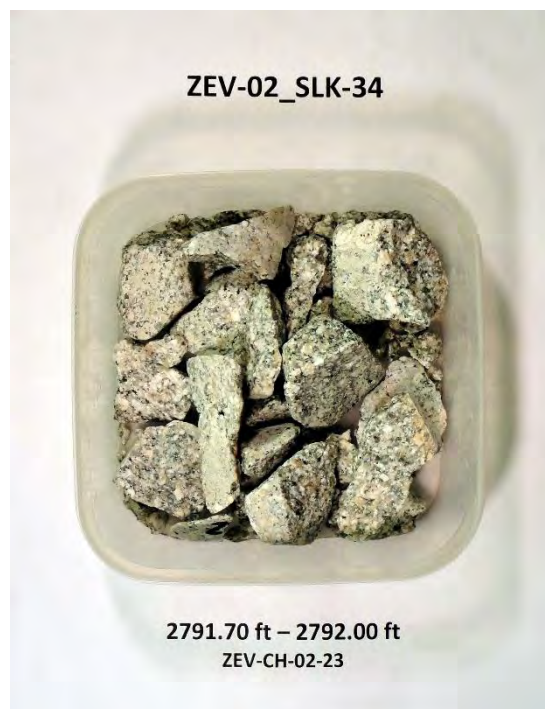


ZEV-02_SLK-33—After

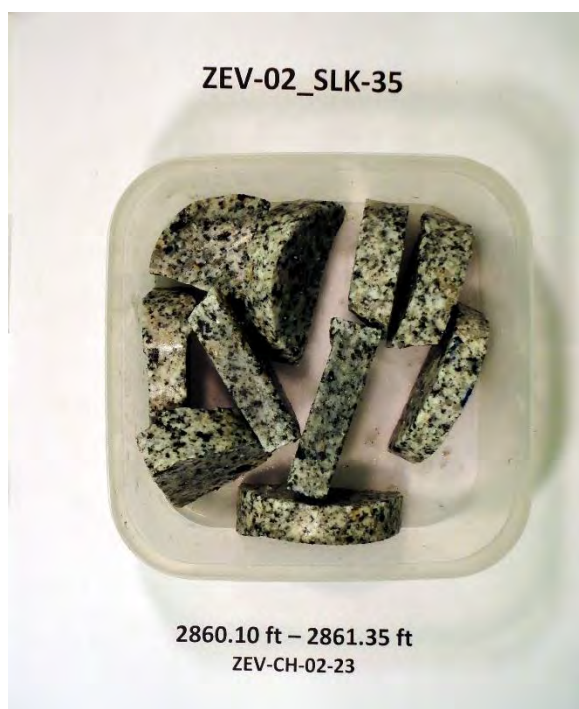
Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



ZEV-02_SLK-34—Before



ZEV-02_SLK-34—After

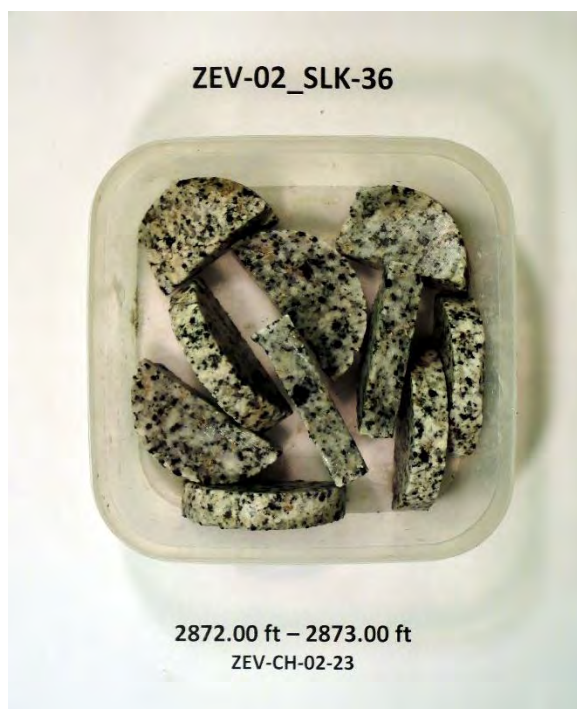


ZEV-02_SLK-35—Before

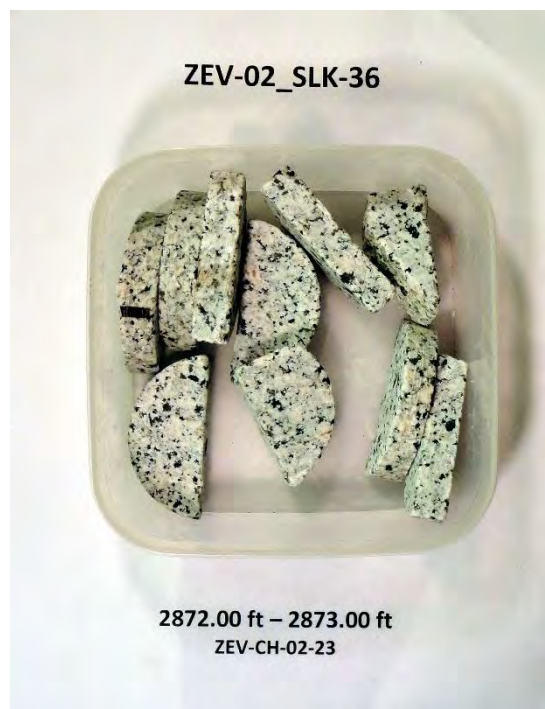


ZEV-02_SLK-35—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



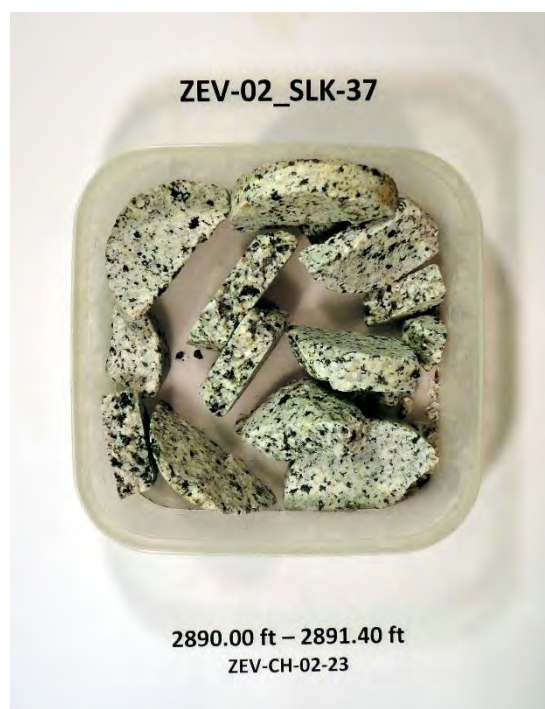
ZEV-02_SLK-36—Before



ZEV-02_SLK-36—After



ZEV-02_SLK-37—Before



ZEV-02_SLK-37—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



ZEV-02_SLK-38—Before



ZEV-02_SLK-38—After



ZEV-02_SLK-39—Before

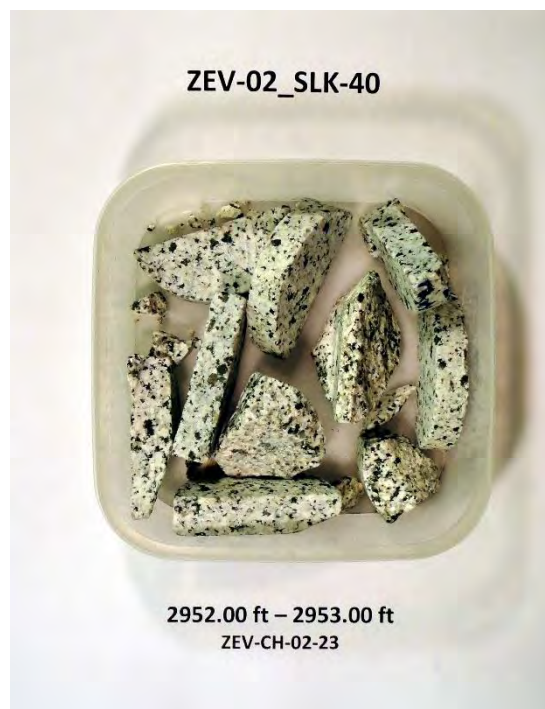


ZEV-02_SLK-39—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



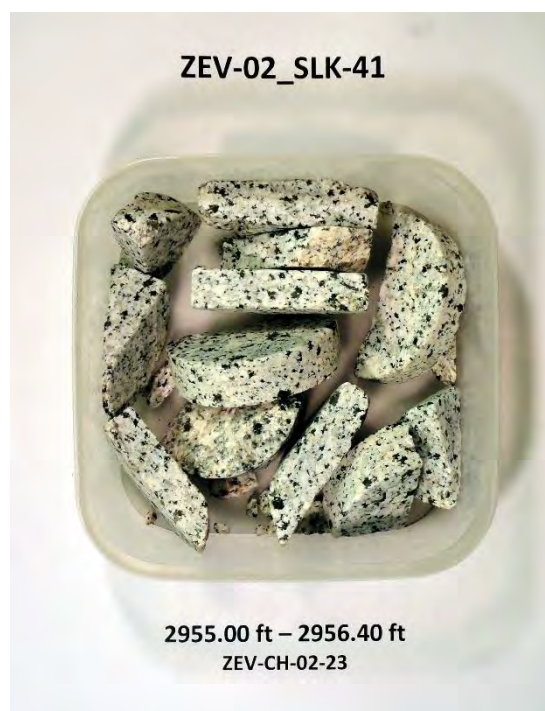
ZEV-02_SLK-40—Before



ZEV-02_SLK-40—After



ZEV-02_SLK-41—Before

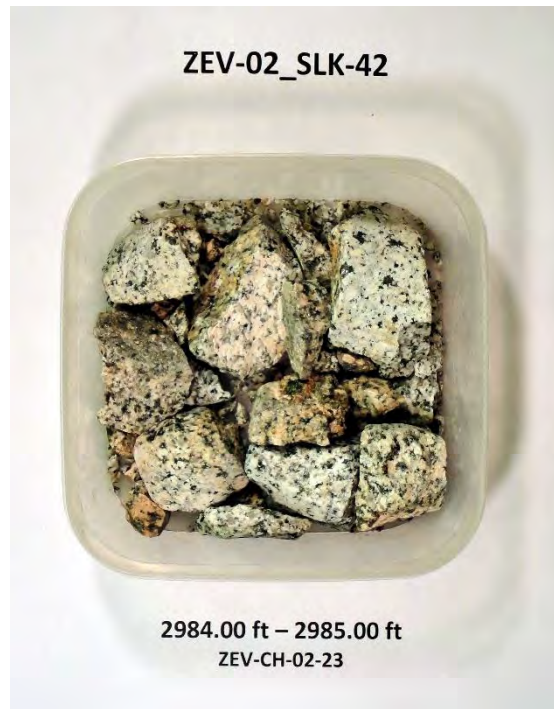


ZEV-02_SLK-41—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



ZEV-02_SLK-42—Before



ZEV-02_SLK-42—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(concluded)*

APPENDIX D

TRIAXIAL COMPRESSION TEST DATASHEET



AGAPITO ASSOCIATES, INC.
2913 Hill Avenue, #B
Grand Junction, CO 81504 USA
970-242-4220

TRIAXIAL COMPRESSION TESTS

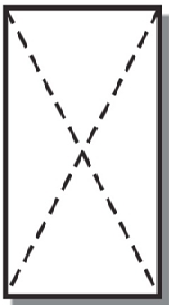
CLIENT	Lane PES
JOB NUMBER	951-20
DATE	May 16, 2023

HOLE NUMBER	ZEV-CH-01-23
MOISTURE CONDITION	As received
TEMPERATURE	Ambient

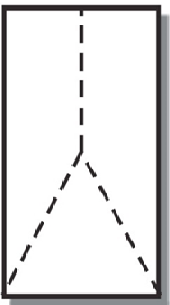
Specimen No.	Hole No.	Interval		Length of Interval (ft)	Weight (oz)	Diameter		Axial Length (in)	Length-to-Diameter Ratio	Area (in ²)	Volume (in ³)	Density (pcf)	Specific Gravity
		From (ft)	To (ft)			D ₁ (in)	D ₂ (in)						
ZEV-01_TRI-01	ZEV-CH-01-23	2,010.10	2,011.00	0.90	38.37	2.39	2.39	5.49	2.3	4.48	24.57	169	2.70
ZEV-01_TRI-02	ZEV-CH-01-23	2,035.00	2,036.00	1.00	38.21	2.39	2.39	5.46	2.3	4.48	24.46	169	2.70
ZEV-01_TRI-03	ZEV-CH-01-23	2,071.00	2,072.00	1.00	38.03	2.39	2.40	5.42	2.3	4.50	24.40	168	2.70
ZEV-01_TRI-04	ZEV-CH-01-23	2,104.10	2,105.40	1.30	36.47	2.38	2.38	5.26	2.2	4.45	23.43	168	2.69
ZEV-01_TRI-05	ZEV-CH-01-23	2,200.10	2,201.00	0.90	38.09	2.40	2.40	5.40	2.2	4.53	24.43	168	2.70
ZEV-01_TRI-06	ZEV-CH-01-23	2,237.00	2,238.30	1.30	38.79	2.39	2.40	5.52	2.3	4.50	24.88	168	2.70
ZEV-01_TRI-07	ZEV-CH-01-23	2,441.33	2,442.00	0.67	37.61	2.40	2.40	5.32	2.2	4.53	24.11	168	2.70
ZEV-01_TRI-08	ZEV-CH-01-23	2,467.50	2,468.10	0.60	38.17	2.40	2.40	5.50	2.3	4.53	24.91	166	2.65
ZEV-01_TRI-09	ZEV-CH-01-23	2,477.00	2,477.66	0.66	38.24	2.40	2.40	5.47	2.3	4.52	24.77	167	2.67
ZEV-01_TRI-10	ZEV-CH-01-23	2,980.00	2,981.00	1.00	38.15	2.40	2.40	5.43	2.3	4.51	24.50	168	2.69
ZEV-01_TRI-11	ZEV-CH-01-23	3,009.25	3,010.00	0.75	38.64	2.40	2.40	5.47	2.3	4.52	24.73	169	2.70
ZEV-01_TRI-12	ZEV-CH-01-23	3,018.00	3,018.41	0.41	38.61	2.40	2.40	5.49	2.3	4.53	24.87	168	2.69

Specimen No.	Lithological Description	Confining Pressure (psi)	Failure Load (lb)	Failure Stress (psi)	Failure Mode Notes	Comments
ZEV-01_TRI-01	Granite	1,500	117,985	26,365	Shear	
ZEV-01_TRI-02	Granite	2,500	167,480	37,363	Shear	
ZEV-01_TRI-03	Granite	1,500	142,710	31,691	Shear	
ZEV-01_TRI-04	Granite	2,500	185,260	41,616	Shear	
ZEV-01_TRI-05	Granite	1,500	93,275	20,605	Shear	
ZEV-01_TRI-06	Granite	2,500	165,480	36,747	Axial/Shear	
ZEV-01_TRI-07	Granite	1,500	140,660	31,041	Shear	
ZEV-01_TRI-08	Granite	2,500	115,565	25,519	Shear	
ZEV-01_TRI-09	Granite	1,500	119,080	26,317	Axial/Shear	
ZEV-01_TRI-10	Granite	2,500	73,750	16,343	Shear	
ZEV-01_TRI-11	Granite	1,500	161,545	35,732	Shear	
ZEV-01_TRI-12	Granite	2,500	146,070	32,268	Axial	

Failure Mode Sketches



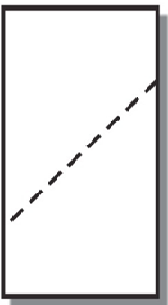
Cone



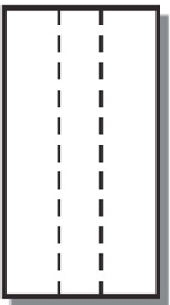
Cone and Axial Fracture/ Columnar



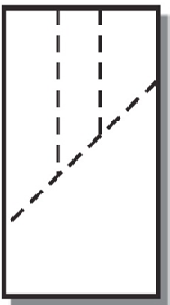
Cone and Shear



Shear



Axial Fracture/ Columnar



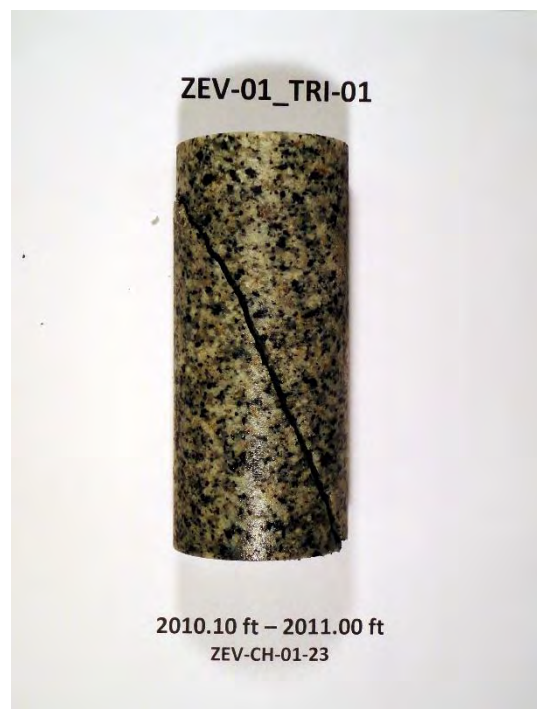
Shear and Axial Fracture/ Columnar

APPENDIX E

**BEFORE AND AFTER PHOTOGRAPHS OF
TRIAXIAL COMPRESSION TEST SPECIMENS**



ZEV-01_TRI-01—Before



ZEV-01_TRI-01—After



ZEV-01_TRI-02—Before



ZEV-01_TRI-02—After

Figure E-1. Before and After Photographs of Triaxial Compressive Strength Test Specimens



ZEV-01_TRI-03—Before



ZEV-01_TRI-03—After



ZEV-01_TRI-04—Before

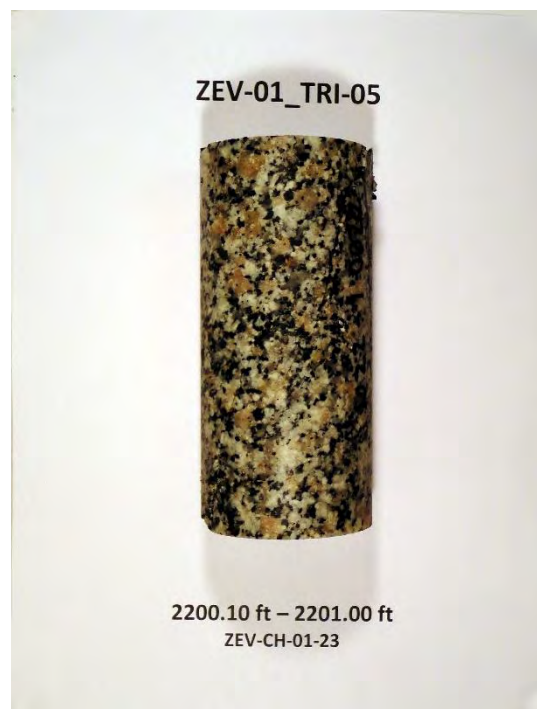


ZEV-01_TRI-04—After

Figure E-1. Before and After Photographs of Triaxial Compressive Strength Test Specimens *(continued)*



ZEV-01_TRI-05—Before



ZEV-01_TRI-05—After



ZEV-01_TRI-06—Before

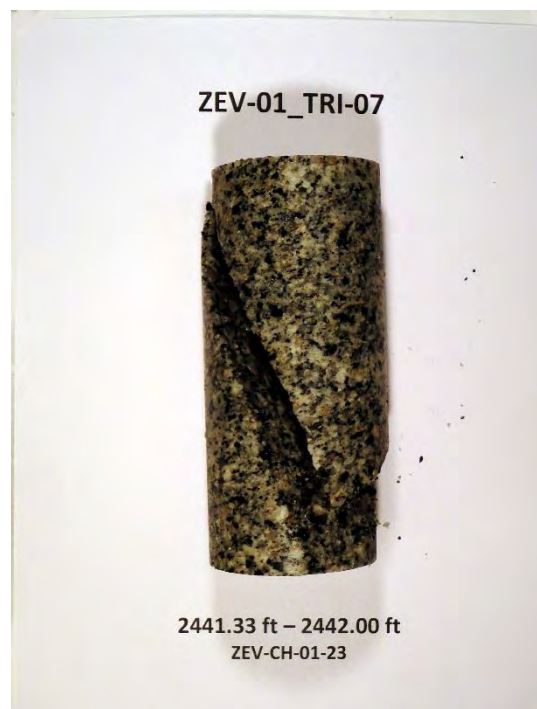


ZEV-01_TRI-06—After

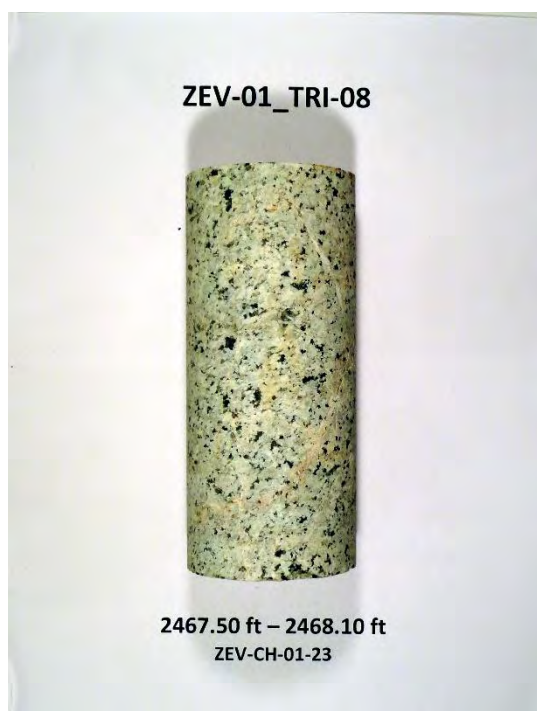
Figure E-1. Before and After Photographs of Triaxial Compressive Strength Test Specimens *(continued)*



ZEV-01_TRI-07—Before



ZEV-01_TRI-07—After

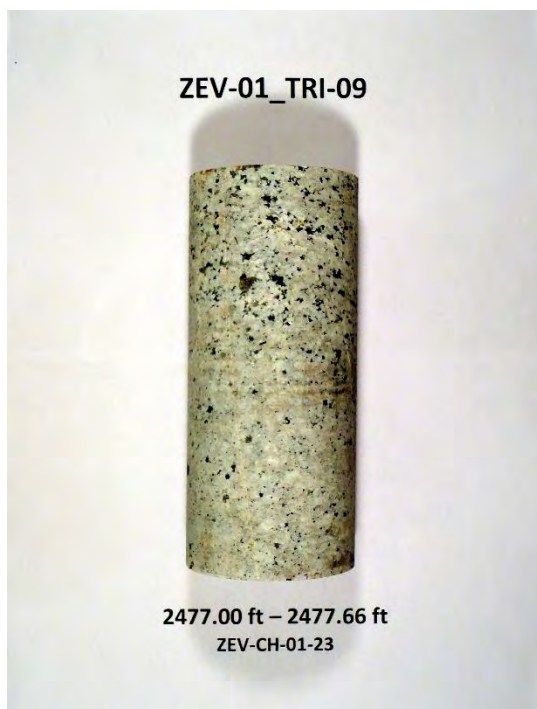


ZEV-01_TRI-08—Before



ZEV-01_TRI-08—After

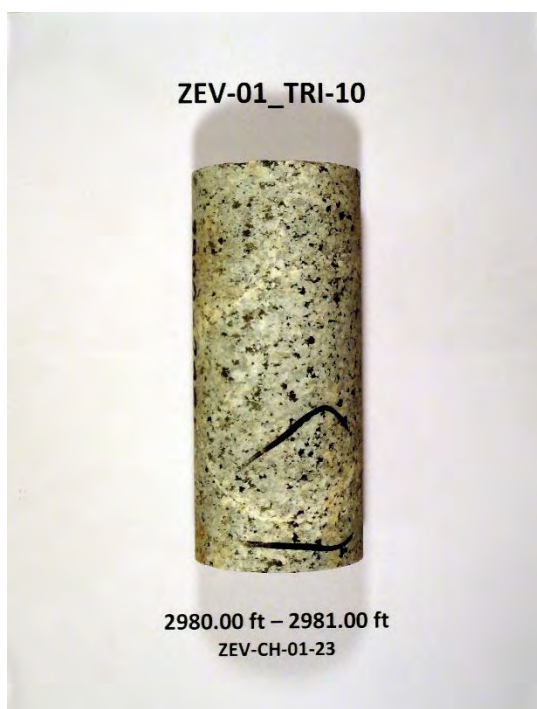
Figure E-1. Before and After Photographs of Triaxial Compressive Strength Test Specimens *(continued)*



ZEV-01_TRI-09—Before



ZEV-01_TRI-09—After



ZEV-01_TRI-10—Before



ZEV-01_TRI-10—After

Figure E-1. Before and After Photographs of Triaxial Compressive Strength Test Specimens *(continued)*



ZEV-01_TRI-11—Before



ZEV-01_TRI-11—After



ZEV-01_TRI-12—Before



ZEV-01_TRI-12—After

Figure E-1. Before and After Photographs of Triaxial Compressive Strength Test Specimens (concluded)

APPENDIX F

SPLITTING TENSILE STRENGTH (BRAZILIAN) TEST DATASHEET



AGAPITO ASSOCIATES, INC.
2913 Hill Avenue, #B
Grand Junction, CO 81504 USA
970-242-4220

INDIRECT TENSILE STRENGTH
(BRAZILIAN) TESTS

CLIENT	Lane PES
JOB NUMBER	951-20
DATE	May 16, 2023

HOLE NUMBER	ZEV-CH-01-23
MOISTURE CONDITION	As Received
TEMPERATURE	Ambient

Specimen No.	Hole No.	Depth		Length of Interval (ft)	Weight (oz)	Diameter		Axial Length (inch)	Area (inch ²)	Density (pcf)	Specific Gravity
		From (ft)	To (ft)			D ₁ (in)	D ₂ (in)				
ZEV-01_BTS-01	ZEV-CH-01-23	2,007.00	2,008.00	1.00	9.35	2.38	2.38	1.36	4.45	167	2.7
ZEV-01_BTS-02	ZEV-CH-01-23	2,032.00	2,033.00	1.00	9.46	2.39	2.39	1.37	4.49	167	2.7
ZEV-01_BTS-03	ZEV-CH-01-23	2,067.00	2,068.80	1.80	9.83	2.40	2.40	1.40	4.52	167	2.7
ZEV-01_BTS-04	ZEV-CH-01-23	2,096.00	2,097.00	1.00	9.63	2.40	2.40	1.37	4.52	168	2.7
ZEV-01_BTS-05	ZEV-CH-01-23	2,218.70	2,219.70	1.00	9.89	2.40	2.40	1.41	4.51	168	2.7
ZEV-01_BTS-06	ZEV-CH-01-23	2,243.00	2,244.00	1.00	9.37	2.39	2.39	1.35	4.49	167	2.7
ZEV-01_BTS-07	ZEV-CH-01-23	2,419.00	2,420.33	1.33	9.80	2.40	2.40	1.39	4.53	168	2.7
ZEV-01_BTS-08	ZEV-CH-01-23	2,456.00	2,456.90	0.90	9.94	2.40	2.40	1.41	4.53	168	2.7
ZEV-01_BTS-09	ZEV-CH-01-23	2,487.00	2,488.00	1.00	9.61	2.40	2.40	1.38	4.53	166	2.7
ZEV-01_BTS-10	ZEV-CH-01-23	2,978.00	2,978.58	0.58	9.23	2.40	2.40	1.31	4.51	168	2.7
ZEV-01_BTS-11	ZEV-CH-01-23	3,004.58	3,005.41	0.83	9.23	2.40	2.40	1.32	4.53	167	2.7
ZEV-01_BTS-12	ZEV-CH-01-23	1,519.30	1,520.10	0.80	8.26	2.40	2.41	1.23	4.54	160	2.6
ZEV-01_BTS-13	ZEV-CH-01-23	1,623.50	1,624.90	1.40	9.05	2.40	2.40	1.30	4.51	167	2.7
ZEV-01_BTS-14	ZEV-CH-01-23	1,794.40	1,795.40	1.00	6.77	2.40	2.39	0.98	4.51	165	2.6
ZEV-01_BTS-15	ZEV-CH-01-23	1,929.00	1,930.40	1.40	6.91	2.40	2.40	1.01	4.51	163	2.6
ZEV-01_BTS-16	ZEV-CH-01-23	2,032.00	2,033.00	1.00	9.46	2.39	2.39	1.37	4.49	167	2.7
ZEV-01_BTS-17	ZEV-CH-01-23	2,067.00	2,068.80	1.80	9.83	2.40	2.40	1.40	4.52	167	2.7
ZEV-01_BTS-18	ZEV-CH-01-23	2,096.00	2,097.00	1.00	9.63	2.40	2.40	1.37	4.52	168	2.7
ZEV-01_BTS-19	ZEV-CH-01-23	2,129.20	2,130.50	1.30	9.30	2.39	2.39	1.34	4.47	168	2.7
ZEV-01_BTS-20	ZEV-CH-01-23	2,161.10	2,162.00	0.90	7.04	2.39	2.39	1.02	4.47	166	2.7
ZEV-01_BTS-21	ZEV-CH-01-23	2,185.10	2,186.60	1.50	6.65	2.40	2.40	0.98	4.52	162	2.6
ZEV-01_BTS-22	ZEV-CH-01-23	2,243.00	2,244.00	1.00	8.73	2.39	2.39	1.27	4.48	165	2.7
ZEV-01_BTS-23	ZEV-CH-01-23	2,284.25	2,285.00	0.75	9.24	2.40	2.40	1.33	4.51	166	2.7
ZEV-01_BTS-24	ZEV-CH-01-23	2,302.00	2,303.25	1.25	8.61	2.41	2.40	1.32	4.54	155	2.5
ZEV-01_BTS-25	ZEV-CH-01-23	2,323.00	2,324.00	1.00	8.01	2.40	2.40	1.33	4.53	144	2.3
ZEV-01_BTS-26	ZEV-CH-01-23	2,371.25	2,372.00	0.75	8.58	2.40	2.40	1.27	4.53	161	2.6
ZEV-01_BTS-27	ZEV-CH-01-23	2,397.66	2,399.33	1.67	8.94	2.40	2.40	1.31	4.53	162	2.6
ZEV-01_BTS-28	ZEV-CH-01-23	2,524.20	2,525.33	1.13	8.84	2.40	2.40	1.26	4.52	168	2.7
ZEV-01_BTS-29	ZEV-CH-01-23	2,535.50	2,536.50	1.00	8.63	2.40	2.40	1.25	4.52	165	2.6
ZEV-01_BTS-30	ZEV-CH-01-23	2,584.75	2,585.67	0.92	8.58	2.42	2.41	1.31	4.57	154	2.5
ZEV-01_BTS-31	ZEV-CH-01-23	2,618.00	2,619.00	1.00	9.01	2.40	2.41	1.29	4.54	167	2.7
ZEV-01_BTS-32	ZEV-CH-01-23	2,622.80	2,624.50	1.70	8.78	2.40	2.40	1.24	4.54	168	2.7
ZEV-01_BTS-33	ZEV-CH-01-23	2,655.33	2,656.50	1.17	8.95	2.40	2.40	1.27	4.53	168	2.7
ZEV-01_BTS-34	ZEV-CH-01-23	2,686.90	2,689.45	2.55	7.94	2.42	2.41	1.28	4.57	147	2.4
ZEV-01_BTS-35	ZEV-CH-01-23	2,739.70	2,740.70	1.00	8.74	2.39	2.39	1.29	4.50	163	2.6
ZEV-01_BTS-36	ZEV-CH-01-23	2,773.70	2,775.35	1.65	8.80	2.40	2.40	1.27	4.51	166	2.7
ZEV-01_BTS-37	ZEV-CH-01-23	2,803.00	2,804.35	1.35	8.98	2.39	2.39	1.31	4.49	165	2.7
ZEV-01_BTS-38	ZEV-CH-01-23	2,842.20	2,843.20	1.00	8.46	2.41	2.40	1.30	4.54	155	2.5
ZEV-01_BTS-39	ZEV-CH-01-23	2,874.80	2,876.00	1.20	9.10	2.40	2.40	1.42	4.51	153	2.5
ZEV-01_BTS-40	ZEV-CH-01-23	2,902.50	2,903.30	0.80	8.88	2.40	2.40	1.27	4.52	167	2.7
ZEV-01_BTS-41	ZEV-CH-01-23	2,942.00	2,943.00	1.00	9.25	2.40	2.40	1.35	4.52	164	2.6

Specimen No.	Lithological Description	Failure Load (lb)	Splitting Tensile Strength (psi)	Failure Mode Notes
ZEV-01_BTS-01	Granite	8,425	1,661	Tensile - Center
ZEV-01_BTS-02	Granite	5,789	1,129	Tensile - Center; Secondary failure at pre-existing fracture
ZEV-01_BTS-03	Granite	7,213	1,364	Tensile - Center
ZEV-01_BTS-04	Granite	6,432	1,247	Tensile - Center
ZEV-01_BTS-05	Granite	3,132	590	Tensile - Center
ZEV-01_BTS-06	Granite	3,546	699	Tensile - Center
ZEV-01_BTS-07	Granite	7,539	1,440	Tensile - Center
ZEV-01_BTS-08	Granite	8,007	1,502	Tensile - Center
ZEV-01_BTS-09	Granite	6,576	1,265	Tensile - Offcenter
ZEV-01_BTS-10	Granite	7,425	1,501	Tensile - Center; Secondary failure at pre-existing fracture
ZEV-01_BTS-11	Granite	6,559	1,319	Tensile - Center; Secondary failure at pre-existing fracture
ZEV-01_BTS-12	Granite	3,077	665	Tensile - Center; Secondary failure at pre-existing fracture
ZEV-01_BTS-13	Granite	3,503	716	Tensile - Center
ZEV-01_BTS-14	Granite	4,726	1,278	Tensile - Center
ZEV-01_BTS-15	Granite	4,791	1,256	Tensile - Center
ZEV-01_BTS-16	Granite	-	-	Duplicates from Previous Testing, Not tested
ZEV-01_BTS-17	Granite	-	-	Duplicates from Previous Testing, Not tested
ZEV-01_BTS-18	Granite	-	-	Duplicates from Previous Testing, Not tested
ZEV-01_BTS-19	Granite	6,835	1,361	Tensile - Center
ZEV-01_BTS-20	Granite	4,087	1,066	Tensile - Center
ZEV-01_BTS-21	Granite	2,886	782	Tensile - Center; Secondary failure at pre-existing fracture
ZEV-01_BTS-22	Granite	5,601	1,174	Tensile - Center
ZEV-01_BTS-23	Granite	2,506	499	Tensile - Center
ZEV-01_BTS-24	Granite	946	190	Tensile - Offcenter
ZEV-01_BTS-25	Granite	1,222	244	Tensile - Offcenter
ZEV-01_BTS-26	Granite	4,030	840	Tensile - Center; Secondary failure at pre-existing fracture
ZEV-01_BTS-27	Granite	3,741	756	Tensile - Center; Secondary failure at pre-existing fracture
ZEV-01_BTS-28	Granite	6,229	1,314	Tensile - Center
ZEV-01_BTS-29	Granite	4,711	999	Tensile - Offcenter
ZEV-01_BTS-30	Granite	555	112	Tensile - Offcenter; multiple failures along fractures
ZEV-01_BTS-31	Granite	4,916	1,013	Tensile - Center
ZEV-01_BTS-32	Granite	6,412	1,366	Tensile - Center
ZEV-01_BTS-33	Granite	3,139	656	Tensile - Center; Secondary failure at pre-existing fracture
ZEV-01_BTS-34	Granite	586	121	Tensile - Center
ZEV-01_BTS-35	Granite	7,660	1,582	Tensile - Center
ZEV-01_BTS-36	Granite	4,965	1,042	Tensile - Center; Secondary failure at pre-existing fracture

APPENDIX G

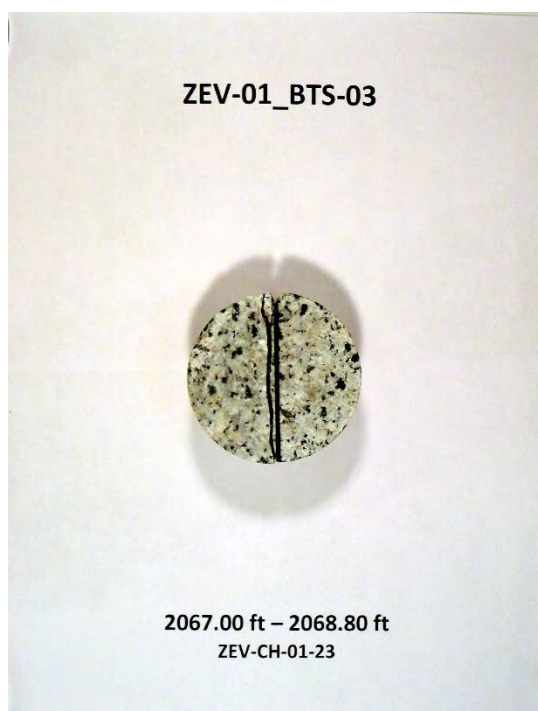
AFTER PHOTOGRAPHS OF BRAZILIAN TEST SPECIMENS



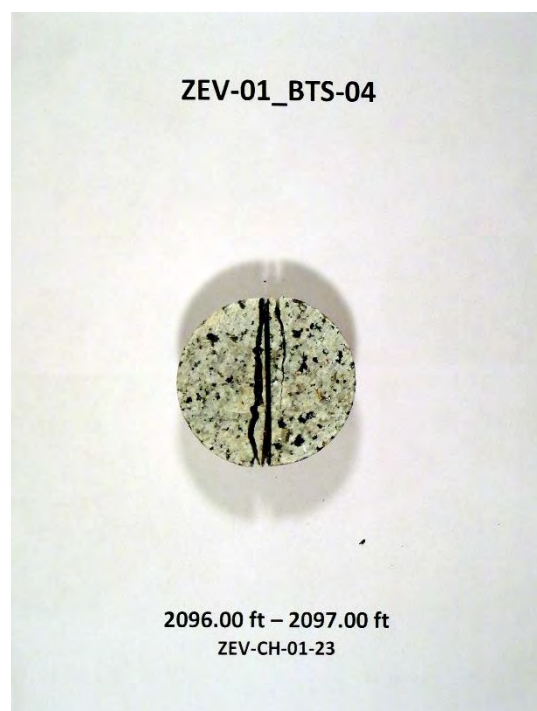
ZEV-01_BTS-01—After



ZEV-01_BTS-02—After



ZEV-01_BTS-03—After

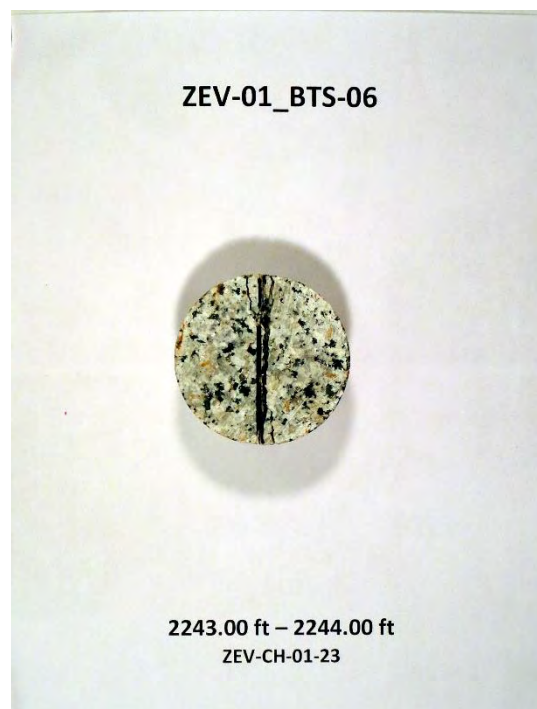


ZEV-01_BTS-04—After

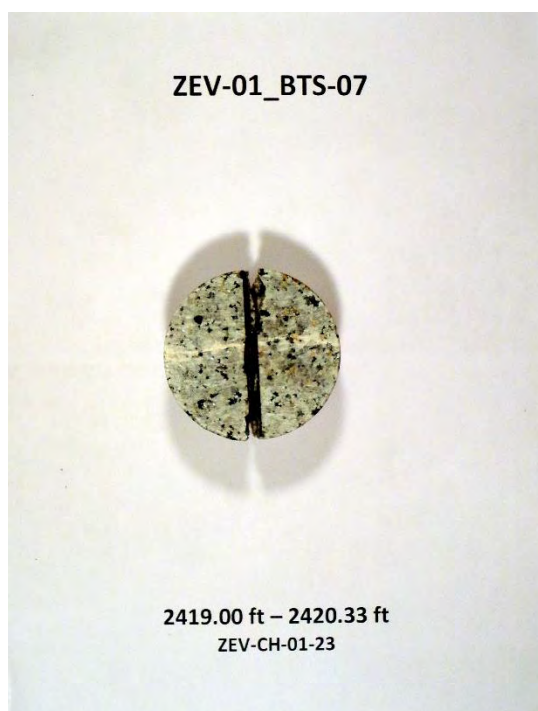
Figure G-1. After Photographs of Brazilian Test Specimens *(continued)*



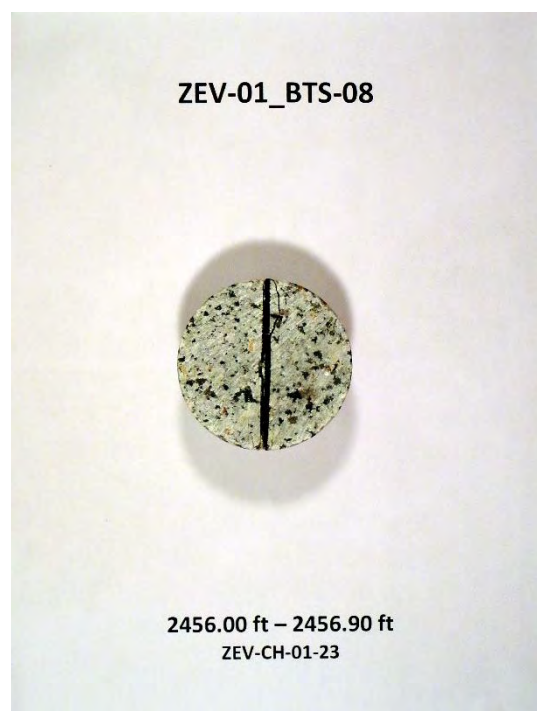
ZEV-01_BTS-05—After



ZEV-01_BTS-06—After

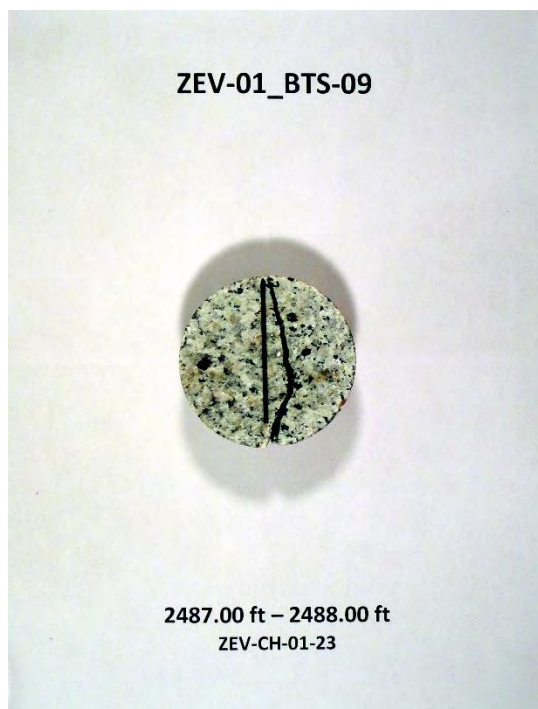


ZEV-01_BTS-07—After

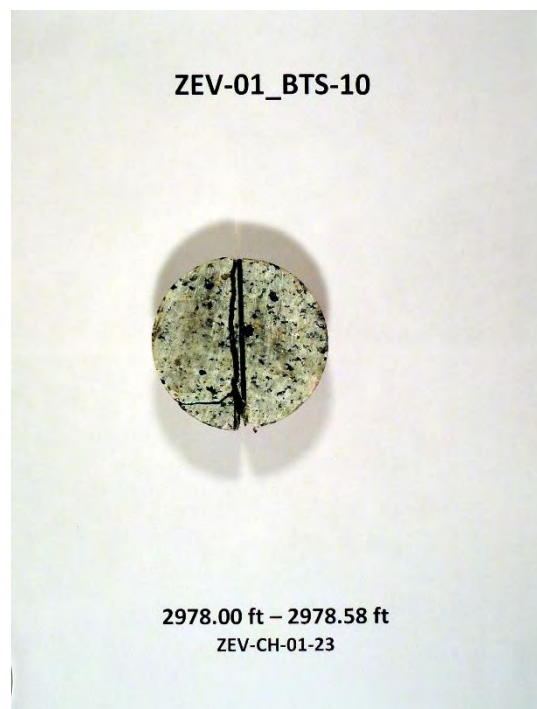


ZEV-01_BTS-08—After

Figure G-1. After Photographs of Brazilian Test Specimens *(continued)*



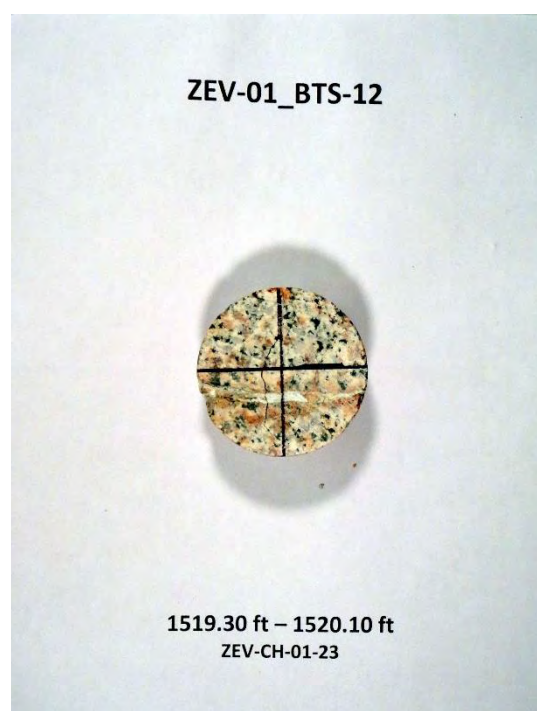
ZEV-01_BTS-09—After



ZEV-01_BTS-10—After

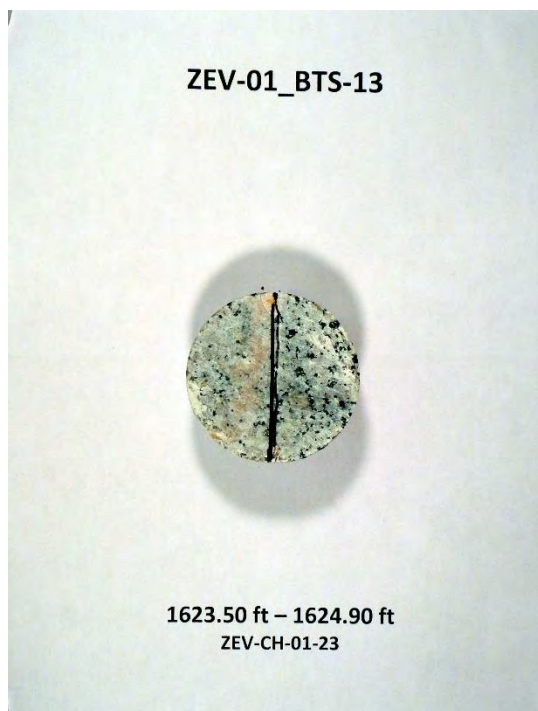


ZEV-01_BTS-11—After

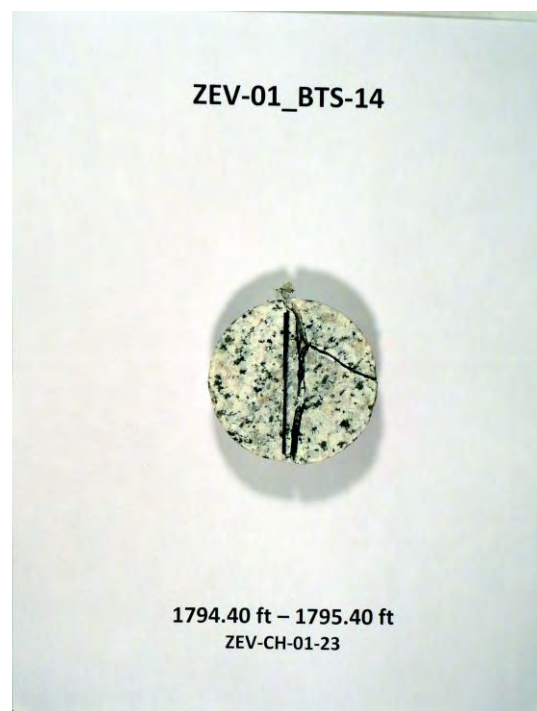


ZEV-01_BTS-12—After

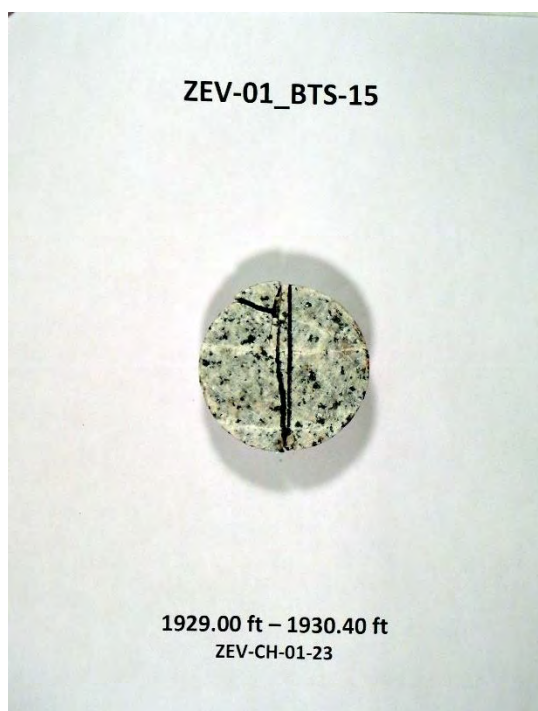
Figure G-1. After Photographs of Brazilian Test Specimens *(continued)*



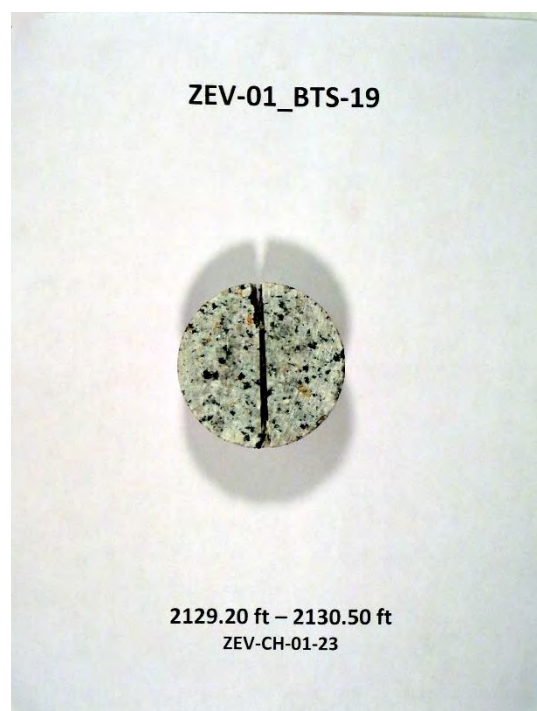
ZEV-01_BTS-13—After



ZEV-01_BTS-14—After

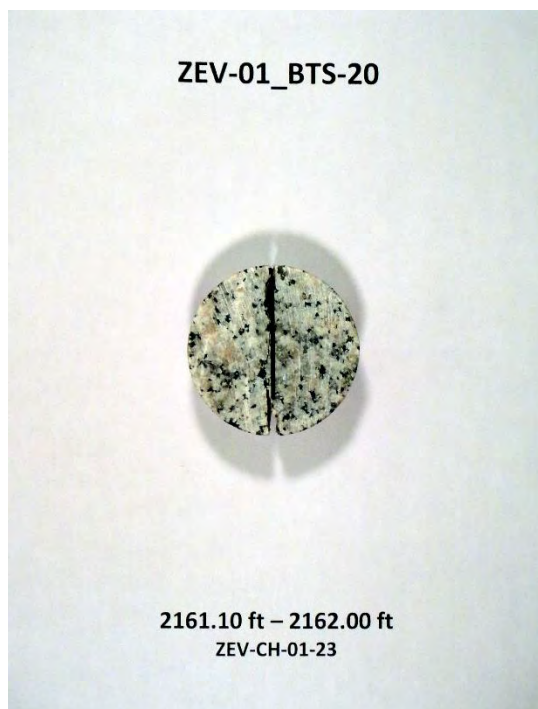


ZEV-01_BTS-15—After



ZEV-01_BTS-19—After

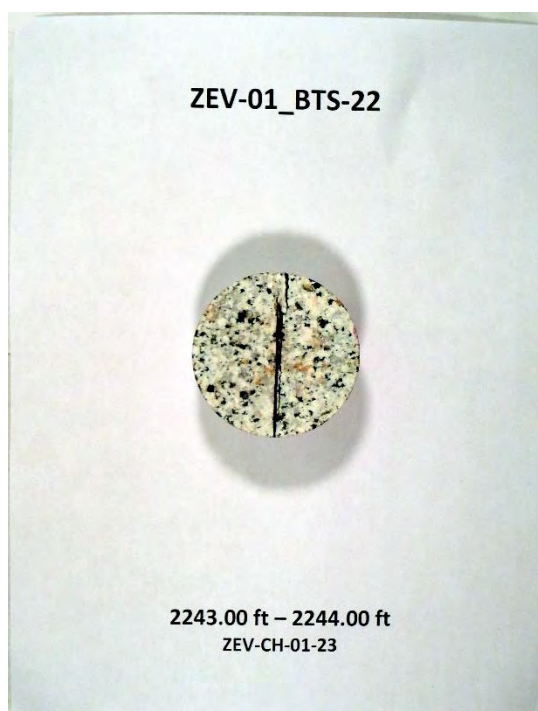
Figure G-1. After Photographs of Brazilian Test Specimens *(continued)*



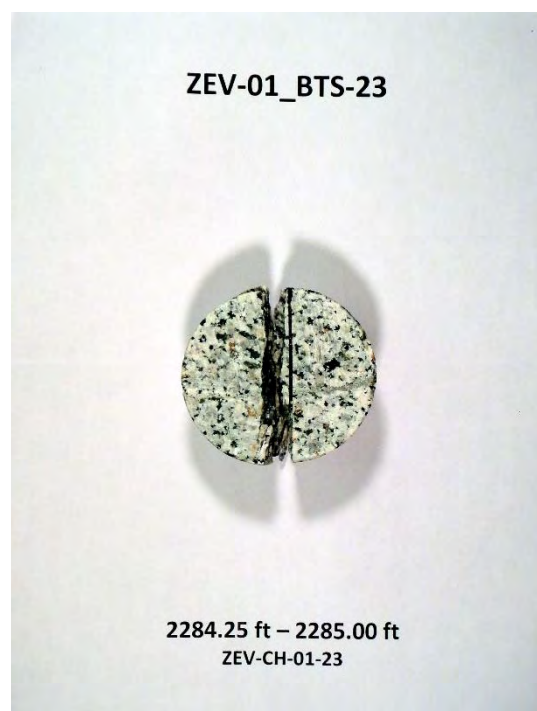
ZEV-01_BTS-20—After



ZEV-01_BTS-21—After

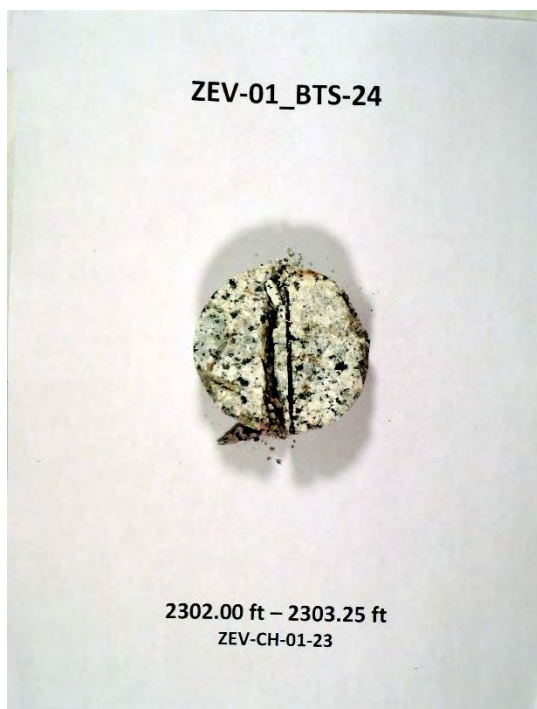


ZEV-01_BTS-22—After

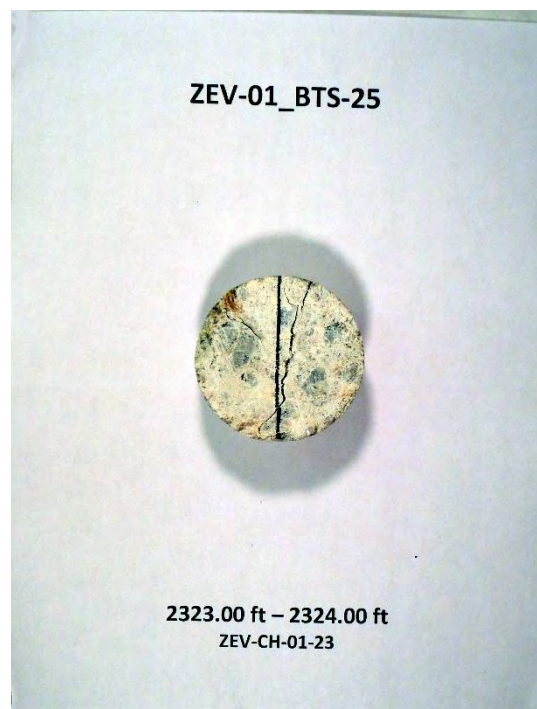


ZEV-01_BTS-23—After

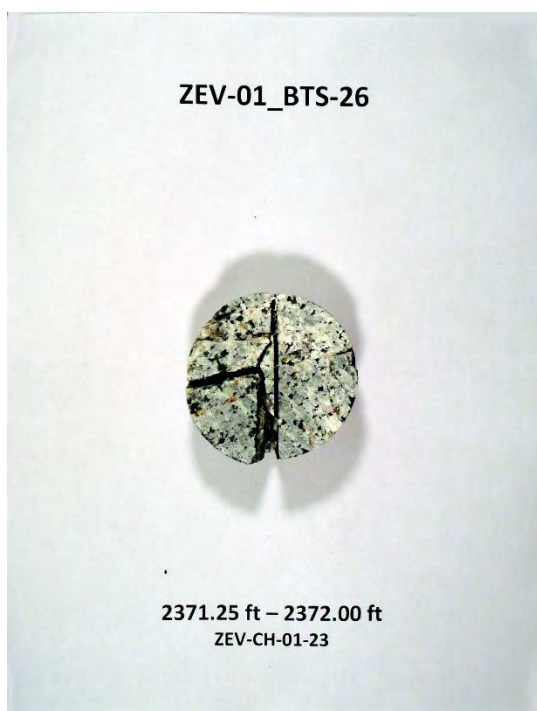
Figure G-1. After Photographs of Brazilian Test Specimens *(continued)*



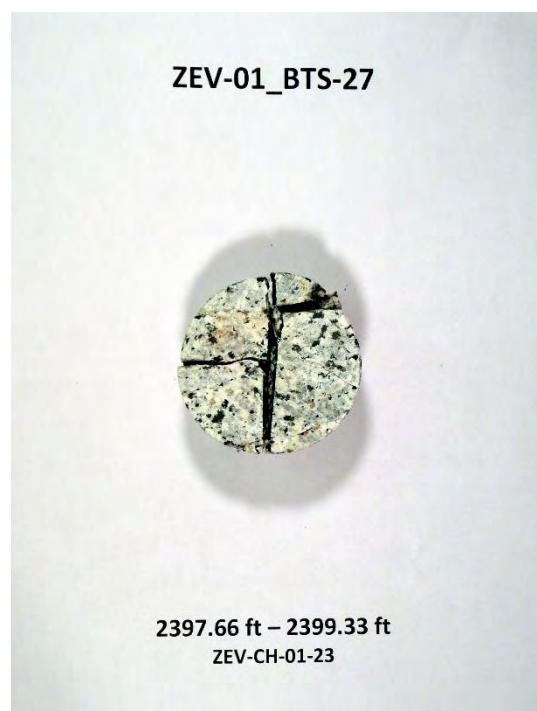
ZEV-01_BTS-24—After



ZEV-01_BTS-25—After

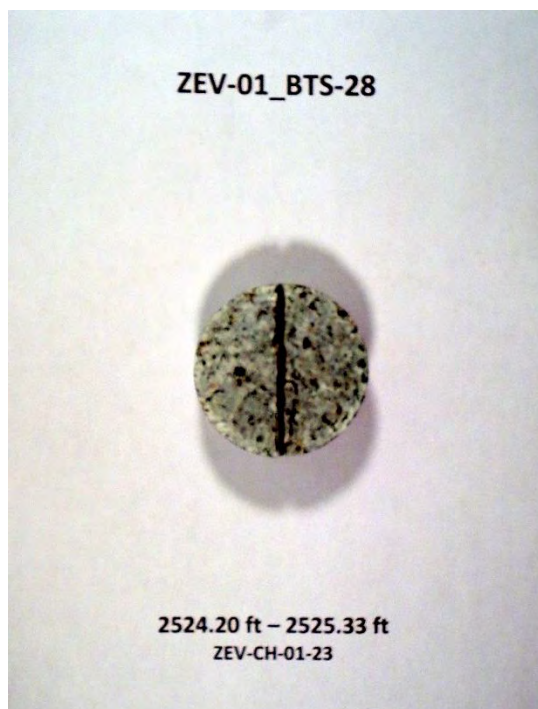


ZEV-01_BTS-26—After



ZEV-01_BTS-27—After

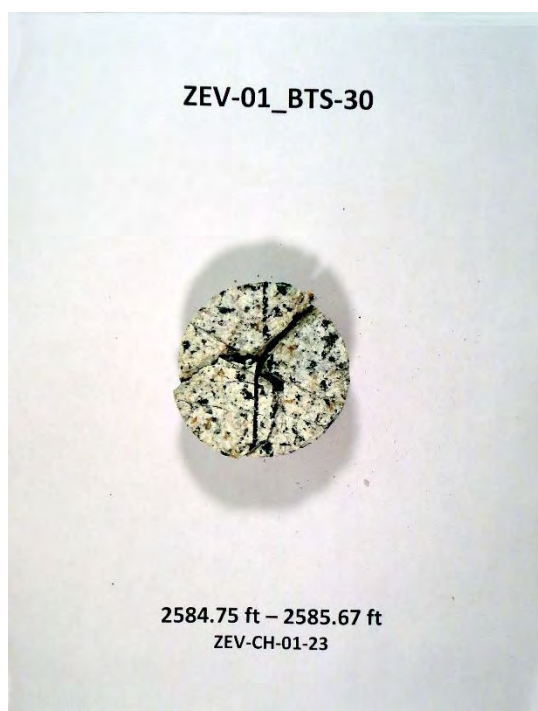
Figure G-1. After Photographs of Brazilian Test Specimens *(continued)*



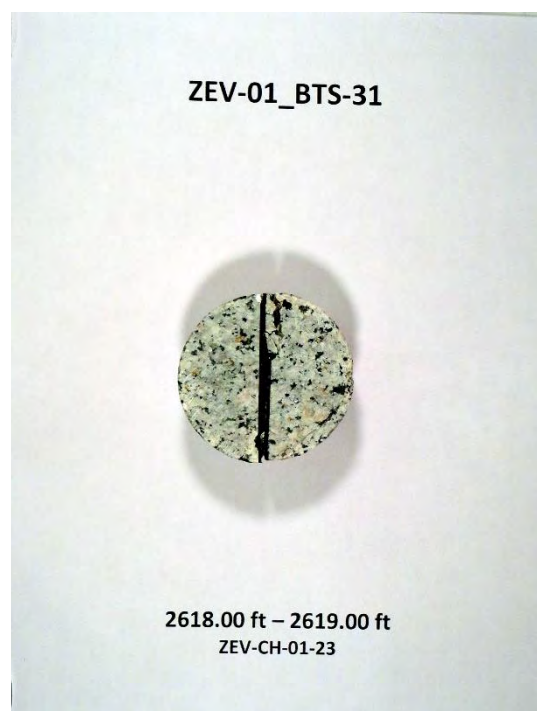
ZEV-01_BTS-28—After



ZEV-01_BTS-29—After



ZEV-01_BTS-30—After

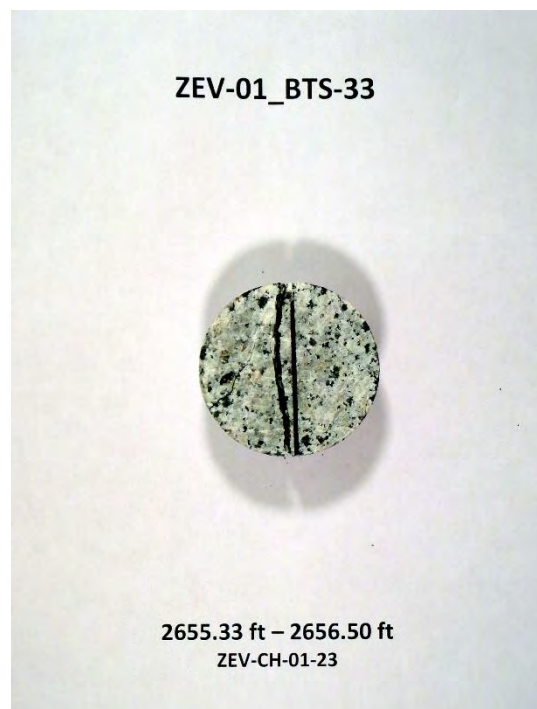


ZEV-01_BTS-31—After

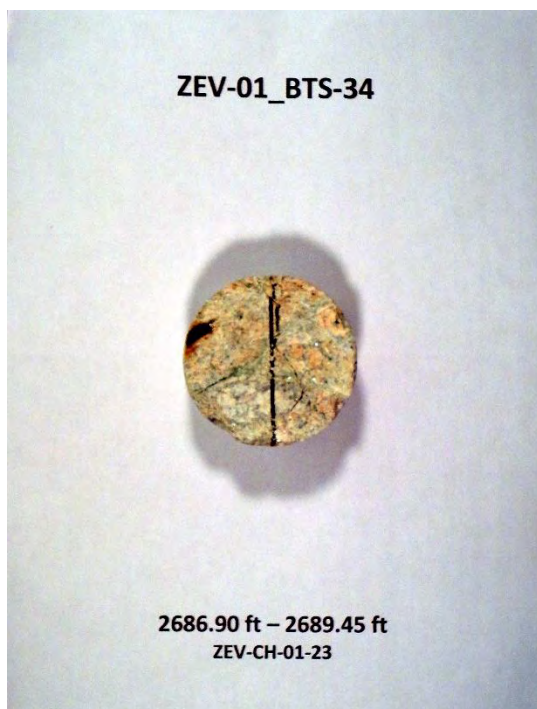
Figure G-1. After Photographs of Brazilian Test Specimens *(continued)*



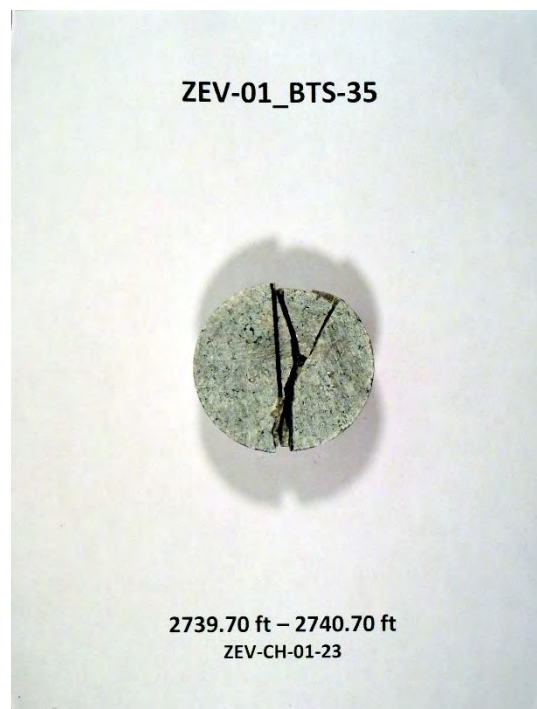
ZEV-01_BTS-32—After



ZEV-01_BTS-33—After

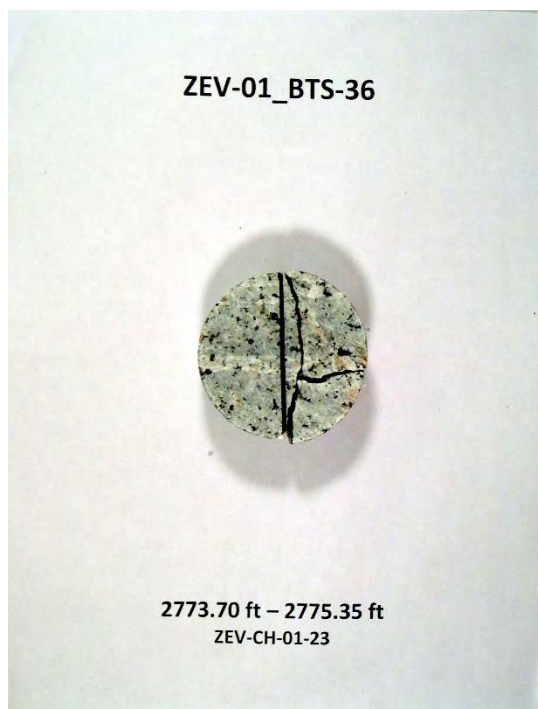


ZEV-01_BTS-34—After

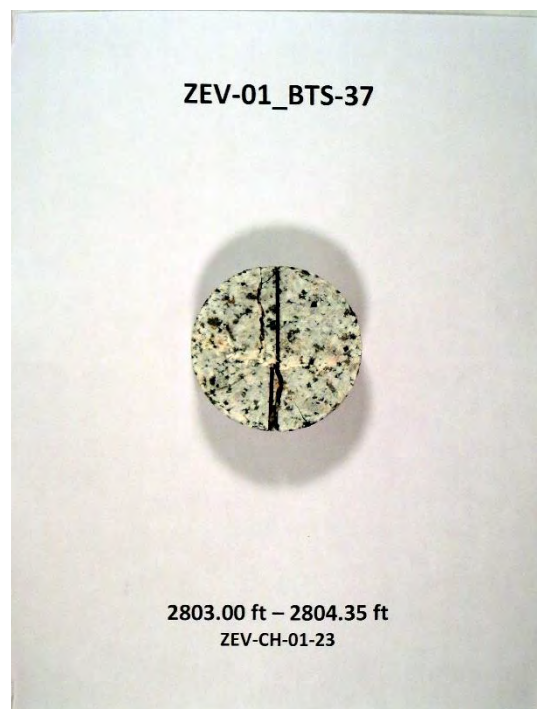


ZEV-01_BTS-35—After

Figure G-1. After Photographs of Brazilian Test Specimens *(continued)*



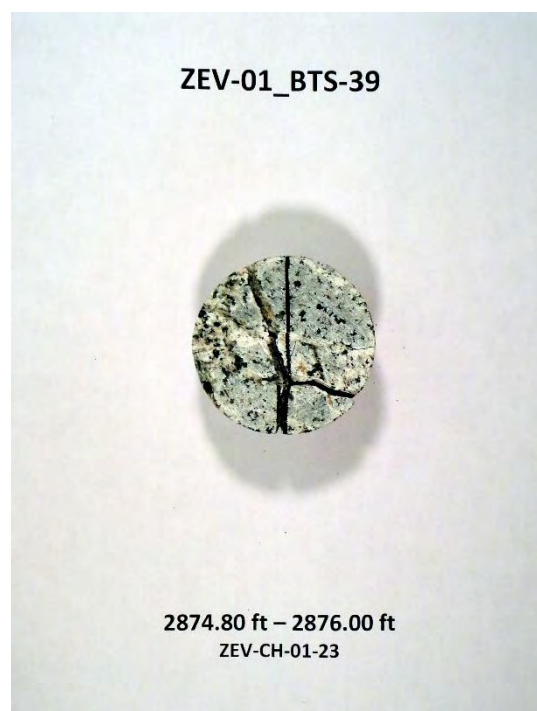
ZEV-01_BTS-36—After



ZEV-01_BTS-37—After

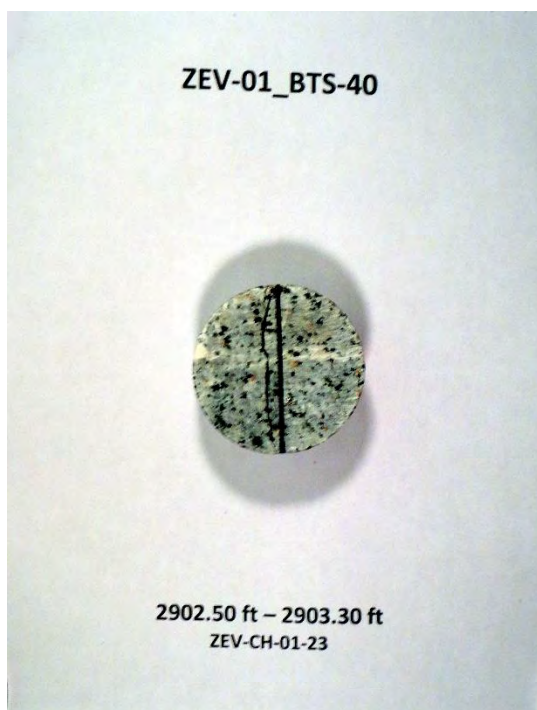


ZEV-01_BTS-38—After



ZEV-01_BTS-39—After

Figure G-1. After Photographs of Brazilian Test Specimens *(continued)*



ZEV-01_BTS-40—After



ZEV-01_BTS-41—After

Figure G-1. After Photographs of Brazilian Test Specimens (concluded)

APPENDIX H

POINT LOAD TEST DATASHEET



AGAPITO ASSOCIATES, INC.
2913 Hill Avenue, #B
Grand Junction, CO 81504 USA
970-242-4220

POINT LOAD TESTS

CLIENT	Lane PES
JOB NUMBER	951-20
DATE	May 16, 2023

HOLE NUMBER	ZEV-CH-01-23
MOISTURE CONDITION	As Received
TEMPERATURE	Ambient

Sample No.	Hole ID	Depth From (ft)	Depth To (ft)	Failure Gauge Pressure (psi)	"Failure Load" P (lb)	Test Orientation (A/D)	D (inch)	W (inch)	D _e (inch)	D _e ² (inch ²)	Uncorrected Point Load Index, I _p (psi)	Size Correction Factor, F	Corrected Point Load Index, I _{p(50)} (psi)	UCS (psi)	Lithology
ZEV-01_PLT-01	ZEV-CH-01-23	2005.00	2005.80	2,606	4,518	A	1.24	2.39	1.94	3.78	1,195	0.99	1,189	24,961	Granite
ZEV-01_PLT-02	ZEV-CH-01-23	2036.00	2037.00	1,664	2,885	A	1.34	2.39	2.02	4.07	708	1.01	716	15,044	Granite
ZEV-01_PLT-03	ZEV-CH-01-23	2062.00	2062.80	2,207	3,826	A	1.34	2.40	2.02	4.09	936	1.01	947	19,895	Granite
ZEV-01_PLT-04	ZEV-CH-01-23	2094.60	2095.90	1,030	1,786	A	1.28	2.40	1.98	3.91	457	1.00	458	9,611	Granite
ZEV-01_PLT-05	ZEV-CH-01-23	2195.80	2196.50	1,578	2,736	A	1.30	2.40	1.99	3.97	689	1.01	693	14,548	Granite
ZEV-01_PLT-06	ZEV-CH-01-23	2238.30	2239.60	1,899	3,292	A	1.32	2.39	2.00	4.02	819	1.01	826	17,344	Granite
ZEV-01_PLT-07	ZEV-CH-01-23	2243.00	2244.00	1,007	1,746	A	1.24	2.39	1.94	3.78	462	0.99	459	9,647	Granite
ZEV-01_PLT-08	ZEV-CH-01-23	2426.00	2427.00	2,885	5,002	A	1.28	2.40	1.98	3.91	1,278	1.00	1,281	26,893	Granite
ZEV-01_PLT-09	ZEV-CH-01-23	2447.33	2448.60	2,801	4,856	A	1.30	2.40	1.99	3.97	1,223	1.01	1,230	25,820	Granite
ZEV-01_PLT-10	ZEV-CH-01-23	2472.80	2473.50	1,484	2,573	A	1.32	1.56	1.62	2.62	982	0.92	899	18,886	Granite
ZEV-01_PLT-11	ZEV-CH-01-23	2978.00	2978.58	1,593	2,762	A	1.26	2.40	1.96	3.84	719	1.00	717	15,063	Granite
ZEV-01_PLT-12	ZEV-CH-01-23	2993.00	2994.00	1,349	2,339	A	1.20	2.40	1.92	3.67	638	0.99	630	13,224	Granite
ZEV-01_PLT-13	ZEV-CH-01-23	3017.25	3017.91	2,587	4,485	A	1.26	2.40	1.96	3.85	1,164	1.00	1,163	24,415	Granite
ZEV-01_PLT-14	ZEV-CH-01-23	1562.4	1563.40	696	1,207	A	0.98	2.40	1.73	3.00	402	0.94	379	7,965	Granite
ZEV-01_PLT-15	ZEV-CH-01-23	1565.50	1566.45	313	543	A	1.26	2.40	1.96	3.85	141	1.00	141	2,958	Granite
ZEV-01_PLT-16	ZEV-CH-01-23	1575.00	1576.30	1,880	3,259	A	0.98	2.40	1.73	3.00	1,086	0.94	1,025	21,528	Granite
ZEV-01_PLT-17	ZEV-CH-01-23	1623.50	1624.90	1,917	3,324	A	1.14	2.40	1.87	3.48	955	0.98	932	19,570	Granite
ZEV-01_PLT-18	ZEV-CH-01-23	1646.00	1646.70	1,263	2,190	A	0.94	2.40	1.70	2.88	759	0.94	710	14,916	Granite
ZEV-01_PLT-19	ZEV-CH-01-23	1647.35	1648.25	1,550	2,687	A	0.96	2.40	1.72	2.95	912	0.94	858	18,009	Granite
ZEV-01_PLT-20	ZEV-CH-01-23	1754.90	1755.50	830	1,439	A	0.98	2.39	1.73	3.00	480	0.94	453	9,508	Granite
ZEV-01_PLT-21	ZEV-CH-01-23	1760.70	1761.60	1,552	2,691	A	1.02	2.39	1.77	3.12	862	0.95	821	17,249	Granite
ZEV-01_PLT-22	ZEV-CH-01-23	1779.60	1781.30	1,117	1,937	A	0.98	2.39	1.73	3.00	646	0.94	610	12,804	Granite
ZEV-01_PLT-23	ZEV-CH-01-23	1831.10	1831.70	1,038	1,800	A	0.98	2.39	1.73	3.00	600	0.94	567	11,898	Granite
ZEV-01_PLT-24	ZEV-CH-01-23	1867.80	1868.30	1,550	2,687	A	0.96	2.39	1.71	2.94	915	0.94	860	18,050	Granite
ZEV-01_PLT-25	ZEV-CH-01-23	1932.30	1932.90	1,936	3,357	A	0.96	2.39	1.71	2.94	1,142	0.94	1,073	22,542	Granite
ZEV-01_PLT-26	ZEV-CH-01-23	1933.00	1933.60	1,025	1,777	A	0.98	2.39	1.73	3.00	592	0.94	559	11,745	Granite
ZEV-01_PLT-27	ZEV-CH-01-23	1937.20	1938.00	2,081	3,608	A	1.00	2.39	1.75	3.06	1,181	0.95	1,119	23,502	Granite
ZEV-01_PLT-28	ZEV-CH-01-23	1961.00	1961.50	180	312	A	0.96	2.40	1.72	2.94	106	0.94	100	2,093	Granite
ZEV-01_PLT-29	ZEV-CH-01-23	1961.50	1962.00	757	1,312	A	1.00	2.39	1.75	3.06	429	0.95	407	8,539	Granite
ZEV-01_PLT-30	ZEV-CH-01-23	2133.00	2133.60	1,762	3,055	A	0.98	2.39	1.73	2.99	1,021	0.94	963	20,226	Granite
ZEV-01_PLT-31	ZEV-CH-01-23	2177.50	2178.00	468	811	A	0.98	2.40	1.73	3.00	270	0.94	255	5,356	Granite
ZEV-01_PLT-32	ZEV-CH-01-23	2244.00	2244.60	2,206	3,825	A	1.20	2.40	1.92	3.67	1,043	0.99	1,030	21,626	Granite
ZEV-01_PLT-33	ZEV-CH-01-23	2265.83	2266.41	664	1,151	A	1.20	2.40	1.92	3.67	313	0.99	310	6,504	Granite
ZEV-01_PLT-34	ZEV-CH-01-23	2312.50	2322.00	235	407	A	1.18	2.39	1.90	3.59	113	0.98	111	2,341	Granite
ZEV-01_PLT-35	ZEV-CH-01-23	2342.75	2343.50	1,635	2,835	A	1.26	2.40	1.96	3.85	736	1.00	735	15,443	Granite
ZEV-01_PLT-36	ZEV-CH-01-23	2367.58	2368.00	1,984	3,440	A	1.16	2.40	1.88	3.55	969	0.98	950	19,955	Granite
ZEV-01_PLT-37	ZEV-CH-01-23	2530.00	2531.00	1,832	3,176	A	1.20	30.50	6.83	46.63	68	1.75	119	2,504	Granite
ZEV-01_PLT-38	ZEV-CH-01-23	2554.00	2554.80	-	-	-	-	-	-	-	-	-	-	-	Granite
ZEV-01_PLT-39	ZEV-CH-01-23	2590.00	2591.00	52	90	A	1.26	2.41	1.96	3.86	23	1.00	23	490	Granite
ZEV-01_PLT-40	ZEV-CH-01-23	2603.00	2603.80	384	666	A	1.22	2.41	1.93	3.74	178	0.99	177	3,711	Granite
ZEV-01_PLT-41	ZEV-CH-01-23	2632.75	2634.33	1,152	1,997	A	1.22	2.40	1.93	3.73	535	0.99	531	11,145	Granite
ZEV-01_PLT-42	ZEV-CH-01-23	2669.67	2670.25	189	328	A	1.16	2.41	1.89	3.56	92	0.98	90	1,896	Granite
ZEV-01_PLT-43	ZEV-CH-01-23	2689.85	2690.65	380	659	A	1.24	2.40	1.95	3.79	174	1.00	173	3,629	Granite
ZEV-01_PLT-44	ZEV-CH-01-23	2722.30	2722.90	2,360	4,092	A	1.30	2.39	1.99	3.96	1,034	1.00	1,039	21,814	Granite
ZEV-01_PLT-45	ZEV-CH-01-23	2763.25	2763.90	1,584	2,746	A	1.18	2.40	1.90	3.60	763	0.98	750	15,751	Granite
ZEV-01_PLT-46	ZEV-CH-01-23	2775.90	2776.10	1,040	1,803	A	1.16	2.40	1.88	3.54	509	0.98	499	10,474	Granite
ZEV-01_PLT-47	ZEV-CH-01-23	2804.35	2805.00	1,524	2,642	A	1.20	2.40	1.91	3.66	721	0.99	712	14,957	Granite
ZEV-01_PLT-48	ZEV-CH-01-23	2844.12	2844.70	1,313	2,276	A	1.26	2.39	1.96	3.84	593	1.00	591	12,421	Granite
ZEV-01_PLT-49	ZEV-CH-01-23	2873.30	2874.00	299	518	A	1.22	2.40	1.93	3.72	139	0.99	138	2,897	Granite
ZEV-01_PLT-50	ZEV-CH-01-23	2910.30	2911.00	482	836	A	1.22	2.38	1.92	3.70	226	0.99	224	4,699	Granite

APPENDIX I

SLAKE DURABILITY TEST DATASHEET



AGAPITO ASSOCIATES, INC.
2913 Hill Avenue, #B
Grand Junction, CO 81504 USA
970-242-4220

SLAKE DURABILITY TESTS

CLIENT	Lane PES
JOB NUMBER	951-20
DATE	May 16, 2023

HOLE NUMBER	ZEV-CH-01-23
MOISTURE CONDITION	As Received
OVEN TEMPERATURE	230 ± 10 °F

SYMBOLS:

A	Weight of drum plus sample at natural moisture content (oz)	D	Weight of drum plus oven-dried sample before second cycle (oz)
B	Weight of drum plus oven-dried sample before first cycle (oz)	W _F	Weight of drum plus oven-dried sample after second cycle (oz)
C	Weight of drum (oz)		

Specimen No.	Hole ID	Interval		Length of Interval (ft)	A (oz)	B (oz)	C (oz)	D (oz)	Water Temperature Cycle 1		W _F (oz)	Water Temperature Cycle 2		Moisture Content (%)	Slake Durability Index I _d (2)	Retained Materials Description	
		From (ft)	To (ft)						Before (°F)	After (°F)		Before (°F)	After (°F)			Lithology	Type*
ZEV-01_SLK-01	ZEV-CH-01-23	2,009.00	2,010.00	1.00	71.800	71.760	53.410	71.725	61.6	69.0	71.705	71.0	78.0	0.2%	99.7	Granite	I
ZEV-01_SLK-02	ZEV-CH-01-23	2,034.00	2,035.00	1.00	71.720	71.670	53.410	71.645	61.6	69.0	71.620	71.0	78.0	0.3%	99.7	Granite	I
ZEV-01_SLK-03	ZEV-CH-01-23	2,070.00	2,071.00	1.00	70.655	70.595	52.665	70.550	61.6	69.0	70.515	71.0	78.0	0.3%	99.6	Granite	I
ZEV-01_SLK-04	ZEV-CH-01-23	2,102.00	2,102.90	0.90	71.520	71.445	53.655	71.385	68.8	73.0	71.350	78.0	81.9	0.4%	99.5	Granite	I
ZEV-01_SLK-05	ZEV-CH-01-23	2,197.00	2,198.20	1.20	73.750	73.705	54.915	73.665	68.8	73.0	73.635	78.0	81.9	0.2%	99.6	Granite	I
ZEV-01_SLK-06	ZEV-CH-01-23	2,224.50	2,225.50	1.00	72.280	72.245	55.315	72.205	68.8	73.0	72.175	78.0	81.9	0.2%	99.6	Granite	I
ZEV-01_SLK-07	ZEV-CH-01-23	2,247.00	2,248.00	1.00	72.880	72.840	55.400	72.795	71.8	72.0	72.770	81.9	81.9	0.2%	99.6	Granite	I
ZEV-01_SLK-08	ZEV-CH-01-23	2,409.16	2,410.33	1.17	70.980	70.955	53.405	70.870	66.4	72.3	70.840	65.9	73.2	0.1%	99.3	Granite	I
ZEV-01_SLK-09	ZEV-CH-01-23	2,439.00	2,440.96	1.96	70.100	70.085	53.405	70.035	66.4	72.3	70.015	65.9	73.2	0.1%	99.6	Granite	I
ZEV-01_SLK-10	ZEV-CH-01-23	2,448.60	2,450.00	1.40	71.210	71.200	52.665	71.145	66.4	72.3	71.120	65.9	73.2	0.1%	99.6	Granite	I
ZEV-01_SLK-11	ZEV-CH-01-23	2,977.00	2,977.91	0.91	72.275	72.250	53.645	72.190	72.3	73.6	72.130	73.2	74.6	0.1%	99.4	Granite	I
ZEV-01_SLK-12	ZEV-CH-01-23	3,005.50	3,006.50	1.00	72.135	72.110	54.910	72.005	72.3	73.6	71.905	73.2	74.6	0.1%	98.8	Granite	I
ZEV-01_SLK-13	ZEV-CH-01-23	1,979.30	1,980.60	1.30	71.405	71.365	53.755	71.210	82.0	80.0	71.140	59.4	65.5	0.2%	98.7	Granite	I
ZEV-01_SLK-14	ZEV-CH-01-23	1,983.30	1,984.20	0.90	71.440	71.350	53.420	71.200	82.0	80.0	71.070	59.4	65.5	0.5%	98.4	Granite	I
ZEV-01_SLK-15	ZEV-CH-01-23	2,138.50	2,140.00	1.50	70.025	70.000	52.670	69.860	82.0	80.0	69.810	59.4	65.5	0.1%	98.9	Granite	I
ZEV-01_SLK-16	ZEV-CH-01-23	2,174.70	2,175.80	1.10	70.335	70.265	53.655	70.120	80.0	79.4	70.045	64.4	68.9	0.4%	98.7	Granite	I
ZEV-01_SLK-17	ZEV-CH-01-23	2,197.00	2,198.20	1.20	72.605	72.560	54.915	72.400	80.0	79.4	72.345	64.4	68.9	0.3%	98.8	Granite	I
ZEV-01_SLK-18	ZEV-CH-01-23	2,224.50	2,225.50	1.00	72.790	72.745	55.325	72.610	80.0	79.4	72.565	64.4	68.9	0.3%	99.0	Granite	I
ZEV-01_SLK-19	ZEV-CH-01-23	2,247.00	2,248.00	1.00	73.545	73.495	55.405	73.265	79.4	77.4	73.160	67.4	71.6	0.3%	98.1	Granite	I
ZEV-01_SLK-20	ZEV-CH-01-23	2,275.25	2,276.25	1.00	70.235	70.175	52.825	70.000	79.4	77.4	69.870	67.4	71.6	0.3%	98.2	Granite	I
ZEV-01_SLK-21	ZEV-CH-01-23	2,319.25	2,320.33	1.08	68.900	68.885	52.975	68.035	79.4	77.4	67.615	67.4	71.6	0.1%	92.0	Granite	II
ZEV-01_SLK-22	ZEV-CH-01-23	2,350.50	2,351.66	1.16	71.400	71.345	53.415	71.240	67.5	70.2	71.195	67.8	71.0	0.3%	99.2	Granite	I
ZEV-01_SLK-23	ZEV-CH-01-23	2,360.08	2,360.75	0.67	70.190	70.135	53.405	70.035	67.5	70.2	69.990	67.8	71.0	0.3%	99.1	Granite	I
ZEV-01_SLK-24	ZEV-CH-01-23	2,514.67	2,516.00	1.33	70.515	70.375	52.690	70.265	67.5	70.2	70.215	67.8	71.0	0.8%	99.1	Granite	I
ZEV-01_SLK-25	ZEV-CH-01-23	2,560.00	2,561.00	1.00	71.685	71.525	53.650	69.030	68.5	69.8	68.040	68.1	70.6	0.9%	80.5	Granite	II
ZEV-01_SLK-26	ZEV-CH-01-23	2,567.33	2,568.00	0.67	73.180	73.125	54.920	72.790	68.5	69.8	72.720	68.1	70.6	0.3%	97.8	Granite	I
ZEV-01_SLK-27	ZEV-CH-01-23	2,635.25	2,636.50	1.25	74.045	73.930	55.320	73.820	68.5	69.8	73.775	68.1	70.6	0.6%	99.2	Granite	I
ZEV-01_SLK-28	ZEV-CH-01-23	2,666.00	2,667.00	1.00	72.895	72.855	55.415	72.680	79.0	76.0	72.610	70.2	71.1	0.2%	98.6	Granite	I
ZEV-01_SLK-29	ZEV-CH-01-23	2,682.00	2,683.70	1.70	70.070	70.000	52.820	69.365	79.0	76.0	69.110	70.2	71.1	0.4%	94.8	Granite	I
ZEV-01_SLK-30	ZEV-CH-01-23	2,737.00	2,739.00	2.00	71.760	71.735	53.000	71.570	79.0	76.0	71.500	70.2	71.1	0.1%	98.7	Granite	I
ZEV-01_SLK-31	ZEV-CH-01-23	2,772.40	2,773.00	0.60	71.140	71.090	53.415	71.030	71.0	74.3	71.000	69.8	73.8	0.3%	99.5	Granite	I
ZEV-01_SLK-32	ZEV-CH-01-23	2,825.20	2,825.80	0.60	71.205	71.145	53.415	70.965	71.0	74.3	70.900	69.8	73.8	0.3%	98.6	Granite	I
ZEV-01_SLK-33	ZEV-CH-01-23	2,891.45	2,892.00	0.55	68.770	68.630	52.670	67.145	71.0	74.3	66.040	69.8	73.8	0.9%	83.8	Granite	II
ZEV-01_SLK-34	ZEV-CH-01-23	2,903.30	2,903.70	0.40	69.945	69.905	53.665	69.730	73.7	74.8	69.630	73.6	74.4	0.2%	98.3	Granite	I
ZEV-01_SLK-35	ZEV-CH-01-23	2,935.70	2,938.50	2.80	72.110	72.020	54.925	71.340	73.7	74.8	70.680	73.6	74.4	0.5%	92.2	Granite	II

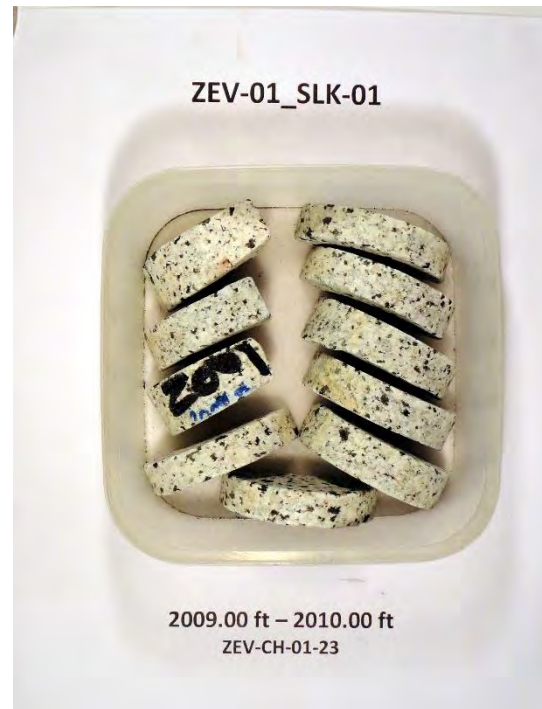
* Type I = Retained pieces remained virtually unchanged
II = Retained materials consist of large and small pieces
III = Retained materials is exclusively small fragments

APPENDIX J

BEFORE AND AFTER PHOTOGRAPHS OF SLAKE DURABILITY TEST SPECIMENS



SLK-01—Before



SLK-01—After



SLK-02—Before

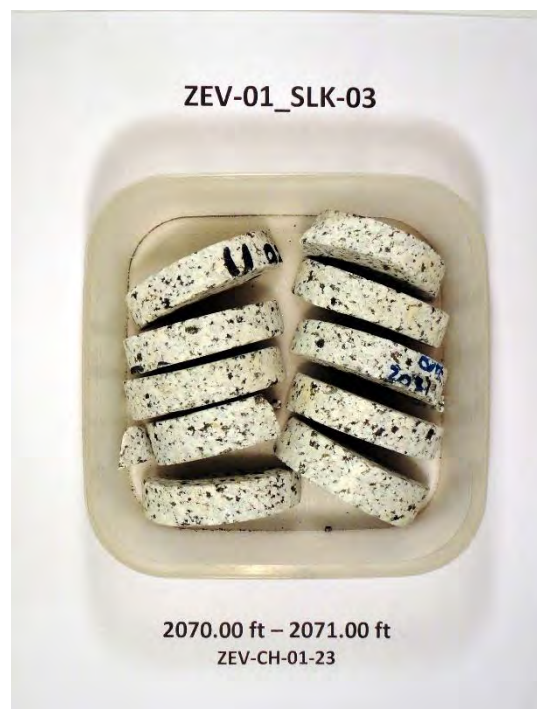


SLK-02—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens



SLK-03—Before



SLK-03—After



SLK-04—Before



SLK-04—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



SLK-05—Before



SLK-05—After



SLK-06—Before



SLK-06—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens (continued)



SLK-07—Before



SLK-07—After



SLK-08—Before

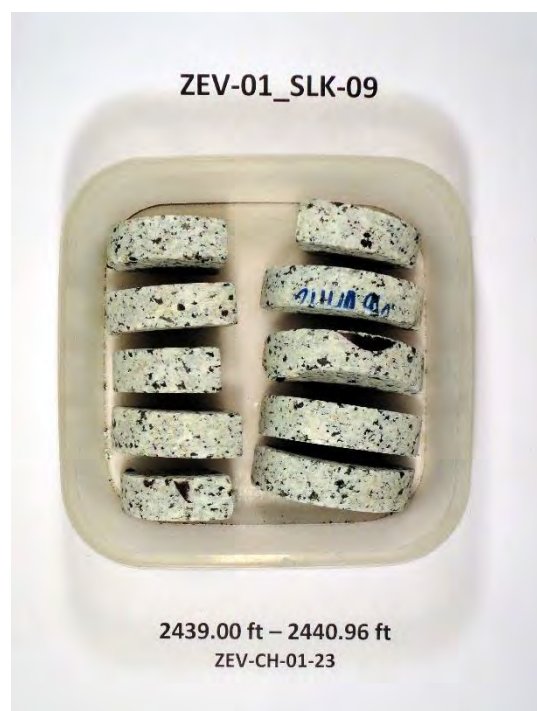


SLK-08—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



SLK-09—Before



SLK-09—After



SLK-10—Before



SLK-10—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



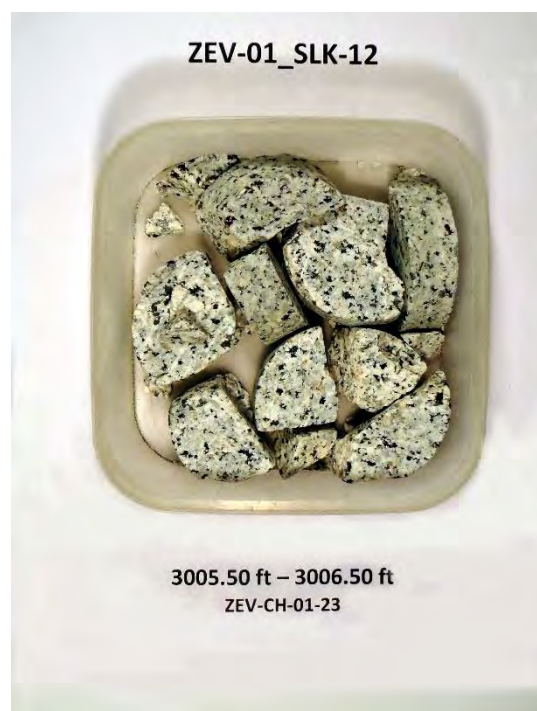
SLK-11—Before



SLK-11—After



SLK-12—Before



SLK-12—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



ZEV-01_SLK-13—Before



ZEV-01_SLK-13—After



ZEV-01_SLK-14—Before



ZEV-01_SLK-14—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



ZEV-01_SLK-15—Before



ZEV-01_SLK-15—After



ZEV-01_SLK-16—Before



ZEV-01_SLK-16—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens (*continued*)



ZEV-01_SLK-17—Before



ZEV-01_SLK-17—After



ZEV-01_SLK-18—Before



ZEV-01_SLK-18—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



ZEV-01_SLK-19—Before



ZEV-01_SLK-19—After



ZEV-01_SLK-20—Before



ZEV-01_SLK-20—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



ZEV-01_SLK-21—Before



ZEV-01_SLK-21—After



ZEV-01_SLK-22—Before



ZEV-01_SLK-22—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



ZEV-01_SLK-23—Before



ZEV-01_SLK-23—After



ZEV-01_SLK-24—Before



ZEV-01_SLK-24—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



ZEV-01_SLK-25—Before



ZEV-01_SLK-25—After



ZEV-01_SLK-26—Before



ZEV-01_SLK-26—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



ZEV-01_SLK-27—Before



ZEV-01_SLK-27—After



ZEV-01_SLK-28—Before



ZEV-01_SLK-28—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens (continued)



ZEV-01_SLK-29—Before



ZEV-01_SLK-29—After



ZEV-01_SLK-30—Before



ZEV-01_SLK-30—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens (continued)



ZEV-01_SLK-31—Before



ZEV-01_SLK-31—After



ZEV-01_SLK-32—Before



ZEV-01_SLK-32—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



ZEV-01_SLK-33—Before



ZEV-01_SLK-33—After



ZEV-01_SLK-34—Before



ZEV-01_SLK-34—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens (continued)



ZEV-01_SLK-35—Before



ZEV-01_SLK-35—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens (*concluded*)

**ROCK MECHANICS TESTING RESULTS FOR
BOREHOLE ZEV-CH-03-23**

Prepared for

LANE POWER AND ENERGY SOLUTIONS, INC.

December 11, 2023

Prepared by



AGAPITO ASSOCIATES, INC.

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ROCK MECHANICS TESTING RESULTS FOR BOREHOLE ZEV-CH-03-23

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DISCLAIMER OF LIABILITY: This work was prepared based on the core samples received and by carefully following the standards and procedures listed in this report. Neither Agapito Associates, Inc. (Agapito) nor any of its employees make any warranty, expressed or implied, or assumes **any** legal liability or responsibility for its application or usage. The user hereby acknowledges that the provisions of this disclaimer shall apply to all contents of this report.

1.0 INTRODUCTION

Lane Power and Energy Solutions, Inc. (Lane) commissioned Agapito Associates, Inc. (Agapito) to complete a rock mechanics laboratory study to determine the mechanical properties of rock samples from core hole ZEV-CH-03-23, originating from the Zevzar site near Rosamond, California. Shipments of core were delivered to Agapito's Grand Junction, Colorado laboratory from September 2023 to October 2023. This laboratory report provides results of the rock mechanics core testing performed by Agapito on behalf of Lane.

2.0 LABORATORY PROCEDURES

Prior to testing, specimens were prepared according to ASTM International (ASTM) standard D4543-08.¹ The following test types were performed by Agapito according to ASTM standards where applicable:

- Uniaxial (unconfined) Compressive Strength (UCS) test: D7012-13²
- Triaxial compression test: D7012-13³
- Splitting tensile strength test (Brazilian): D3967-08⁴
- Point Load Strength (Point Load) test: D5731-08⁵
- Slake Durability test: D4644-08⁶

3.0 TEST RESULTS

The laboratory results for the UCS tests are summarized in Table 1. Triaxial Compression test results are summarized in Table 2. Brazilian test results are presented in Table 3. Point Load results are presented in Table 4 and Slake Durability test results are presented in Table 5. Point Load testing has been shown to provide an approximation of compressive strength, and using a conversion factor of 21.0 to convert the Corrected Point Load Index ($I_{s(50)}$) into approximate UCS provides the best approximation over a variety of rock types.⁷

Elastic properties are normally calculated from sample data collected between 0% and 55% of the ultimate strength of a sample, where most competent rock experiences its most-linear zone of elastic deformation. For some samples the zone of elastic deformation occurred at lower than

¹ ASTM, "Standard Practice for Preparing Rock Core as Cylindrical Test Specimens and Verifying Conformance to Dimensional Shape Tolerances," Designation D4543-08.

² ASTM, "Standard Test Method for Compressive Strength and Elastic Moduli of Intact Rock Core Specimens under Varying States of Stress and Temperature," Designation D7012-13 (Methods C and D).

³ ASTM, "Standard Test Method for Compressive Strength and Elastic Moduli of Intact Rock Core Specimens under Varying States of Stress and Temperature," Designation D7012-13 (Methods A and B).

⁴ American Society for Testing and Materials (ASTM), "Standard Test Method for Splitting Tensile Strength of Intact Rock Core Specimens," Designation D3967-08.

⁵ ASTM, "Standard Test Method for Determination of the Point Load Strength Index of Rock and Application to Rock Strength Classification," Designation D5731-08.

⁶ ASTM, "Standard Test Method for Slake Durability of Shales and Similar Weak Rocks," Designation D4644-08.

⁷ Rusnak, J. and Mark, C. (2000), "Using the Point Load Test to Determine the Uniaxial Compressive Strength of Coal Measure Rock," *19th Ground Control Conference in Mining*, West Virginia University, pp. 362–371.

expected of percentages of ultimate strength. Some samples did not enter elastic deformation for a sufficient period to collect elastic properties. Elastic properties, therefore, were calculated individually based on data from zones of elastic deformation present for each specimen rather than the standard 0–55% of ultimate strength. Table 6 provides a summary of samples whose elastic properties were calculated using nonstandard data ranges.

The datasheet for the UCS tests with failure mode descriptions is presented in Appendix A. Stress-strain plots for UCS tests are in Appendix B. The before and after photographs of UCS test specimens are displayed in Appendix C. The datasheet for the Triaxial Compression tests with failure mode descriptions is presented in Appendix D, and the before and after photographs of Triaxial Compression test specimens are presented in Appendix E. The Brazilian test datasheet is presented in Appendix F, and after photographs of the Brazilian test specimens are presented in Appendix G. The Point Load test datasheet is presented in Appendix H. The Slake Durability test datasheet is presented in Appendix I, and the before and after photographs of the Slake Durability test specimens are presented in Appendix J.

Table 1. Uniaxial Compressive Strength Test Results

Specimen No.	Hole ID	Depth		Rock Type	Weight (oz)	Average Diameter (inch)	Axial Length (inch)	Density (pcf)	Failure Load (lb)	UCS (psi)	Young's Modulus†	Poisson's Ratio†	Young's Modulus‡	Poisson's Ratio‡
		From (ft)	To (ft)								(×10 ⁶ psi)		(×10 ⁶ psi)	
ZEV-03 UCS-01	ZEV-CH-03-23	1588.91	1589.75	Granite	37.22	2.38	5.36	169	111,045	24,992	10.75	0.32	11.06	0.15
ZEV-03 UCS-02	ZEV-CH-03-23	1632.00	1633.00	Granite	37.04	2.37	5.38	168	67,255	15,188	9.28	0.19	8.39	0.07
ZEV-03 UCS-03	ZEV-CH-03-23	1721.41	1722.00	Granite	36.72	2.37	5.41	167	65,970	15,017	9.12	0.20	9.05	0.07
ZEV-03 UCS-04	ZEV-CH-03-23	1832.66	1833.41	Granite	38.12	2.39	5.49	167	66,370	14,797	8.76	0.22	8.06	0.07
ZEV-03 UCS-05	ZEV-CH-03-23	1909.70	1910.50	Granite	36.07	2.36	5.33	168	34,525	7,909	6.88	0.33	6.67	0.15
ZEV-03 UCS-06	ZEV-CH-03-23	2019.20	2019.90	Granite	36.93	2.39	5.35	166	91,715	20,362	10.70	0.27	10.64	0.11
ZEV-03 UCS-07	ZEV-CH-03-23	2076.00	2077.00	Granite	36.84	2.40	5.26	168	29,960	6,646	8.31	0.38	8.03	0.09
ZEV-03 UCS-08	ZEV-CH-03-23	2103.00	2104.00	Granite	40.08	2.40	5.72	167	6,675	1,475	1.24	-	1.22	-
ZEV-03 UCS-09	ZEV-CH-03-23	2112.00	2113.00	Granite	38.01	2.39	5.41	168	88,020	19,550	10.04	0.30	9.77	0.17
ZEV-03 UCS-10	ZEV-CH-03-23	2190.00	2191.00	Granite	35.97	2.40	5.38	160	16,180	3,583	1.83	0.29	1.63	0.13
ZEV-03 UCS-11	ZEV-CH-03-23	2212.40	2213.50	Granite	38.51	2.37	5.58	168	71,900	16,257	9.40	-	8.62	-
ZEV-03 UCS-12	ZEV-CH-03-23	2224.00	2224.65	Granite	37.38	2.40	5.38	166	83,850	18,589	11.43	0.19	11.53	0.09
ZEV-03 UCS-13	ZEV-CH-03-23	2254.20	2255.50	Granite	38.27	2.40	5.41	169	72,725	16,103	9.35	0.17	8.59	0.11
ZEV-03 UCS-14	ZEV-CH-03-23	2279.00	2280.00	Granite	38.06	2.40	5.40	169	87,880	19,470	10.47	0.17	9.86	0.07
ZEV-03 UCS-15	ZEV-CH-03-23	2308.20	2309.40	Granite	38.46	2.39	5.46	169	67,690	15,069	9.78	0.20	9.20	0.10
ZEV-03 UCS-16	ZEV-CH-03-23	2321.00	2322.00	Granite	38.00	2.39	5.39	170	114,705	25,541	11.10	0.23	9.96	0.11
ZEV-03 UCS-17	ZEV-CH-03-23	2352.50	2354.00	Granite	38.34	2.39	5.46	169	81,550	18,181	10.66	0.18	10.73	0.08
ZEV-03 UCS-18	ZEV-CH-03-23	2384.50	2386.00	Granite	38.46	2.39	5.46	169	80,690	17,945	9.84	0.24	8.76	0.08
ZEV-03 UCS-19	ZEV-CH-03-23	2412.40	2414.00	Granite	38.73	2.39	5.52	168	70,480	15,661	9.72	0.25	9.03	0.08
ZEV-03 UCS-20	ZEV-CH-03-23	2444.00	2445.50	Granite	37.27	2.39	5.32	169	62,820	14,006	10.25	0.19	8.28	0.07
ZEV-03 UCS-21	ZEV-CH-03-23	2461.00	2462.00	Granite	35.33	2.39	5.10	167	32,140	7,179	-	-	-	-
ZEV-03 UCS-22	ZEV-CH-03-23	2527.33	2528.00	Granite	37.87	2.39	5.46	167	41,340	9,224	6.74	-	6.43	-
ZEV-03 UCS-23	ZEV-CH-03-23	2576.00	2576.91	Granite	38.24	2.40	5.41	169	87,240	19,333	10.91	0.24	10.82	0.13
ZEV-03 UCS-24	ZEV-CH-03-23	2603.50	2604.00	Granite	38.24	2.41	5.52	165	15,875	3,493	1.99	-	1.32	-
ZEV-03 UCS-25	ZEV-CH-03-23	2648.00	2648.66	Granite	37.57	2.40	5.34	168	102,825	22,748	9.97	0.29	8.72	0.14
ZEV-03 UCS-26	ZEV-CH-03-23	2677.58	2678.41	Granite	37.18	2.38	5.36	169	98,115	22,096	10.27	0.33	9.87	0.16
ZEV-03 UCS-27	ZEV-CH-03-23	2701.00	2701.50	Granite	35.48	2.38	5.84	148	4,510	1,014	0.26	0.32	0.21	0.12
ZEV-03 UCS-28	ZEV-CH-03-23	2724.75	2725.16	Granite	35.98	2.37	5.18	170	29,740	6,744	7.45	0.18	8.24	0.08
ZEV-03 UCS-29	ZEV-CH-03-23	2740.00	2740.50	Granite	37.10	2.40	5.28	168	37,520	8,318	6.05	-	6.10	-
ZEV-03 UCS-30	ZEV-CH-03-23	2876.16	2876.91	Granite	38.49	2.40	5.85	157	6,100	1,345	0.33	-	0.20	-
ZEV-03 UCS-31	ZEV-CH-03-23	2879.00	2880.00	Granite	38.56	2.40	5.89	157	6,430	1,426	0.51	-	0.32	-
ZEV-03 UCS-32	ZEV-CH-03-23	2943.00	2943.50	Granite	37.33	2.39	5.36	167	42,175	9,375	6.43	-	4.53	-
ZEV-03 UCS-33	ZEV-CH-03-23	2983.66	2984.16	Granite	37.01	2.40	5.36	164	71,520	15,773	10.09	-	8.03	-
†Tangent calculation method.														
‡Secant calculation method.														

Table 2. Triaxial Compression Test Results

Specimen No.	Hole ID	Depth		Lithology	Weight (oz)	Average Diameter (inch)	Axial Length (inch)	Density (pcf)	Failure Load (lbs)	Confining Pressure (psi)	Corrected Failure Stress (psi)	Triaxial Compressive Strength (psi)
		From (ft)	To (ft)									
ZEV-03 TRI-01	ZEV-CH-03-23	1,898.70	1,899.70	Granite	36.9	2.38	5.35	168	120,410	1,500	27,500	26,000
ZEV-03 TRI-02	ZEV-CH-03-23	1,901.20	1,902.00	Granite	36.3	2.38	5.24	169	202,450	2,500	46,214	43,714
ZEV-03 TRI-03	ZEV-CH-03-23	1,902.00	1,902.80	Granite	34.9	2.37	5.04	169	153,620	1,500	35,025	33,525
ZEV-03 TRI-04	ZEV-CH-03-23	2,246.00	2,247.70	Granite	36.1	2.36	5.33	164	227,815	2,500	52,732	50,232
ZEV-03 TRI-05	ZEV-CH-03-23	2,246.00	2,247.70	Granite	34.9	2.40	5.08	164	213,135	1,500	47,575	46,075
ZEV-03 TRI-06	ZEV-CH-03-23	2,246.00	2,247.70	Granite	36.1	2.40	5.24	165	269,100	2,500	60,263	57,763
ZEV-03 TRI-07	ZEV-CH-03-23	2,468.70	2,470.00	Granite	34.1	2.39	4.97	165	135,535	1,500	30,592	29,092
ZEV-03 TRI-08	ZEV-CH-03-23	2,468.70	2,470.00	Granite	34.6	2.39	4.99	167	167,430	2,500	37,908	35,408
ZEV-03 TRI-09	ZEV-CH-03-23	2,468.70	2,470.00	Granite	33.9	2.39	5.00	164	144,955	1,500	32,741	31,241
ZEV-03 TRI-10	ZEV-CH-03-23	2,689.00	2,689.75	Granite	37.9	2.39	5.39	169	170,725	2,500	38,687	36,187
ZEV-03 TRI-11	ZEV-CH-03-23	2,684.00	2,685.00	Granite	37.3	2.39	5.38	167	133,270	1,500	30,117	28,617
ZEV-03 TRI-12	ZEV-CH-03-23	2,694.75	2,695.25	Granite	38.1	2.39	5.44	168	133,840	2,500	30,422	27,922
ZEV-03 TRI-13	ZEV-CH-03-23	2,722.50	2,723.00	Granite	41.1	2.41	5.82	167	-	-	-	-
ZEV-03 TRI-14	ZEV-CH-03-23	2,723.33	2,724.16	Granite	37.1	2.40	5.29	168	77,475	2,500	17,746	15,246
ZEV-03 TRI-15	ZEV-CH-03-23	2,723.33	2,724.16	Granite	36.6	2.40	5.19	169	109,320	1,500	24,551	23,051
ZEV-03 TRI-16	ZEV-CH-03-23	2,777.66	2,778.33	Granite	33.4	2.40	5.19	153	12,370	2,500	3,344	844

Table 3. Indirect Tensile Strength (Brazilian) Test Results

Specimen No.	Hole ID	Depth		Rock Type	Weight (oz)	Average Diameter (inch)	Axial Length (inch)	Density (pcf)	Failure Load (lb)	Splitting Tensile Strength (psi)
		From (ft)	To (ft)							
ZEV-03_BTS-01	ZEV-CH-03-23	1561.50	1562.50	Granite	9.09	2.39	1.33	165	6,815	1,366
ZEV-03_BTS-02	ZEV-CH-03-23	1633.08	1634.41	Granite	9.06	2.37	1.32	167	6,602	1,338
ZEV-03_BTS-03	ZEV-CH-03-23	1722.08	1723.16	Granite	8.81	2.36	1.33	164	4,563	927
ZEV-03_BTS-04	ZEV-CH-03-23	1878.70	1879.60	Granite	9.11	2.38	1.35	164	1,935	384
ZEV-03_BTS-05	ZEV-CH-03-23	1913.80	1914.60	Granite	8.94	2.37	1.31	168	5,655	1,163
ZEV-03_BTS-06	ZEV-CH-03-23	2019.90	2020.40	Granite	8.46	2.40	1.21	168	4,677	1,028
ZEV-03_BTS-07	ZEV-CH-03-23	2074.60	2076.00	Granite	8.92	2.40	1.28	166	5,449	1,127
ZEV-03_BTS-08	ZEV-CH-03-23	2092.70	2093.80	Granite	9.38	2.40	1.34	168	5,651	1,122
ZEV-03_BTS-09	ZEV-CH-03-23	2111.40	2112.00	Granite	8.32	2.40	1.19	167	5,653	1,258
ZEV-03_BTS-10	ZEV-CH-03-23	2175.20	2176.00	Granite	8.89	2.40	1.28	166	2,894	600
ZEV-03_BTS-11	ZEV-CH-03-23	2193.50	2195.00	Granite	8.63	2.40	1.27	162	2,292	478
ZEV-03_BTS-12	ZEV-CH-03-23	2218.00	2219.00	Granite	8.55	2.39	1.25	164	6,432	1,370
ZEV-03_BTS-13	ZEV-CH-03-23	2254.20	2255.50	Granite	8.79	2.39	1.28	165	5,632	1,172
ZEV-03_BTS-14	ZEV-CH-03-23	2276.40	2277.00	Granite	8.73	2.40	1.25	167	3,843	816
ZEV-03_BTS-15	ZEV-CH-03-23	2306.60	2308.20	Granite	7.86	2.39	1.15	165	5,999	1,393
ZEV-03_BTS-16	ZEV-CH-03-23	2321.00	2322.00	Granite	8.54	2.39	1.22	168	5,475	1,194
ZEV-03_BTS-17	ZEV-CH-03-23	2352.50	2354.00	Granite	8.99	2.39	1.30	167	6,151	1,265
ZEV-03_BTS-18	ZEV-CH-03-23	2384.50	2386.00	Granite	9.10	2.39	1.30	168	7,440	1,521
ZEV-03_BTS-19	ZEV-CH-03-23	2412.40	2414.00	Granite	8.10	2.39	1.17	166	2,157	491
ZEV-03_BTS-20	ZEV-CH-03-23	2438.40	2440.60	Granite	8.82	2.38	1.30	164	6,288	1,293
ZEV-03_BTS-21	ZEV-CH-03-23	2482.83	2483.25	Granite	9.20	2.39	1.35	164	-	-
ZEV-03_BTS-22	ZEV-CH-03-23	2526.83	2527.25	Granite	8.99	2.39	1.30	166	5,114	1,047
ZEV-03_BTS-23	ZEV-CH-03-23	2559.83	2560.00	Granite	9.64	2.40	1.38	167	8,503	1,640
ZEV-03_BTS-24	ZEV-CH-03-23	2578.83	2579.58	Granite	9.19	2.40	1.31	168	6,083	1,232
ZEV-03_BTS-25	ZEV-CH-03-23	2654.00	2654.50	Granite	8.91	2.39	1.28	167	7,396	1,535
ZEV-03_BTS-26	ZEV-CH-03-23	2677.58	2678.41	Granite	8.80	2.38	1.30	165	6,800	1,405
ZEV-03_BTS-27	ZEV-CH-03-23	2707.00	2707.50	Granite	8.52	2.38	1.26	164	6,987	1,481
ZEV-03_BTS-28	ZEV-CH-03-23	2740.00	2740.50	Granite	8.96	2.40	1.30	165	5,150	1,053
ZEV-03_BTS-29	ZEV-CH-03-23	2759.91	2760.33	Granite	8.82	2.40	1.29	163	2,639	542
ZEV-03_BTS-30	ZEV-CH-03-23	2939.58	2940.00	Granite	8.63	2.38	1.26	165	3,887	822
ZEV-03_BTS-31	ZEV-CH-03-23	2946.50	2947.00	Granite	9.39	2.40	1.35	167	7,537	1,485

Table 4. Point Load Test Results

Specimen No.	Hole ID	From (ft)	To (ft)	Lithology	"Failure Load" P (lb)	Test Orientation (A/D)	Corrected Point Load Index, I _{S(50)} (psi)	Estimated UCS (psi)
ZEV-03_PLT-01	ZEV-CH-03-23	1,539.25	1,540.50	Granite	340	A	104	2,175
ZEV-03_PLT-02	ZEV-CH-03-23	1,576.00	1,577.00	Granite	4,463	A	1,288	27,051
ZEV-03_PLT-03	ZEV-CH-03-23	1,598.75	1,599.66	Granite	3,043	A	824	17,300
ZEV-03_PLT-04	ZEV-CH-03-23	1,616.33	1,617.00	Granite	4,170	A	1,140	23,946
ZEV-03_PLT-05	ZEV-CH-03-23	1,617.08	1,618.00	Granite	3,946	A	1,093	22,956
ZEV-03_PLT-06	ZEV-CH-03-23	1,618.18	1,619.00	Granite	1,808	A	488	10,256
ZEV-03_PLT-07	ZEV-CH-03-23	1,701.25	1,701.75	Granite	4,657	A	1,282	26,928
ZEV-03_PLT-08	ZEV-CH-03-23	1,701.83	1,702.33	Granite	2,502	A	717	15,056
ZEV-03_PLT-09	ZEV-CH-03-23	1,778.16	1,779.00	Granite	3,251	A	900	18,906
ZEV-03_PLT-10	ZEV-CH-03-23	1,851.00	1,851.70	Granite	1,602	A	438	9,194
ZEV-03_PLT-11	ZEV-CH-03-23	1,869.30	1,869.70	Granite	3,344	A	914	19,201
ZEV-03_PLT-12	ZEV-CH-03-23	1,869.70	1,870.10	Granite	3,261	A	880	18,484
ZEV-03_PLT-13	ZEV-CH-03-23	1,921.60	1,922.00	Granite	3,979	A	1,123	23,586
ZEV-03_PLT-14	ZEV-CH-03-23	1,922.00	1,922.40	Granite	5,238	A	1,459	30,638
ZEV-03_PLT-15	ZEV-CH-03-23	1,922.40	1,922.80	Granite	4,451	A	1,179	24,755
ZEV-03_PLT-16	ZEV-CH-03-23	2,020.40	2,021.00	Granite	3,835	A	1,034	21,717
ZEV-03_PLT-17	ZEV-CH-03-23	2,077.00	2,077.60	Granite	3,797	A	1,050	22,056
ZEV-03_PLT-18	ZEV-CH-03-23	2,105.50	2,106.00	Granite	2,306	A	621	13,041
ZEV-03_PLT-19	ZEV-CH-03-23	2,111.40	2,112.00	Granite	3,168	A	912	19,145
ZEV-03_PLT-20	ZEV-CH-03-23	2,175.20	2,176.00	Granite	1,278	A	349	7,322
ZEV-03_PLT-21	ZEV-CH-03-23	2,196.00	2,196.60	Granite	688	A	190	3,999
ZEV-03_PLT-22	ZEV-CH-03-23	2,218.00	2,219.00	Granite	3,976	A	1,086	22,816
ZEV-03_PLT-23	ZEV-CH-03-23	2,237.00	2,237.70	Granite	5,203	A	1,385	29,088
ZEV-03_PLT-24	ZEV-CH-03-23	2,254.20	2,255.50	Granite	2,866	A	773	16,231
ZEV-03_PLT-25	ZEV-CH-03-23	2,271.60	2,272.65	Granite	1,094	A	299	6,275
ZEV-03_PLT-26	ZEV-CH-03-23	2,306.60	2,308.20	Granite	2,474	A	695	14,585
ZEV-03_PLT-27	ZEV-CH-03-23	2,321.00	2,322.00	Granite	3,974	A	1,116	23,433
ZEV-03_PLT-28	ZEV-CH-03-23	2,352.50	2,354.00	Granite	3,707	A	1,014	21,295
ZEV-03_PLT-29	ZEV-CH-03-23	2,389.20	2,390.00	Granite	4,371	A	1,180	24,770
ZEV-03_PLT-30	ZEV-CH-03-23	2,417.00	2,418.50	Granite	3,674	A	1,013	21,271
ZEV-03_PLT-31	ZEV-CH-03-23	2,451.60	2,453.00	Granite	614	A	161	3,391
ZEV-03_PLT-32	ZEV-CH-03-23	2,478.00	2,478.40	Granite	4,633	A	1,220	25,618
ZEV-03_PLT-33	ZEV-CH-03-23	2,533.41	2,533.83	Granite	1,160	A	301	6,323
ZEV-03_PLT-34	ZEV-CH-03-23	2,577.83	2,578.33	Granite	5,233	A	1,342	28,174
ZEV-03_PLT-35	ZEV-CH-03-23	2,602.75	2,603.33	Granite	815	A	214	4,488
ZEV-03_PLT-36	ZEV-CH-03-23	2,631.83	2,632.33	Granite	1,637	A	441	9,261
ZEV-03_PLT-37	ZEV-CH-03-23	2,654.66	2,655.25	Granite	3,424	A	913	19,171
ZEV-03_PLT-38	ZEV-CH-03-23	2,684.00	2,685.00	Granite	2,422	A	655	13,760
ZEV-03_PLT-39	ZEV-CH-03-23	2,721.16	2,721.66	Granite	4,175	A	1,170	24,567
ZEV-03_PLT-40	ZEV-CH-03-23	2,751.75	2,752.25	Granite	295	A	75	1,569
ZEV-03_PLT-41	ZEV-CH-03-23	2,835.58	2,836.00	Granite	3,979	A	1,033	21,688
ZEV-03_PLT-42	ZEV-CH-03-23	2,853.00	2,853.58	Granite	3,154	A	828	17,398
ZEV-03_PLT-43	ZEV-CH-03-23	2,879.00	2,880.00	Granite	144	A	35	740
ZEV-03_PLT-44	ZEV-CH-03-23	2,939.58	2,940.00	Granite	2,115	A	565	11,870
ZEV-03_PLT-45	ZEV-CH-03-23	2,949.25	2,949.50	Granite	1,144	A	308	6,475

Table 5. Slake Durability Test Results

Specimen ID	Hole ID	Sample Depth		Lithology	Slake Durability		
		From (ft)	To (ft)		Moisture Content (%)	Index, I _d (2)	Retained Material Description (Type) *
ZEV-03 SLK-01	ZEV-CH-03-23	1897.70	1898.70	Granite	0.3%	99.7	I
ZEV-03 SLK-02	ZEV-CH-03-23	2021.70	2023.10	Granite	0.3%	99.7	I
ZEV-03 SLK-03	ZEV-CH-03-23	2074.60	2076.00	Granite	0.2%	99.6	I
ZEV-03 SLK-04	ZEV-CH-03-23	2092.70	2093.80	Granite	0.3%	99.5	I
ZEV-03 SLK-05	ZEV-CH-03-23	2112.00	2113.00	Granite	0.2%	99.7	I
ZEV-03 SLK-06	ZEV-CH-03-23	2151.00	2151.60	Granite	0.3%	99.4	I
ZEV-03 SLK-07	ZEV-CH-03-23	2187.00	2188.04	Granite	0.2%	93.4	II
ZEV-03 SLK-08	ZEV-CH-03-23	2245.00	2246.00	Granite	0.2%	99.7	I
ZEV-03 SLK-09	ZEV-CH-03-23	2277.50	2279.00	Granite	0.3%	99.0	I
ZEV-03 SLK-10	ZEV-CH-03-23	2306.60	2308.20	Granite	0.2%	99.6	I
ZEV-03 SLK-11	ZEV-CH-03-23	2335.00	2335.80	Granite	0.2%	99.2	I
ZEV-03 SLK-12	ZEV-CH-03-23	2359.70	2360.50	Granite	0.2%	99.4	I
ZEV-03 SLK-13	ZEV-CH-03-23	2389.20	2390.00	Granite	0.2%	99.7	I
ZEV-03 SLK-14	ZEV-CH-03-23	2417.00	2418.50	Granite	0.2%	99.7	I
ZEV-03 SLK-15	ZEV-CH-03-23	2451.60	2453.00	Granite	0.2%	98.9	I
ZEV-03 SLK-16	ZEV-CH-03-23	2486.00	2487.00	Granite	0.3%	99.1	I
ZEV-03 SLK-17	ZEV-CH-03-23	2532.00	2532.75	Granite	0.6%	94.4	II
ZEV-03 SLK-18	ZEV-CH-03-23	2562.83	2563.58	Granite	0.2%	99.3	I
ZEV-03 SLK-19	ZEV-CH-03-23	2604.50	2605.41	Granite	0.6%	90.9	II
ZEV-03 SLK-20	ZEV-CH-03-23	2648.00	2648.66	Granite	0.4%	97.9	I
ZEV-03 SLK-21	ZEV-CH-03-23	2674.50	2675.50	Granite	0.1%	99.6	I
ZEV-03 SLK-22	ZEV-CH-03-23	2717.08	2717.58	Granite	0.2%	99.5	I
ZEV-03 SLK-23	ZEV-CH-03-23	2757.00	2758.00	Granite	0.3%	97.5	I
ZEV-03 SLK-24	ZEV-CH-03-23	2798.58	2799.58	Granite	0.7%	92.8	II
ZEV-03 SLK-25	ZEV-CH-03-23	2867.00	2867.58	Granite	0.7%	44.6	II
ZEV-03 SLK-26	ZEV-CH-03-23	2917.77	2918.33	Granite	2.0%	23.9	III
ZEV-03 SLK-27	ZEV-CH-03-23	2983.66	2984.16	Granite	0.3%	99.0	I
* Type I = Retained pieces remained virtually unchanged II = Retained materials consist of large and small pieces III = Retained materials is exclusively small fragments							

Table 6. List of Samples with Modified Data Ranges Used for Elastic Property Calculation

			Range of data used for elastic properties calculation (percent of sample maximum strength)			
			Tangent Properties		Secant Properties	
Specimen No.	Hole ID	Test Type	Lower Limit	Upper Limit	Lower Limit	Upper Limit
Standard Procedure	-	-	45	55	0	50
ZEV-03 UCS-05	ZEV-CH-03-23	UCS	20	30	0	25
ZEV-03 UCS-06	ZEV-CH-03-23	UCS	43	53	0	48
ZEV-03 UCS-10	ZEV-CH-03-23	UCS	32	42	0	37
ZEV-03 UCS-13	ZEV-CH-03-23	UCS	30	40	0	35
ZEV-03 UCS-27	ZEV-CH-03-23	UCS	30	40	0	35
ZEV-03 UCS-32	ZEV-CH-03-23	UCS	40	50	0	45

APPENDIX A

UNIAXIAL COMPRESSIVE STRENGTH (UCS) TEST DATASHEET

UNIAXIAL COMPRESSION TESTS

CLIENT:

JOB NO:

DATE:

Lane PES

951-20

October 4, 2023

HOLE NO:

MOISTURE CONDITION:

TEMPERATURE:

ZEV-CH-03-23

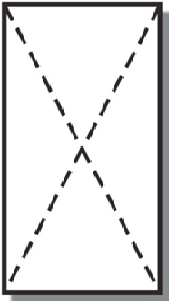
As received

Ambient

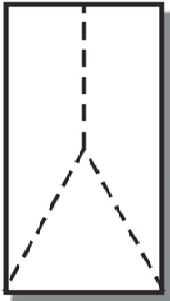
Specimen No.	Hole No.	Depth		Length of Interval (ft)	Weight (oz)	Diameter		Axial Length (inch)	Length-to-Diameter Ratio	Area (inch ²)	Density (pcf)	Specific Gravity
		From (ft)	To (ft)			D ₁ (in)	D ₂ (in)					
ZEV-03_UCS-01	ZEV-CH-03-23	1,588.91	1,589.75	0.84	37.22	2.38	2.38	5.36	2.3	4.44	169	2.7
ZEV-03_UCS-02	ZEV-CH-03-23	1,632.00	1,633.00	1.00	37.04	2.37	2.38	5.38	2.3	4.43	168	2.7
ZEV-03_UCS-03	ZEV-CH-03-23	1,721.41	1,722.00	0.59	36.72	2.36	2.37	5.41	2.3	4.39	167	2.7
ZEV-03_UCS-04	ZEV-CH-03-23	1,832.66	1,833.41	0.75	38.12	2.39	2.39	5.49	2.3	4.49	167	2.7
ZEV-03_UCS-05	ZEV-CH-03-23	1,909.70	1,910.50	0.80	36.07	2.36	2.35	5.33	2.3	4.37	168	2.7
ZEV-03_UCS-06	ZEV-CH-03-23	2,019.20	2,019.90	0.70	36.93	2.40	2.39	5.35	2.2	4.50	166	2.7
ZEV-03_UCS-07	ZEV-CH-03-23	2,076.00	2,077.00	1.00	36.84	2.40	2.40	5.26	2.2	4.51	168	2.7
ZEV-03_UCS-08	ZEV-CH-03-23	2,103.00	2,104.00	1.00	40.08	2.40	2.40	5.72	2.4	4.52	167	2.7
ZEV-03_UCS-09	ZEV-CH-03-23	2,112.00	2,113.00	1.00	38.01	2.39	2.39	5.41	2.3	4.50	168	2.7
ZEV-03_UCS-10	ZEV-CH-03-23	2,190.00	2,191.00	1.00	35.97	2.40	2.40	5.38	2.2	4.52	160	2.6
ZEV-03_UCS-11	ZEV-CH-03-23	2,212.40	2,213.50	1.10	38.51	2.37	2.37	5.58	2.4	4.42	168	2.7
ZEV-03_UCS-12	ZEV-CH-03-23	2,224.00	2,224.65	0.65	37.38	2.40	2.40	5.38	2.2	4.51	166	2.7
ZEV-03_UCS-13	ZEV-CH-03-23	2,254.20	2,255.50	1.30	38.27	2.40	2.40	5.41	2.3	4.52	169	2.7
ZEV-03_UCS-14	ZEV-CH-03-23	2,279.00	2,280.00	1.00	38.06	2.40	2.40	5.40	2.3	4.51	169	2.7
ZEV-03_UCS-15	ZEV-CH-03-23	2,308.20	2,309.40	1.20	38.46	2.39	2.39	5.46	2.3	4.49	169	2.7
ZEV-03_UCS-16	ZEV-CH-03-23	2,321.00	2,322.00	1.00	38.00	2.39	2.39	5.39	2.3	4.49	170	2.7
ZEV-03_UCS-17	ZEV-CH-03-23	2,352.50	2,354.00	1.50	38.34	2.39	2.39	5.46	2.3	4.49	169	2.7
ZEV-03_UCS-18	ZEV-CH-03-23	2,384.50	2,386.00	1.50	38.46	2.39	2.39	5.46	2.3	4.50	169	2.7
ZEV-03_UCS-19	ZEV-CH-03-23	2,412.40	2,414.00	1.60	38.73	2.39	2.39	5.52	2.3	4.50	168	2.7
ZEV-03_UCS-20	ZEV-CH-03-23	2,444.00	2,445.50	1.50	37.27	2.39	2.39	5.32	2.2	4.49	169	2.7
ZEV-03_UCS-21	ZEV-CH-03-23	2,461.00	2,462.00	1.00	35.33	2.39	2.39	5.10	2.1	4.48	167	2.7
ZEV-03_UCS-22	ZEV-CH-03-23	2,527.33	2,528.00	0.67	37.87	2.39	2.39	5.46	2.3	4.48	167	2.7
ZEV-03_UCS-23	ZEV-CH-03-23	2,576.00	2,576.91	0.91	38.24	2.40	2.40	5.41	2.3	4.51	169	2.7
ZEV-03_UCS-24	ZEV-CH-03-23	2,603.50	2,604.00	0.50	38.24	2.40	2.41	5.52	2.3	4.54	165	2.6
ZEV-03_UCS-25	ZEV-CH-03-23	2,648.00	2,648.66	0.66	37.57	2.40	2.40	5.34	2.2	4.52	168	2.7
ZEV-03_UCS-26	ZEV-CH-03-23	2,677.58	2,678.41	0.83	37.18	2.38	2.38	5.36	2.3	4.44	169	2.7
ZEV-03_UCS-27	ZEV-CH-03-23	2,701.00	2,701.50	0.50	35.48	2.38	2.38	5.84	2.5	4.45	148	2.4
ZEV-03_UCS-28	ZEV-CH-03-23	2,724.75	2,725.16	0.41	35.98	2.40	2.34	5.18	2.2	4.41	170	2.7
ZEV-03_UCS-29	ZEV-CH-03-23	2,740.00	2,740.50	0.50	37.10	2.40	2.40	5.28	2.2	4.51	168	2.7
ZEV-03_UCS-30	ZEV-CH-03-23	2,876.16	2,876.91	0.75	38.49	2.41	2.40	5.85	2.4	4.54	157	2.5
ZEV-03_UCS-31	ZEV-CH-03-23	2,879.00	2,880.00	1.00	38.56	2.40	2.39	5.89	2.5	4.51	157	2.5
ZEV-03_UCS-32	ZEV-CH-03-23	2,943.00	2,943.50	0.50	37.33	2.39	2.39	5.36	2.2	4.50	167	2.7
ZEV-03_UCS-33	ZEV-CH-03-23	2,983.66	2,984.16	0.50	37.01	2.40	2.40	5.36	2.2	4.53	164	2.6

Specimen No.	Lithological Description	Failure Load (lb)	UCS (psi)	Failure Mode Notes	Comments
ZEV-03_UCS-01	Granite	111,045	24,992	Axial	
ZEV-03_UCS-02	Granite	67,255	15,188	Shear	Shear along single pre-existing fracture
ZEV-03_UCS-03	Granite	65,970	15,017	Axial	failed partially along pre-existing fracture
ZEV-03_UCS-04	Granite	66,370	14,797	Shear	Along pre-existing fracture, single plane
ZEV-03_UCS-05	Granite	34,525	7,909	Axial	failed partially along pre-existing fracture
ZEV-03_UCS-06	Granite	91,715	20,362	Axial	
ZEV-03_UCS-07	Granite	29,960	6,646	Shear	Failed partially along pre-existing fractures
ZEV-03_UCS-08	Granite	6,675	1,475	Shear	Failed along pre-existing fracture
ZEV-03_UCS-09	Granite	88,020	19,550	Axial	
ZEV-03_UCS-10	Granite	16,180	3,583	Axial	
ZEV-03_UCS-11	Granite	71,900	16,257	Axial	
ZEV-03_UCS-12	Granite	83,850	18,589	Axial/Shear	
ZEV-03_UCS-13	Granite	72,725	16,103	Axial	
ZEV-03_UCS-14	Granite	87,880	19,470	Shear	
ZEV-03_UCS-15	Granite	67,690	15,069	Axial/Shear	Partially along pre-existing fracture.
ZEV-03_UCS-16	Granite	114,705	25,541	Axial	Multiple fracturing.
ZEV-03_UCS-17	Granite	81,550	18,181	Axial/Shear	
ZEV-03_UCS-18	Granite	80,690	17,945	Shear	
ZEV-03_UCS-19	Granite	70,480	15,661	Axial	Failed partially along pre-existing fractures
ZEV-03_UCS-20	Granite	62,820	14,006	Axial	Failed partially along pre-existing fractures
ZEV-03_UCS-21	Granite	32,140	7,179	Axial	Failed mostly along pre-existing fractures
ZEV-03_UCS-22	Granite	41,340	9,224	Axial	Prolonged failure
ZEV-03_UCS-23	Granite	87,240	19,333	Axial	
ZEV-03_UCS-24	Granite	15,875	3,493	Axial	
ZEV-03_UCS-25	Granite	102,825	22,748	Shear	
ZEV-03_UCS-26	Granite	98,115	22,096	Axial	
ZEV-03_UCS-27	Granite	4,510	1,014	Shear	Prolonged failure
ZEV-03_UCS-28	Granite	29,740	6,744	Axial	
ZEV-03_UCS-29	Granite	37,520	8,318	Axial	
ZEV-03_UCS-30	Granite	6,100	1,345	Axial	Prolonged failure
ZEV-03_UCS-31	Granite	6,430	1,426	Axial	Prolonged failure
ZEV-03_UCS-32	Granite	42,175	9,375	Axial	Failed partially along pre-existing fractures
ZEV-03_UCS-33	Granite	71,520	15,773	Axial	

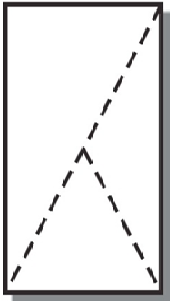
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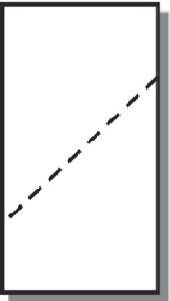
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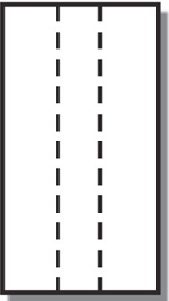
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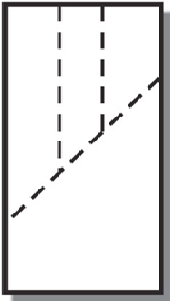
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Shear



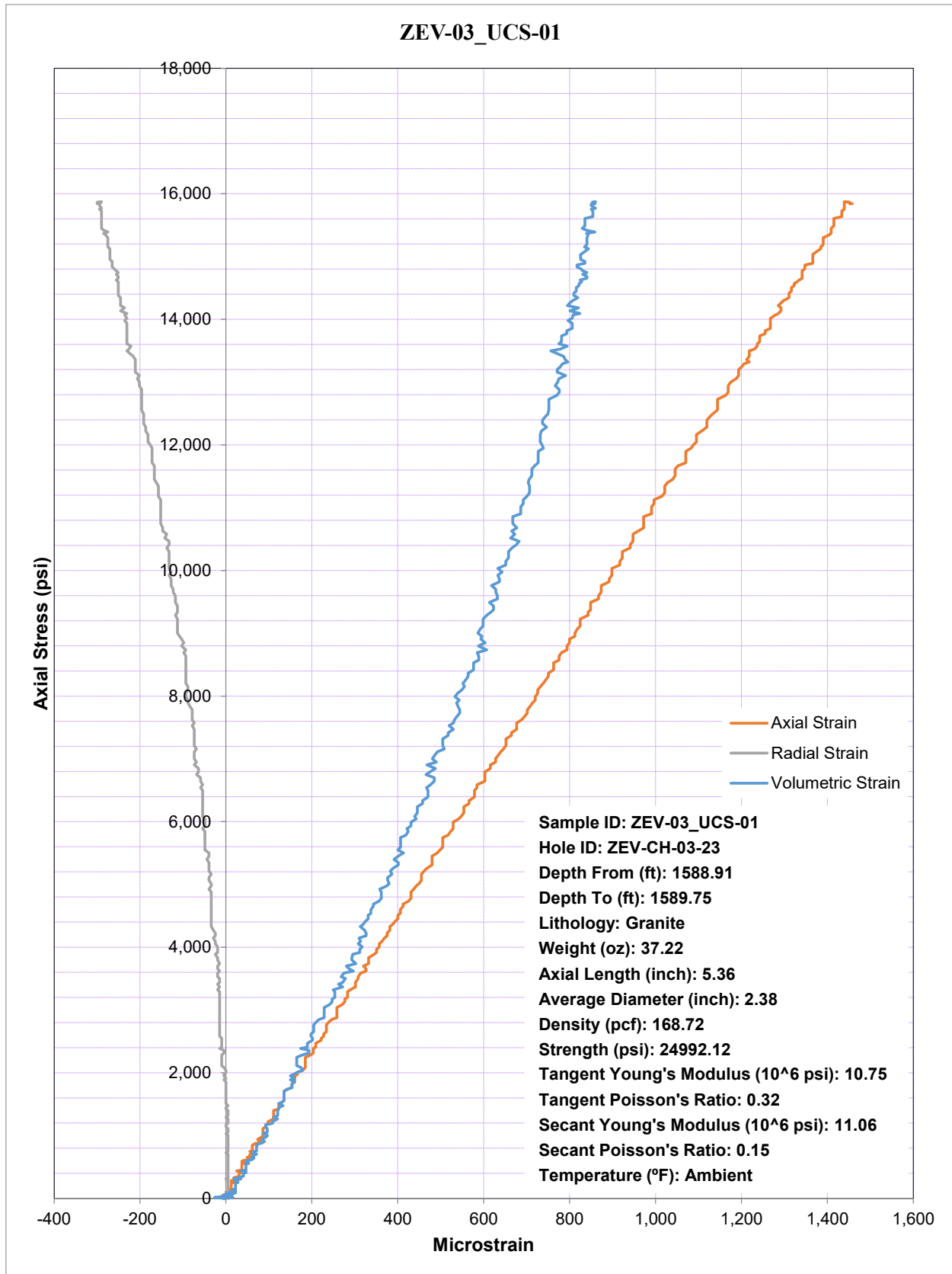
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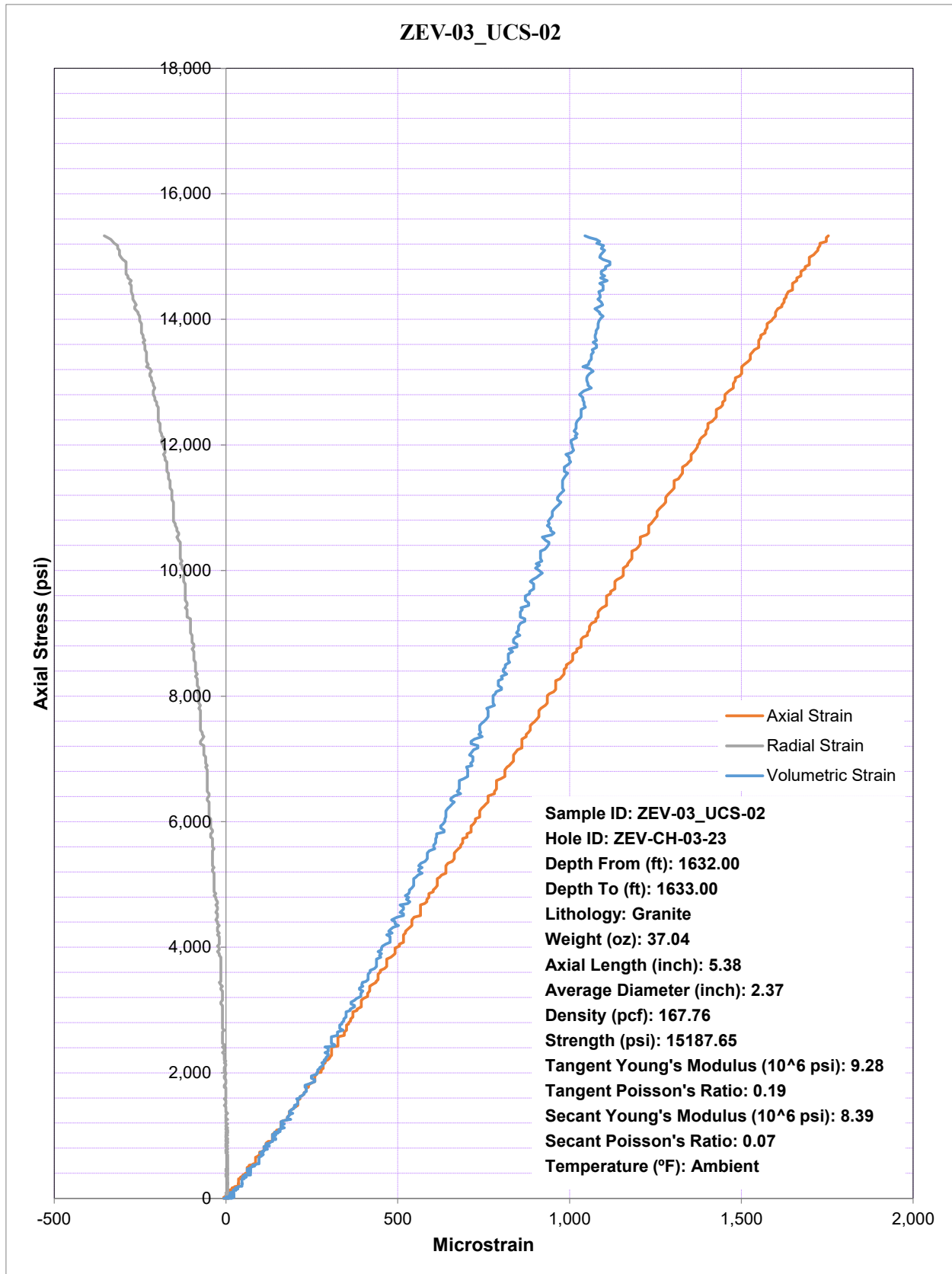


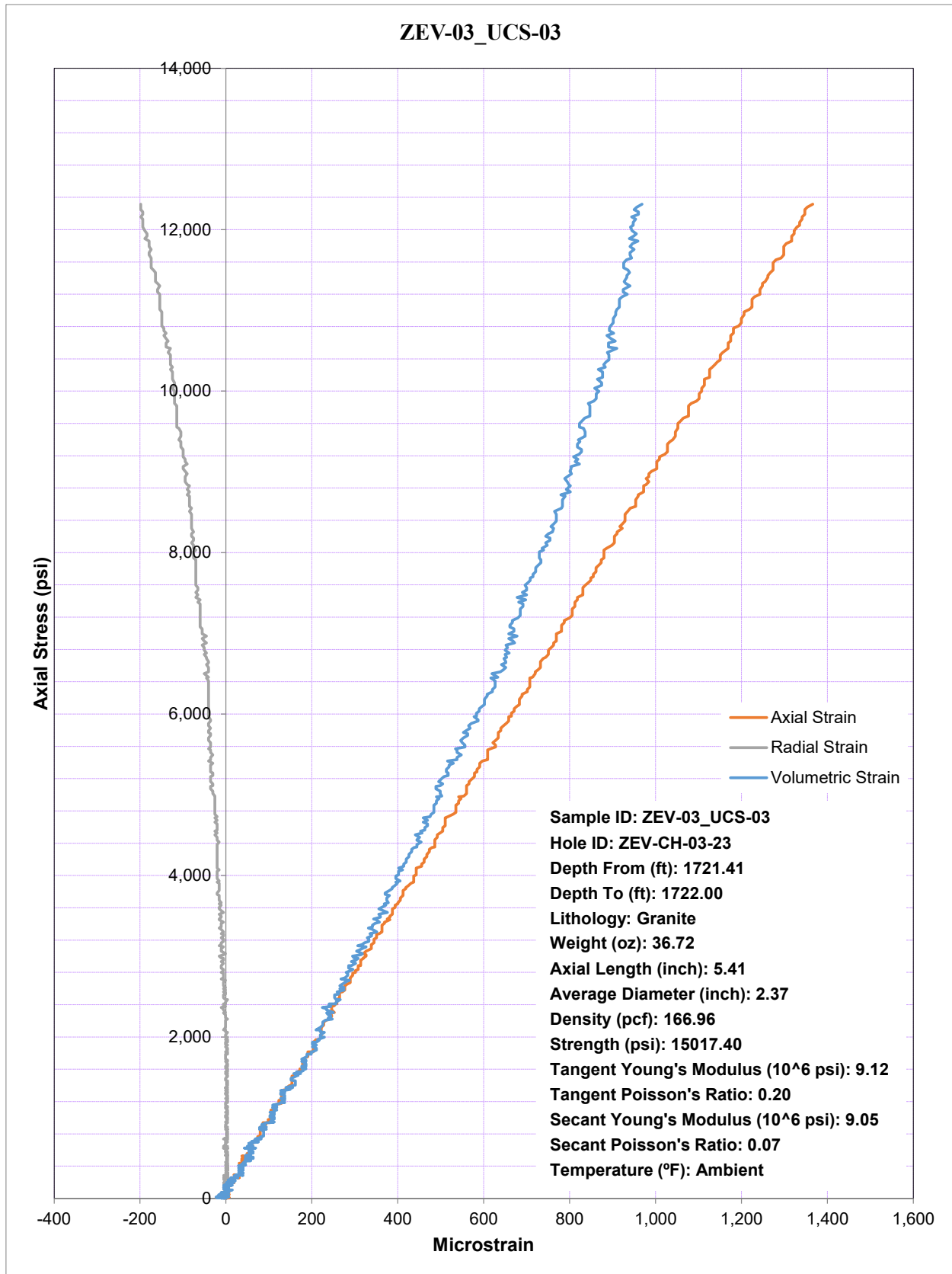
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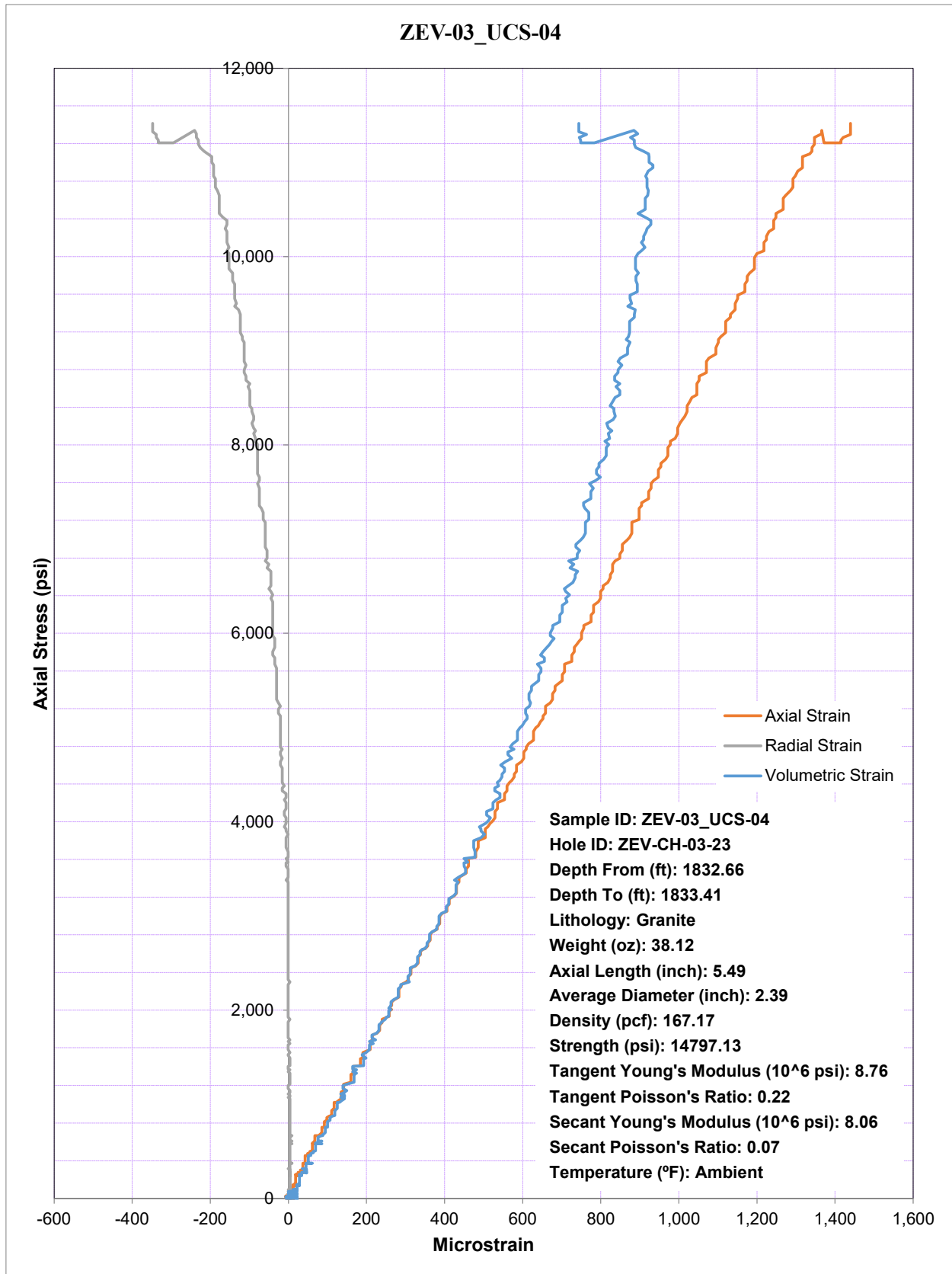
APPENDIX B

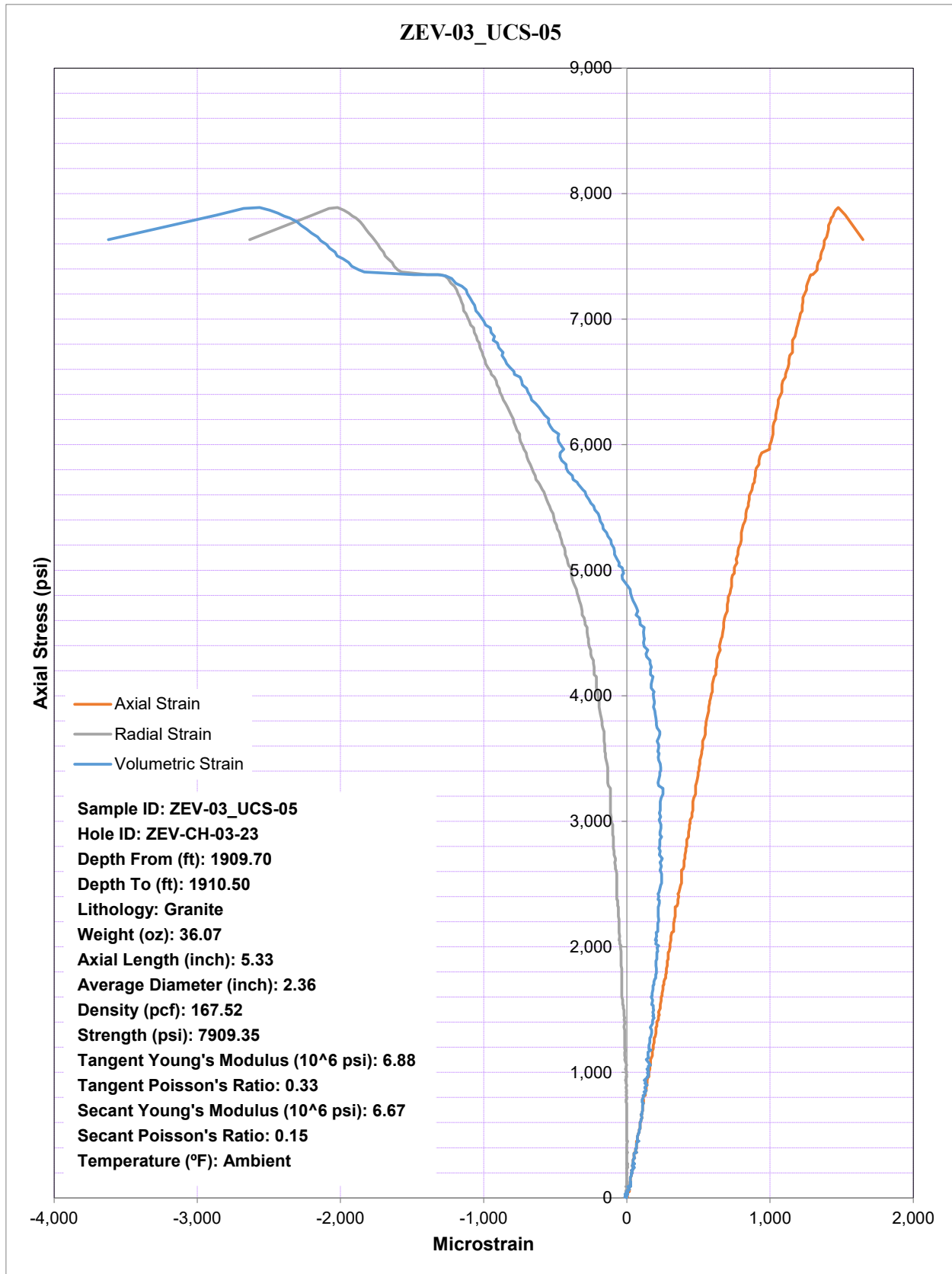
**STRESS-STRAIN PLOTS FOR
UNIAXIAL COMPRESSIVE STRENGTH TEST SPECIMENS**

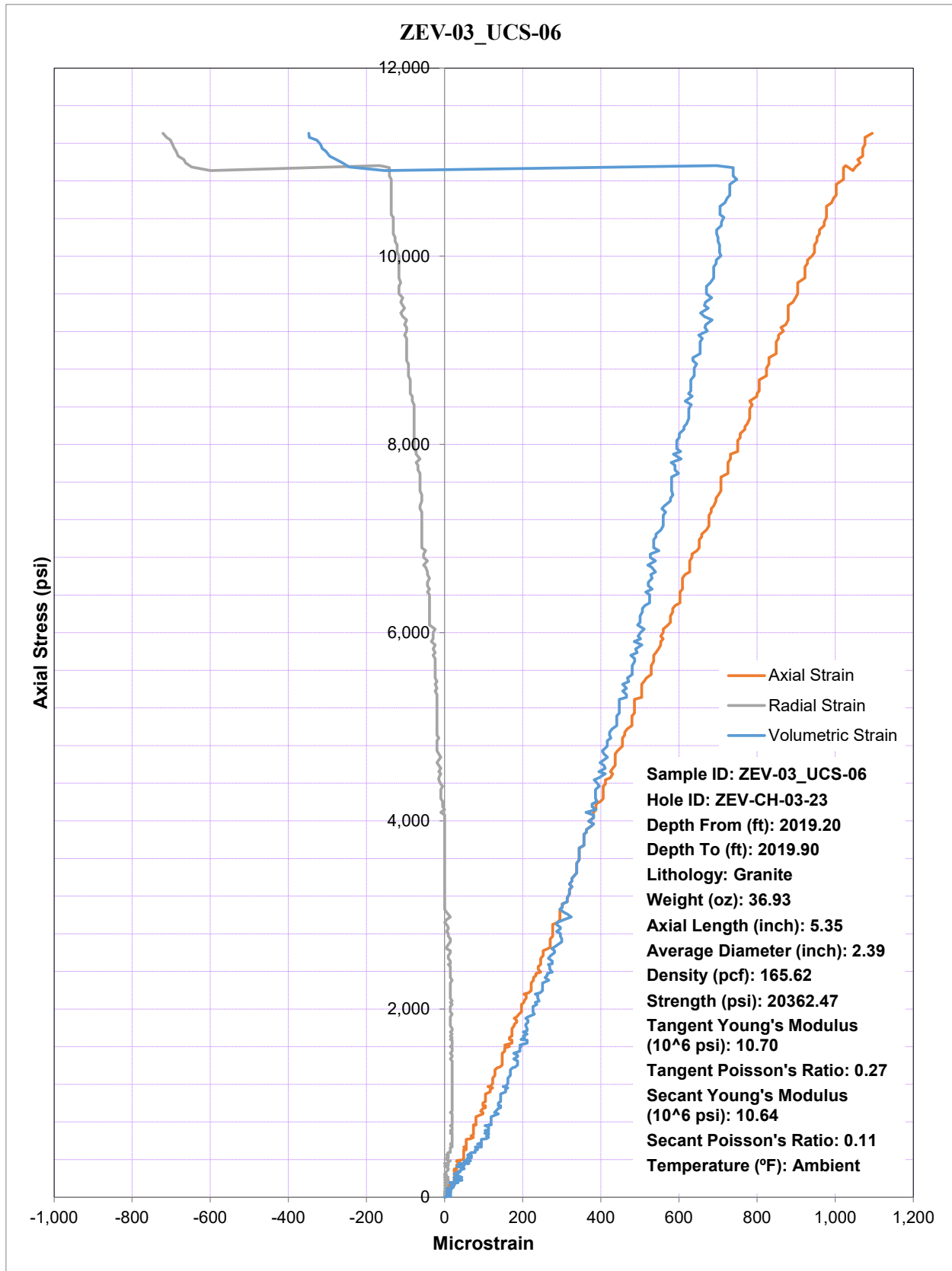


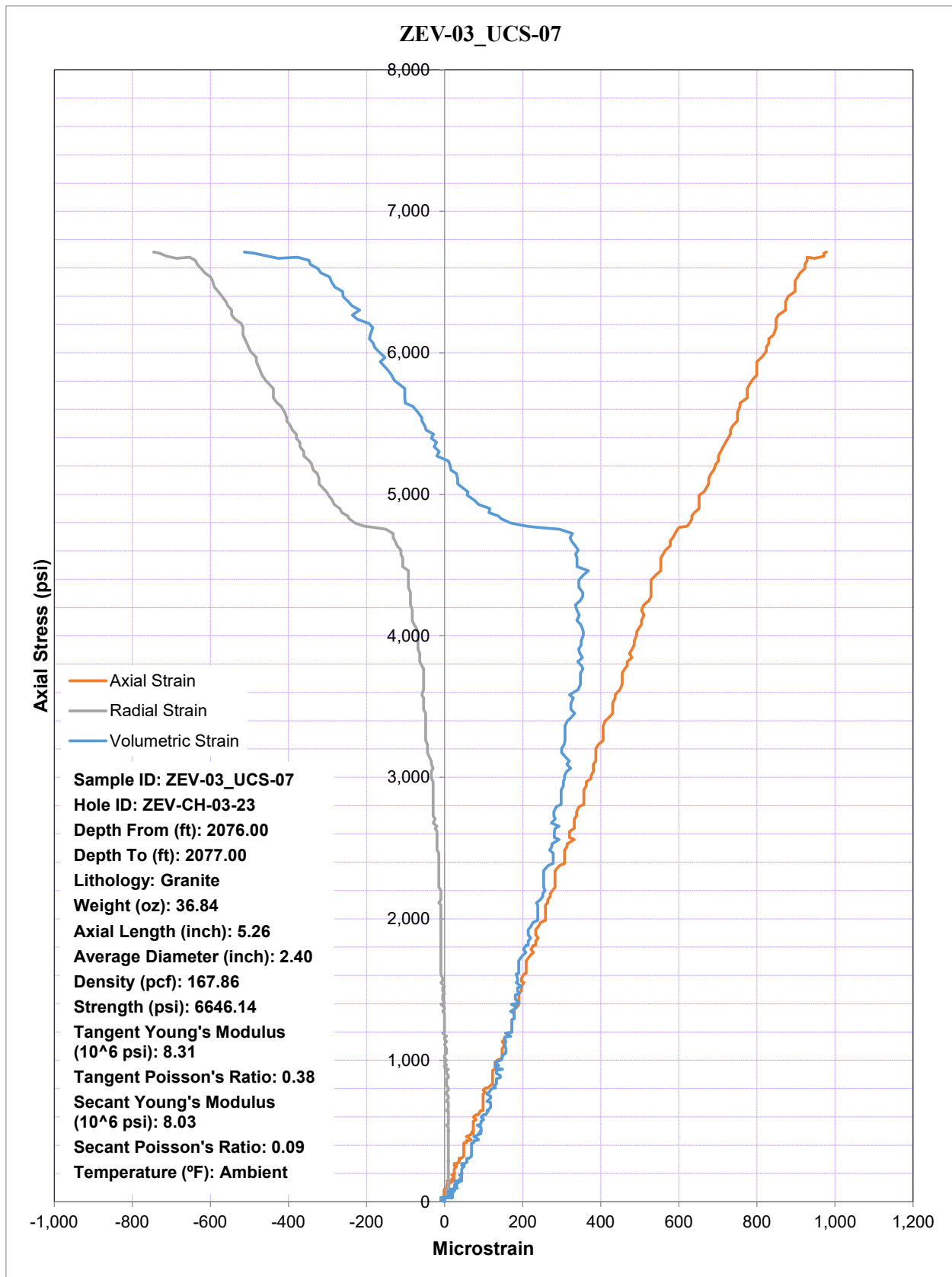


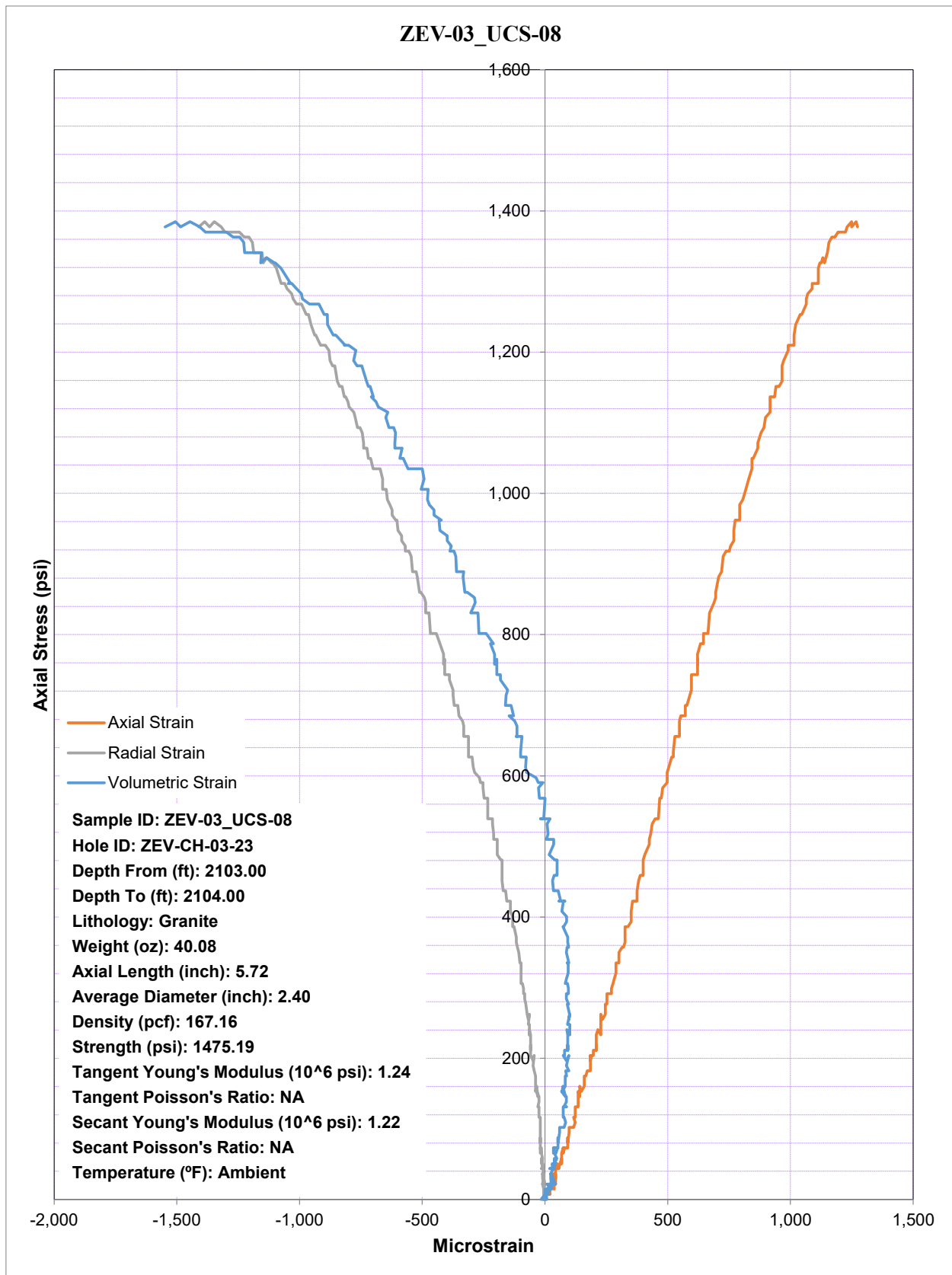


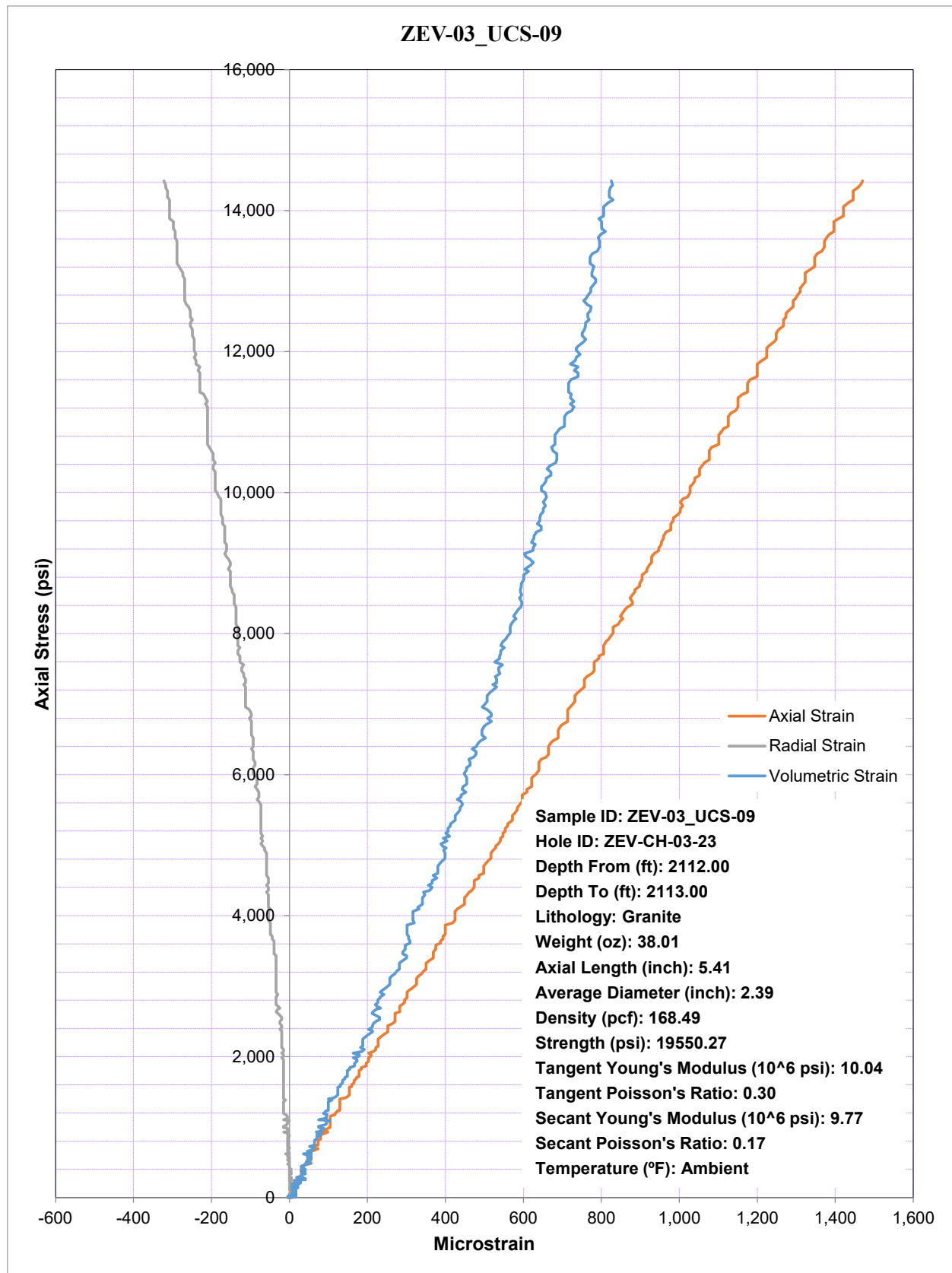


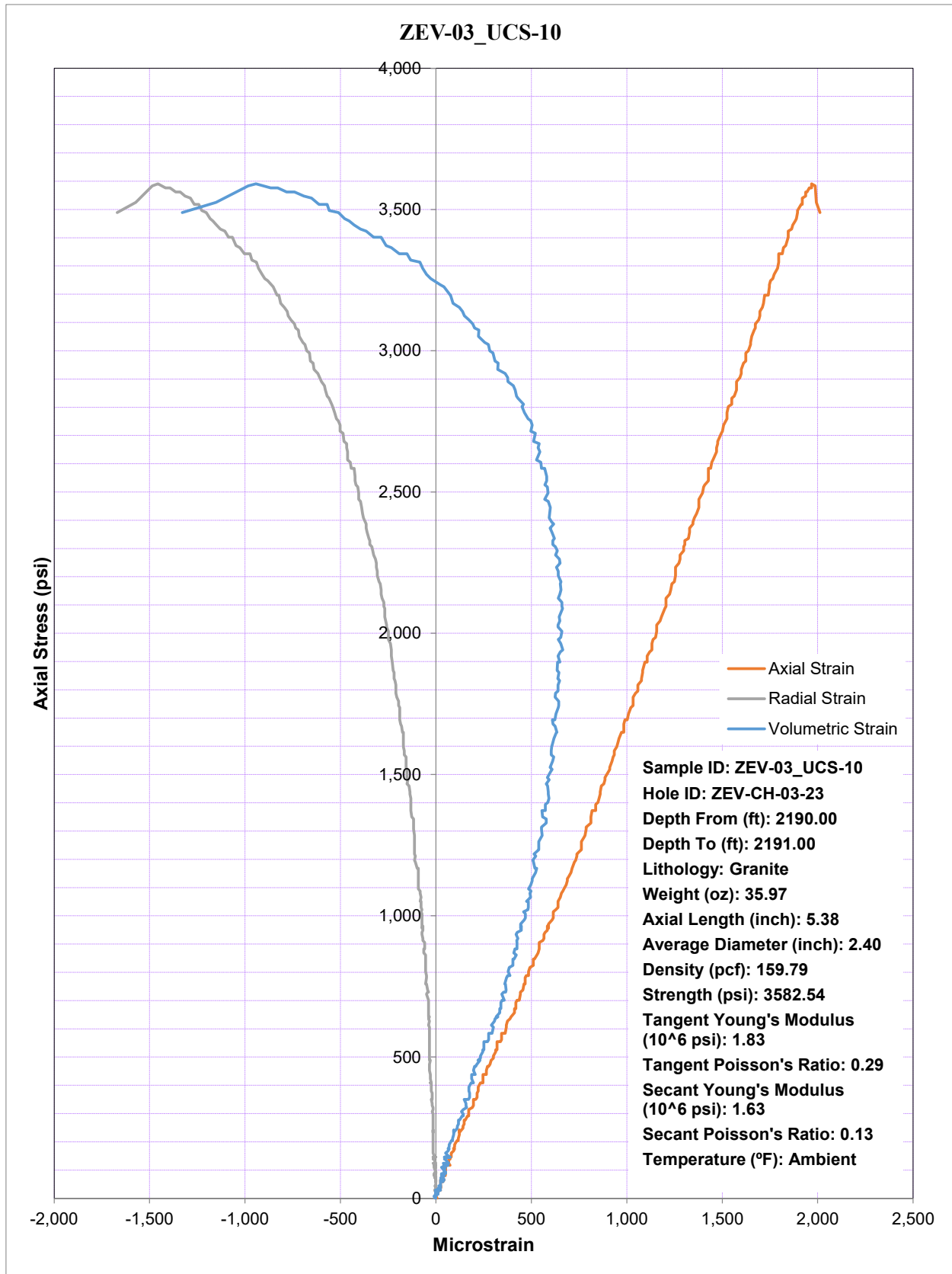


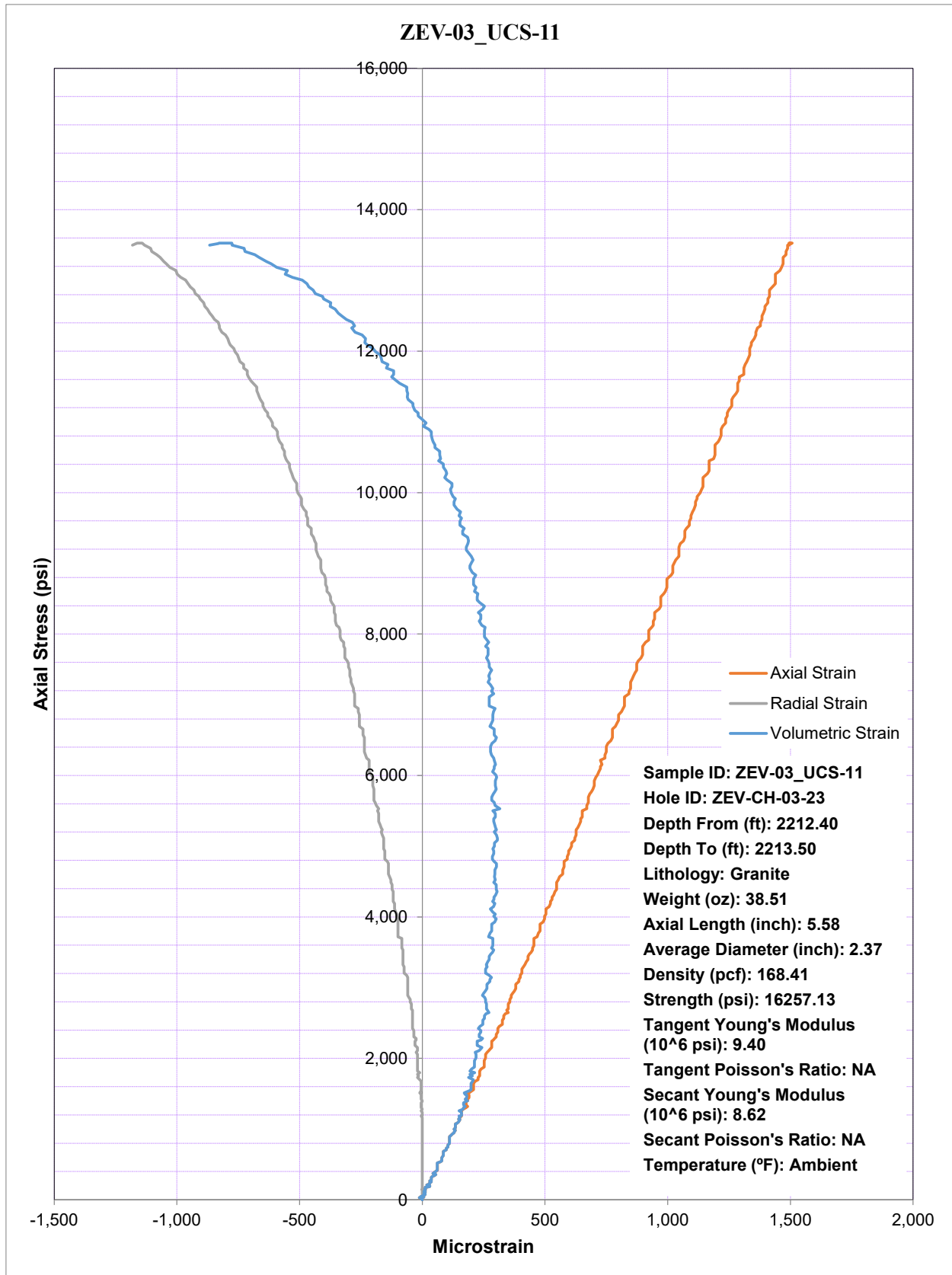


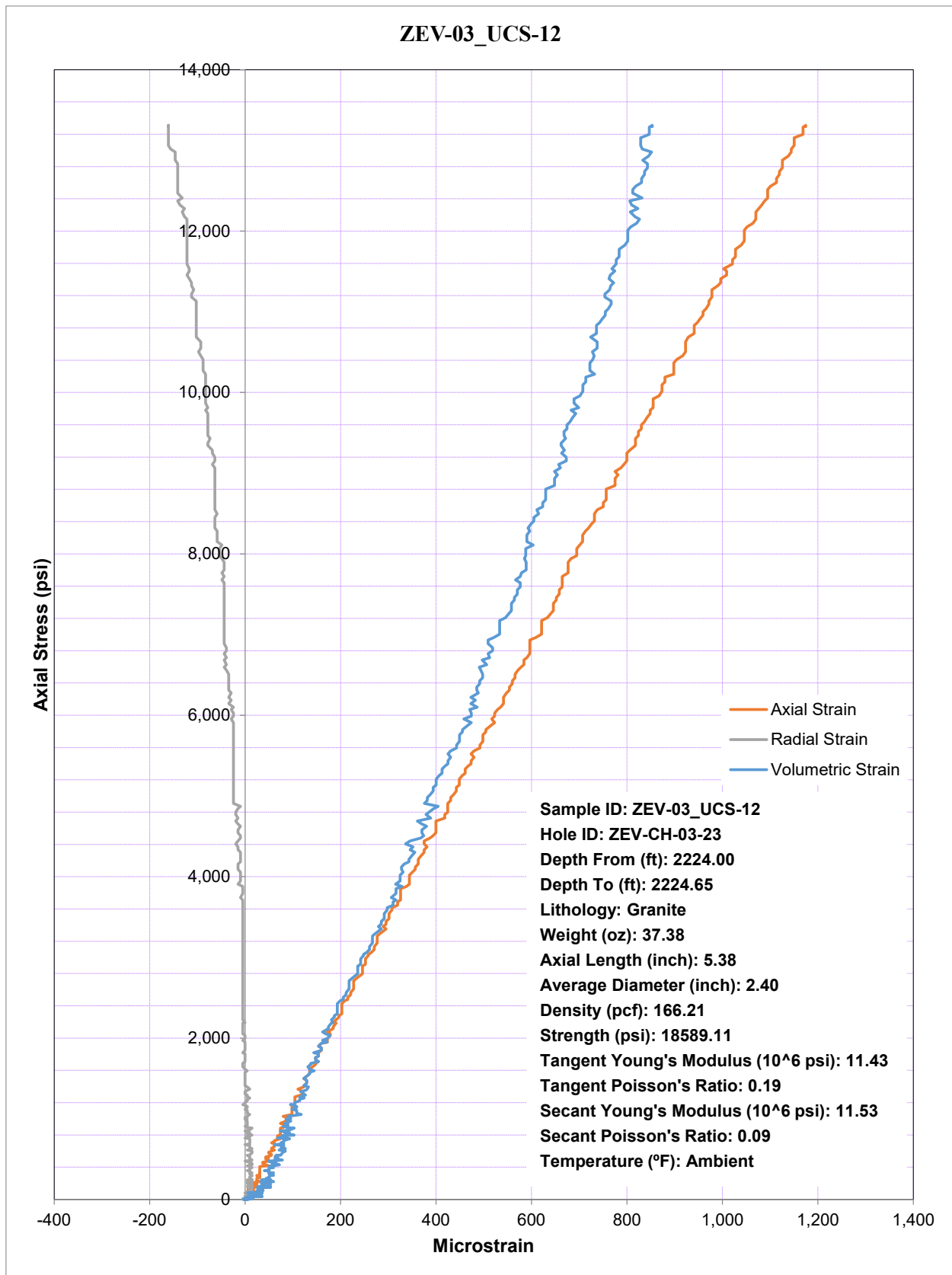


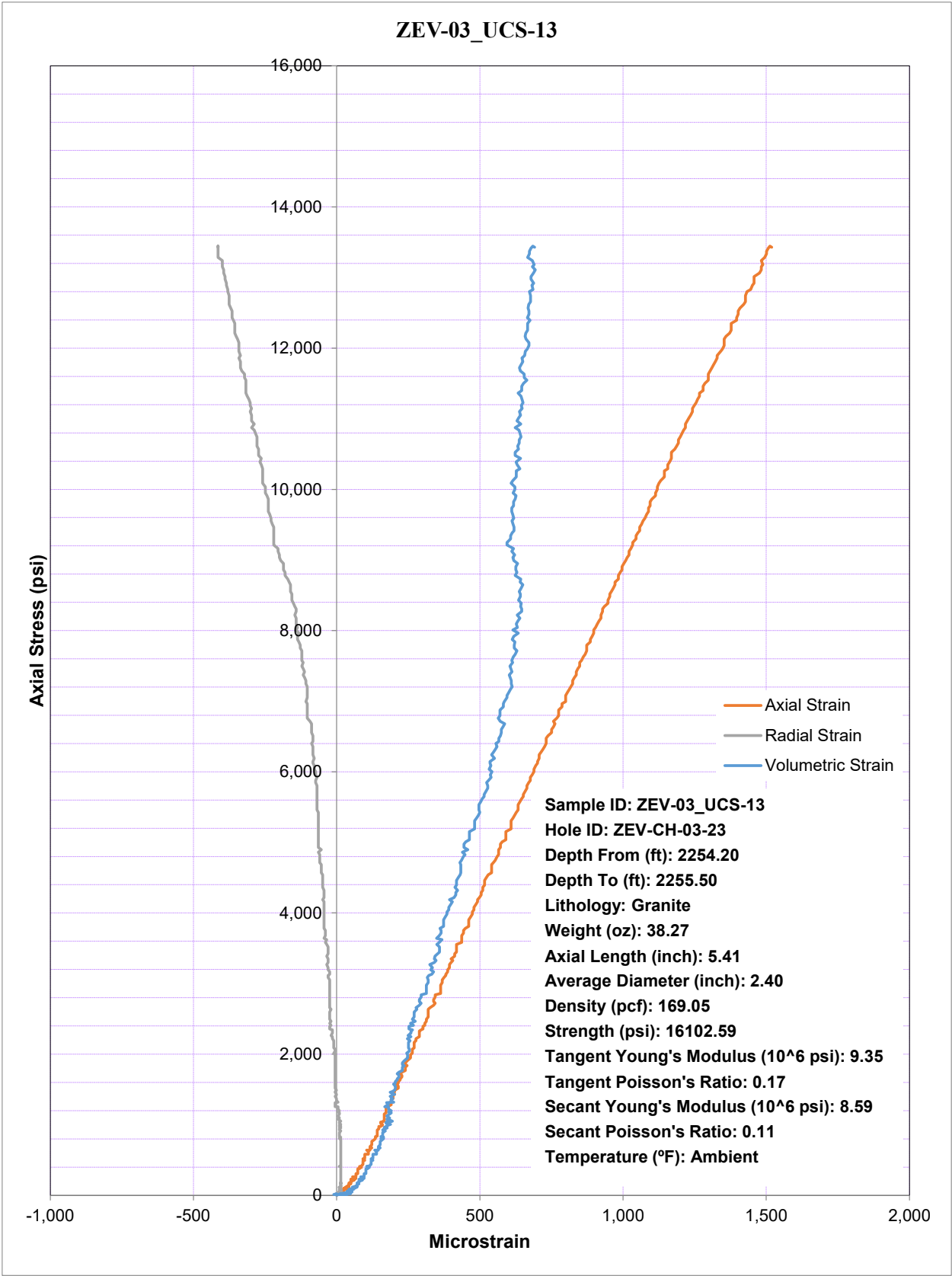


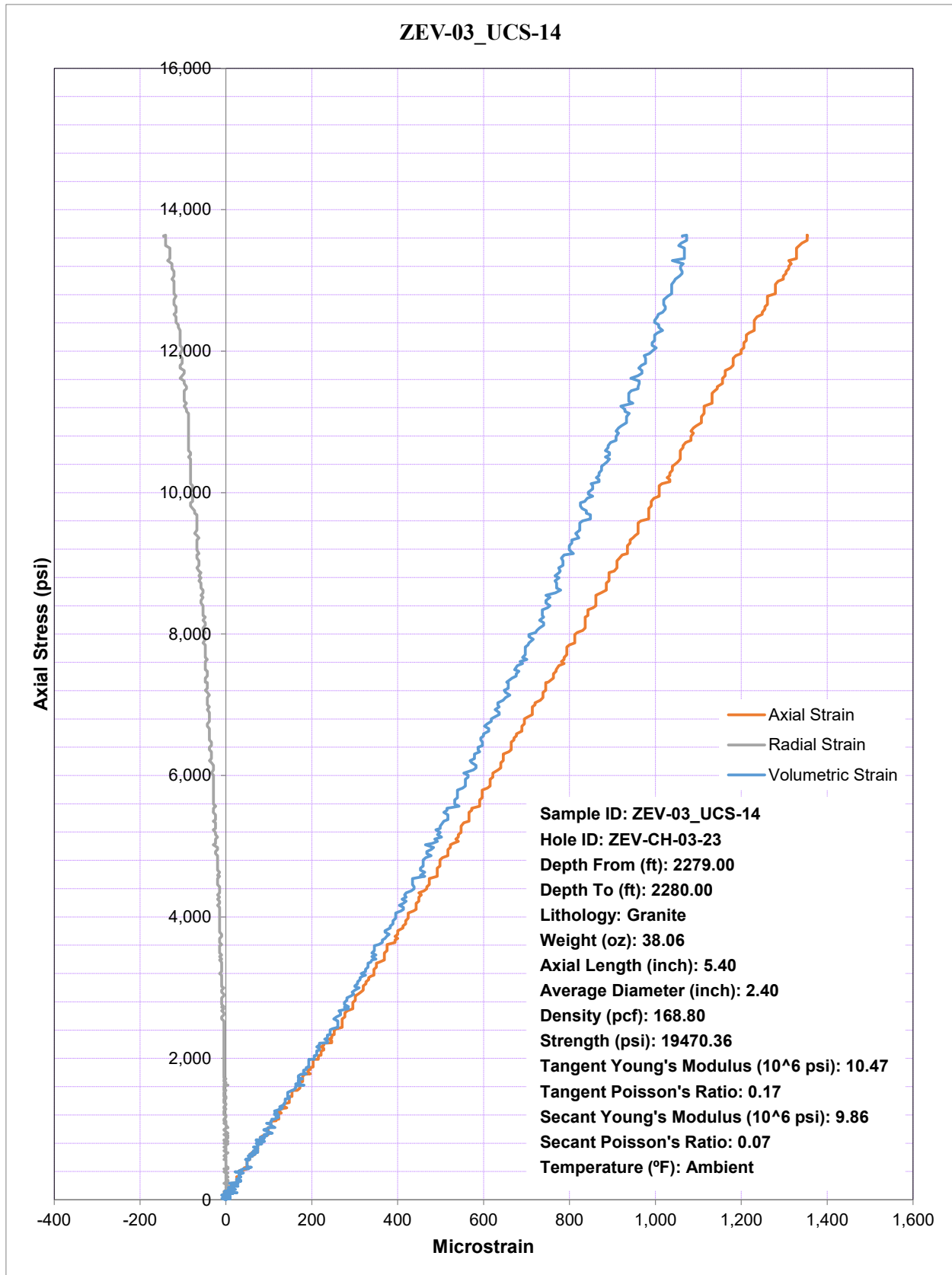


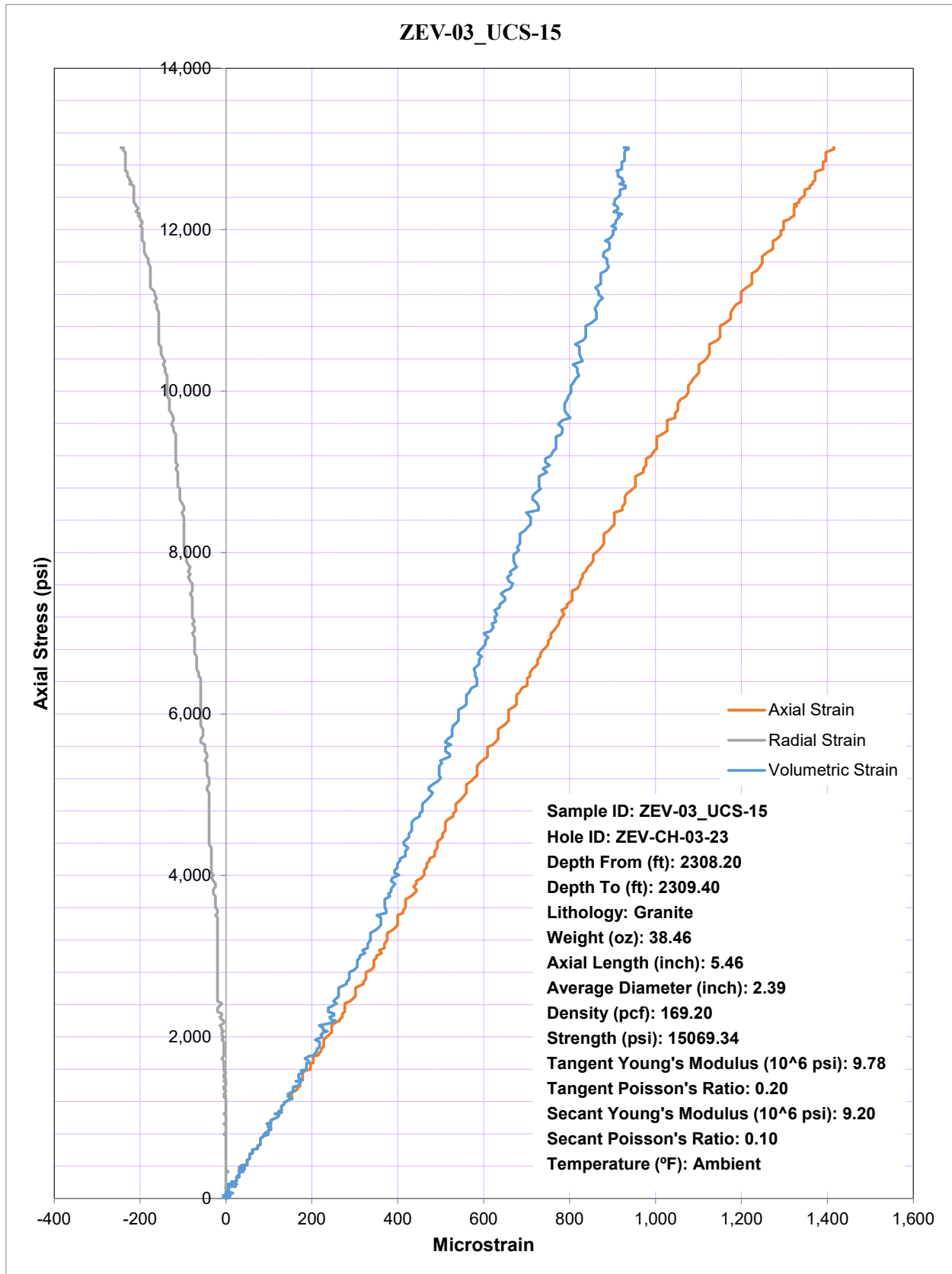


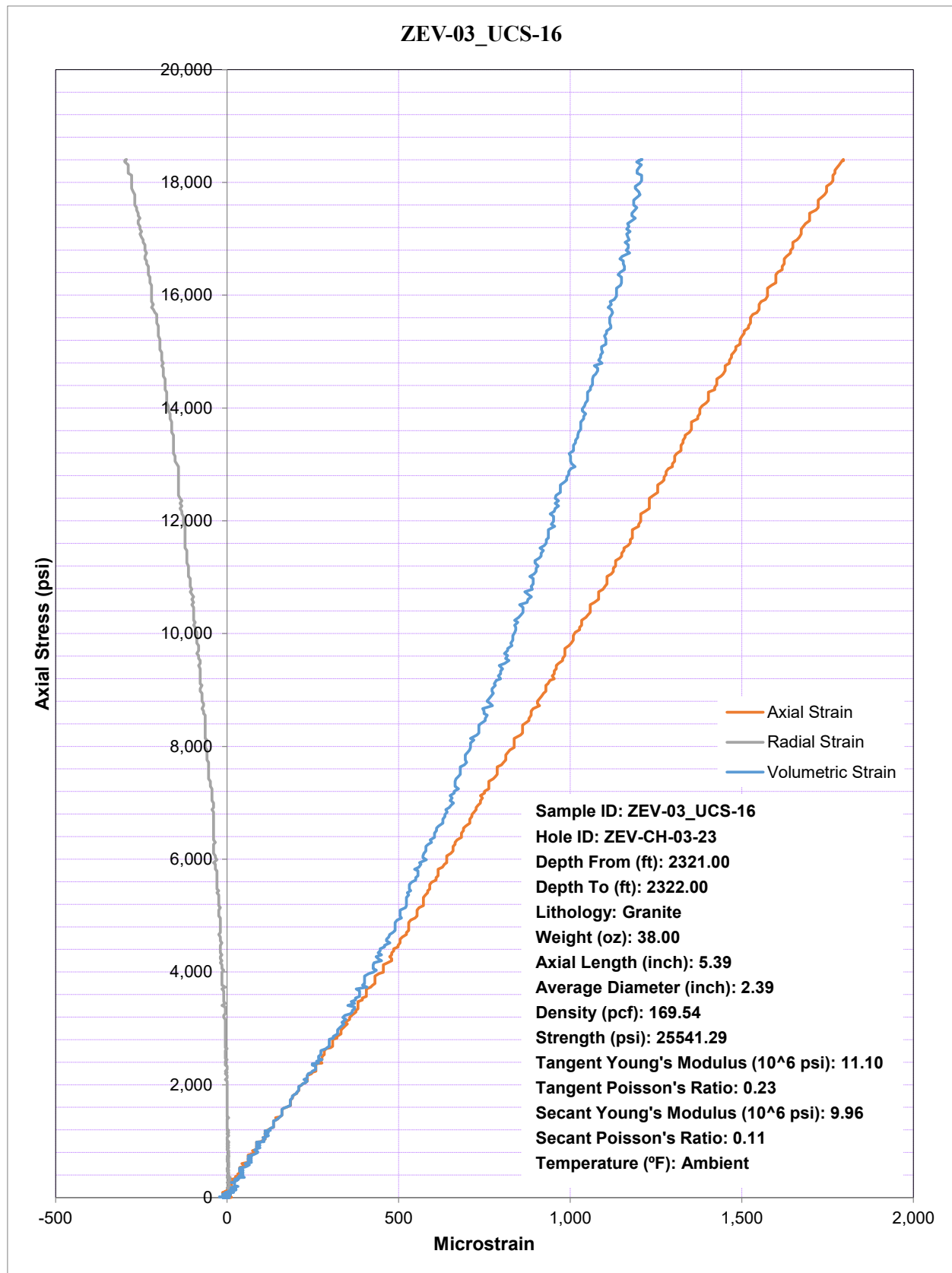


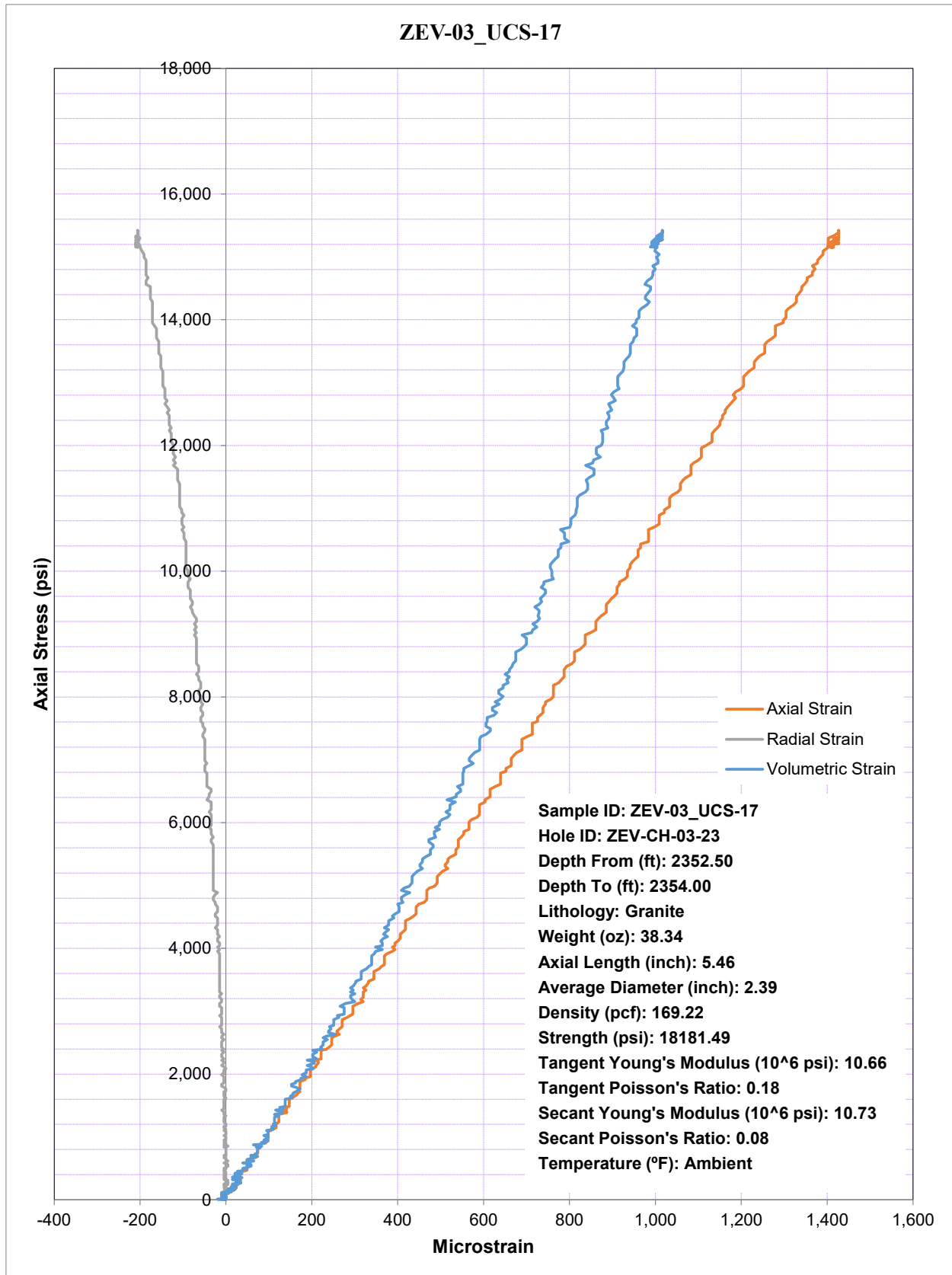


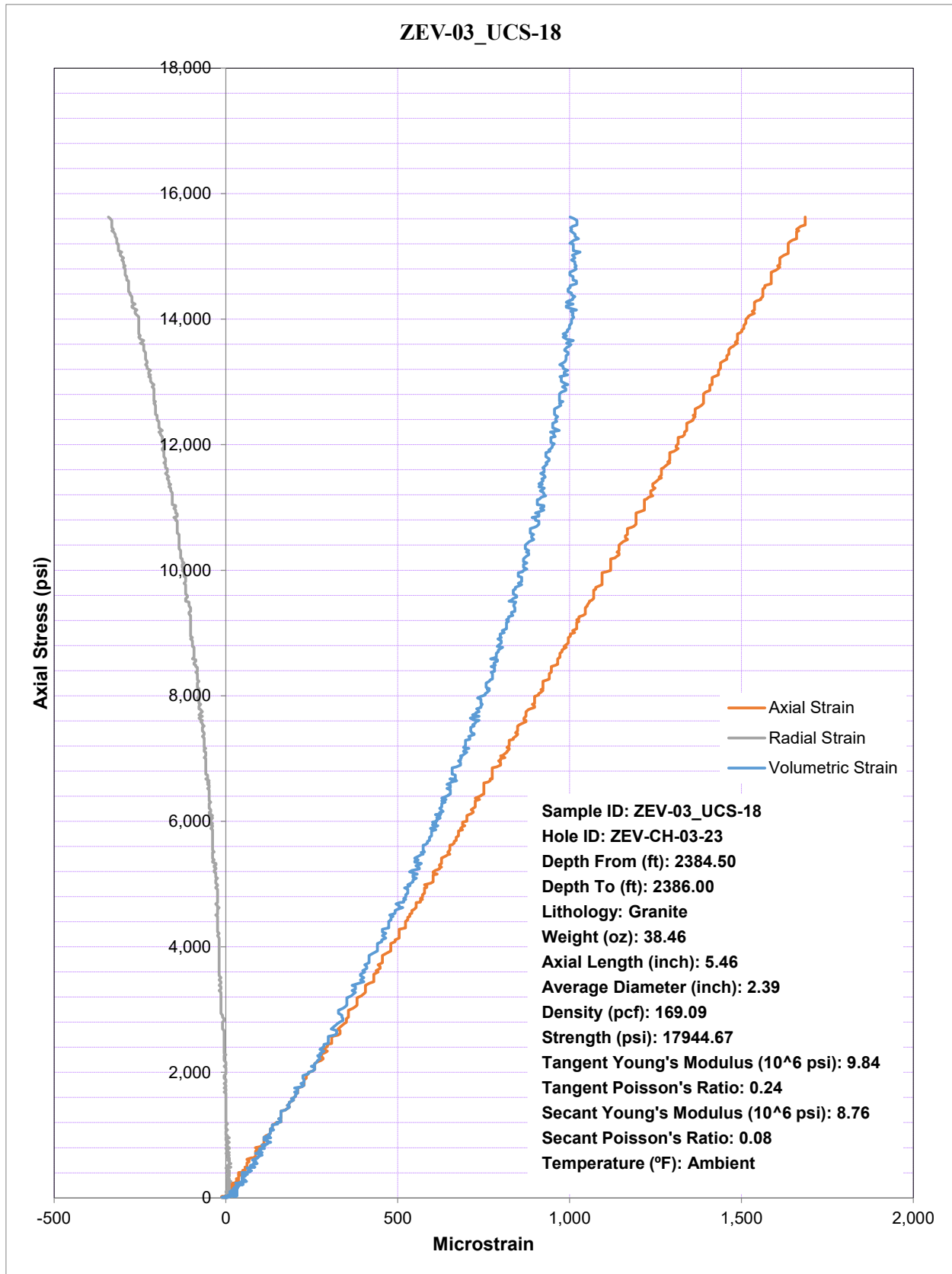


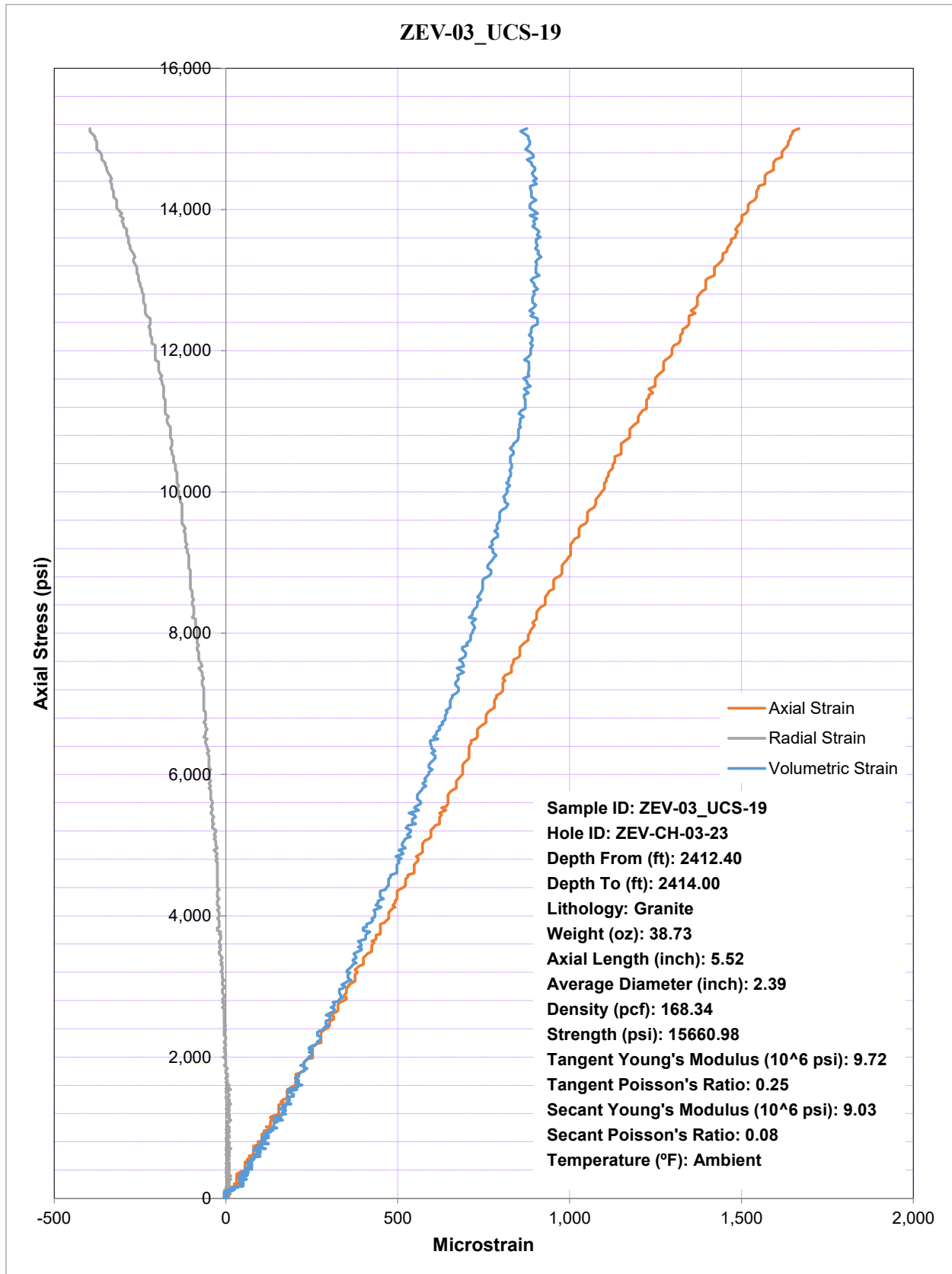


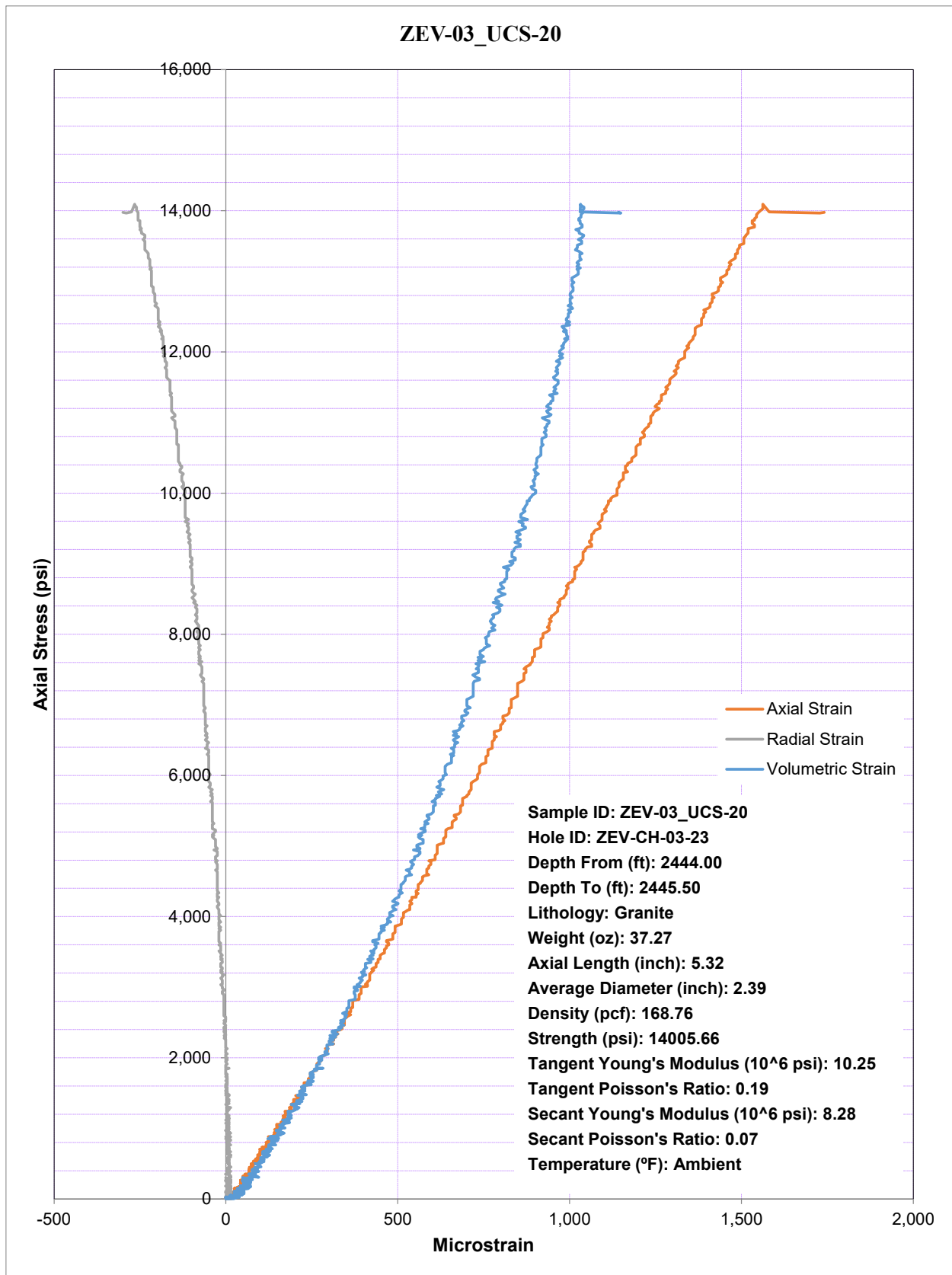


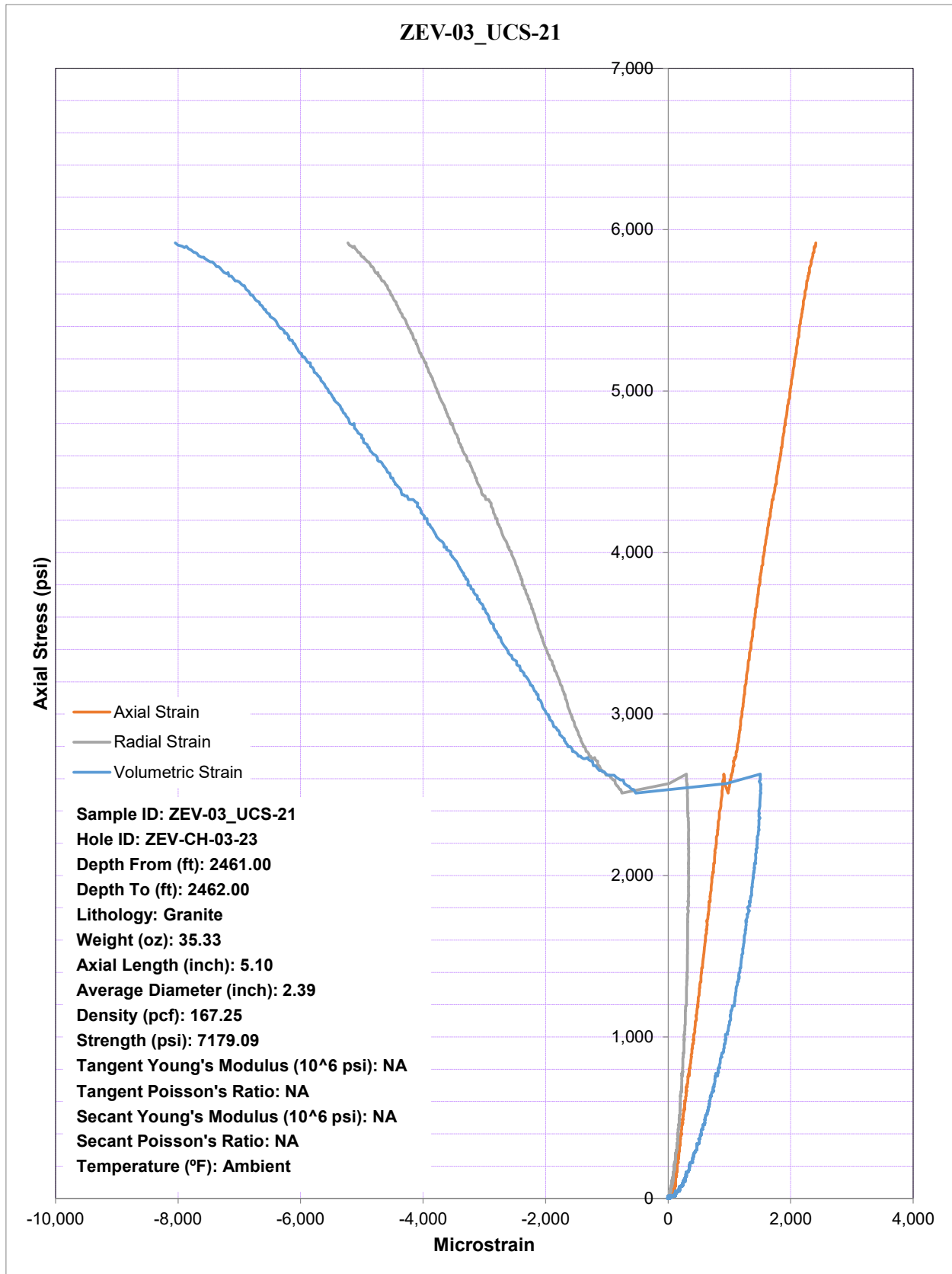


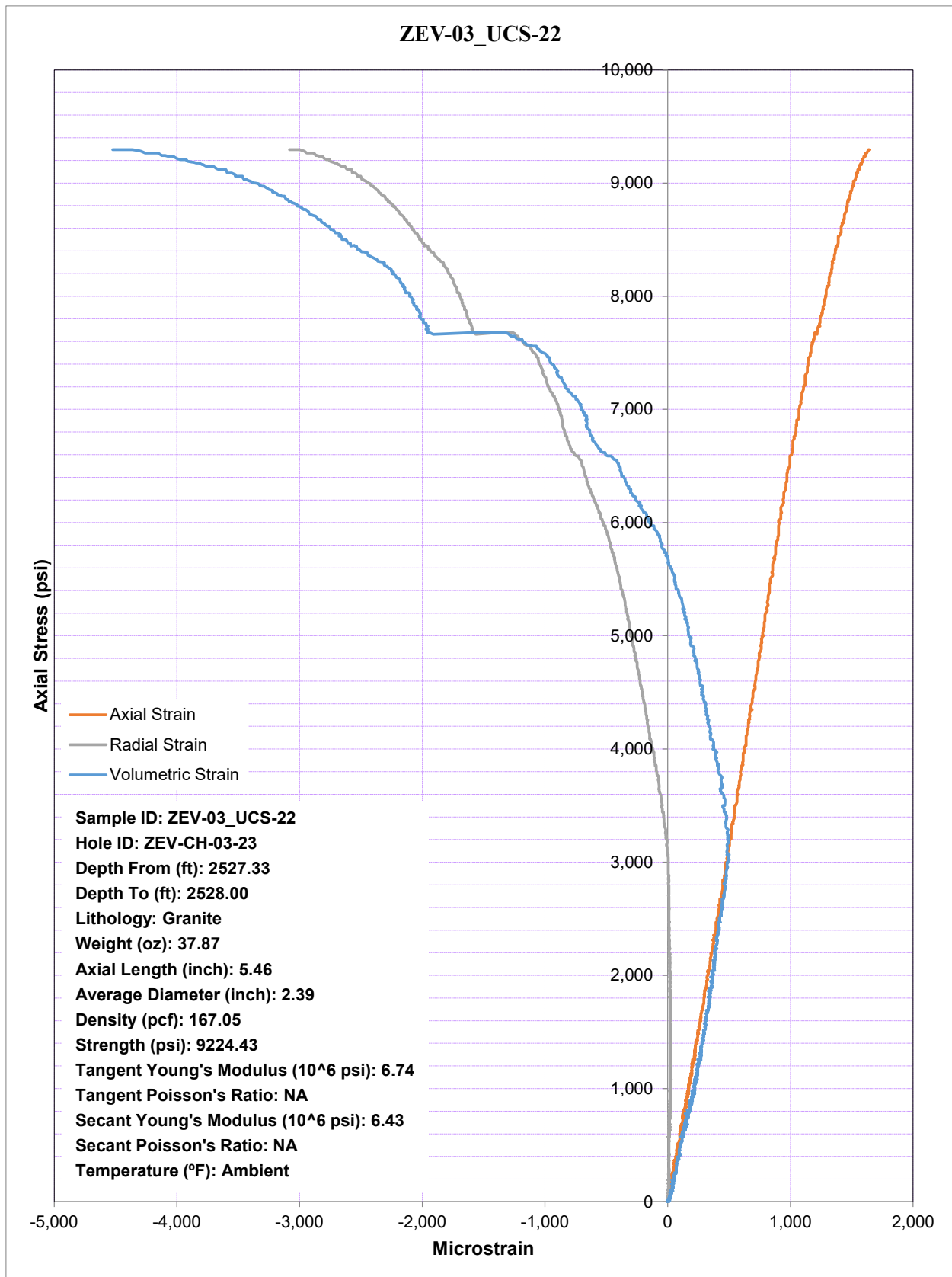


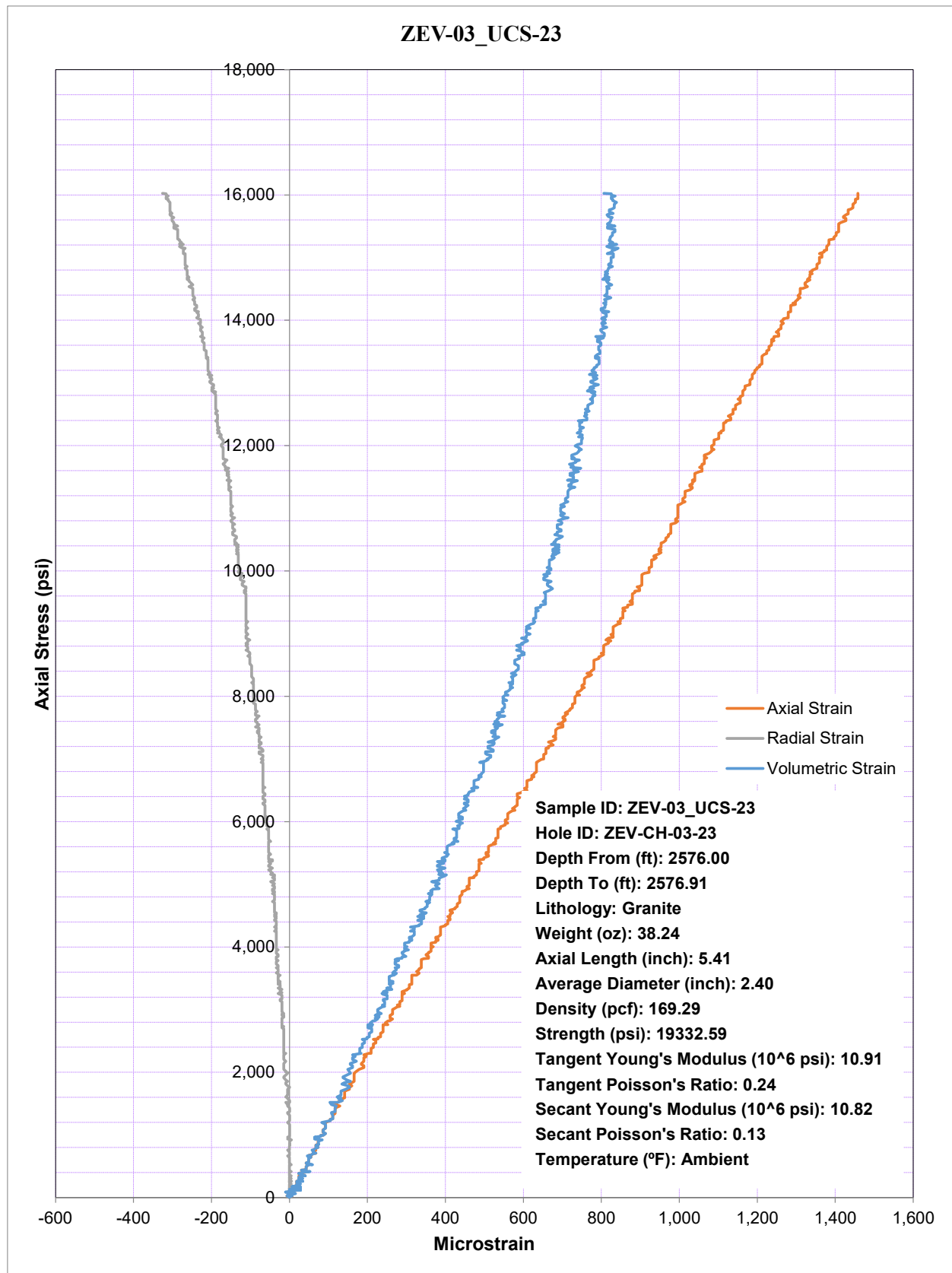


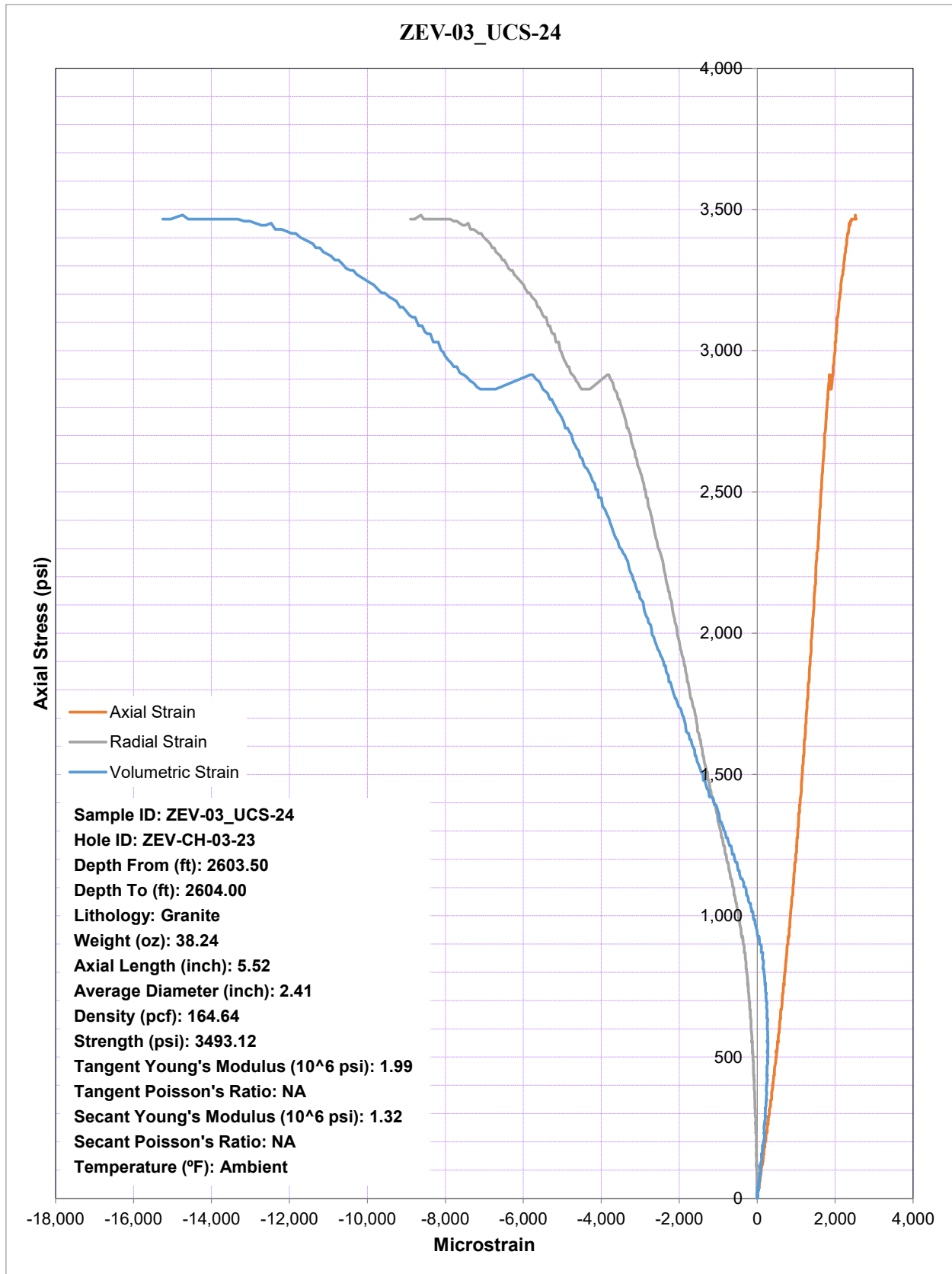


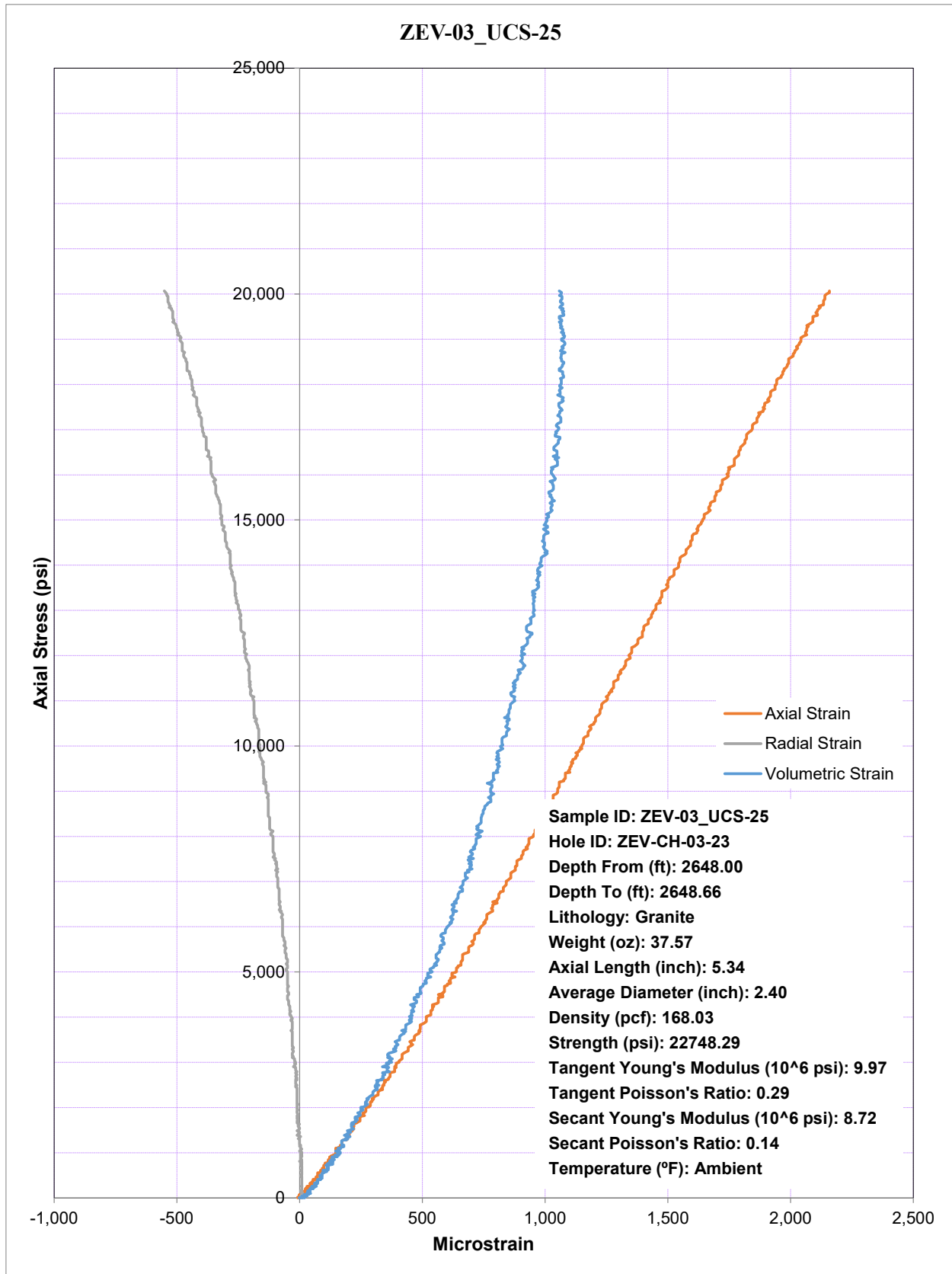


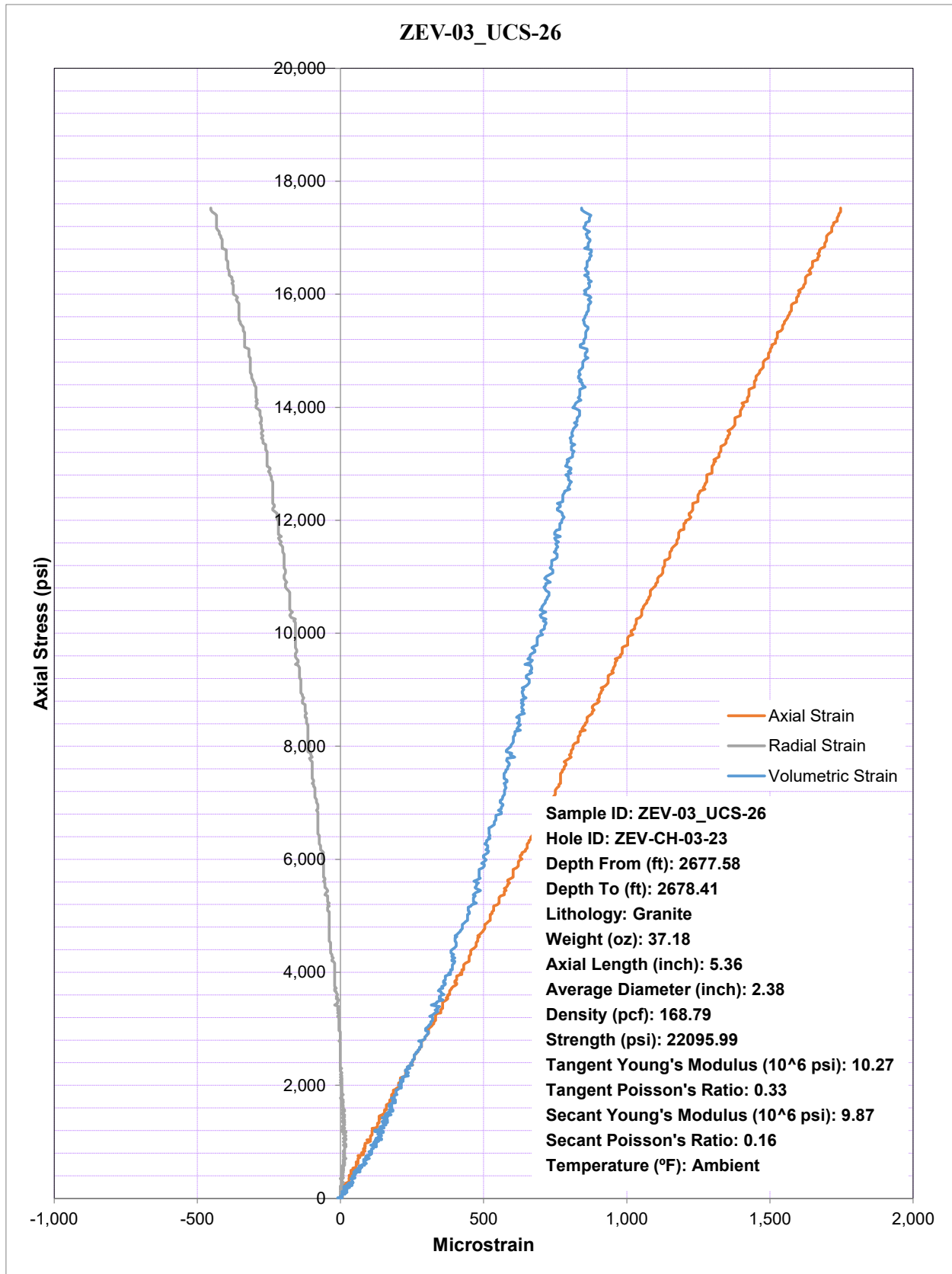


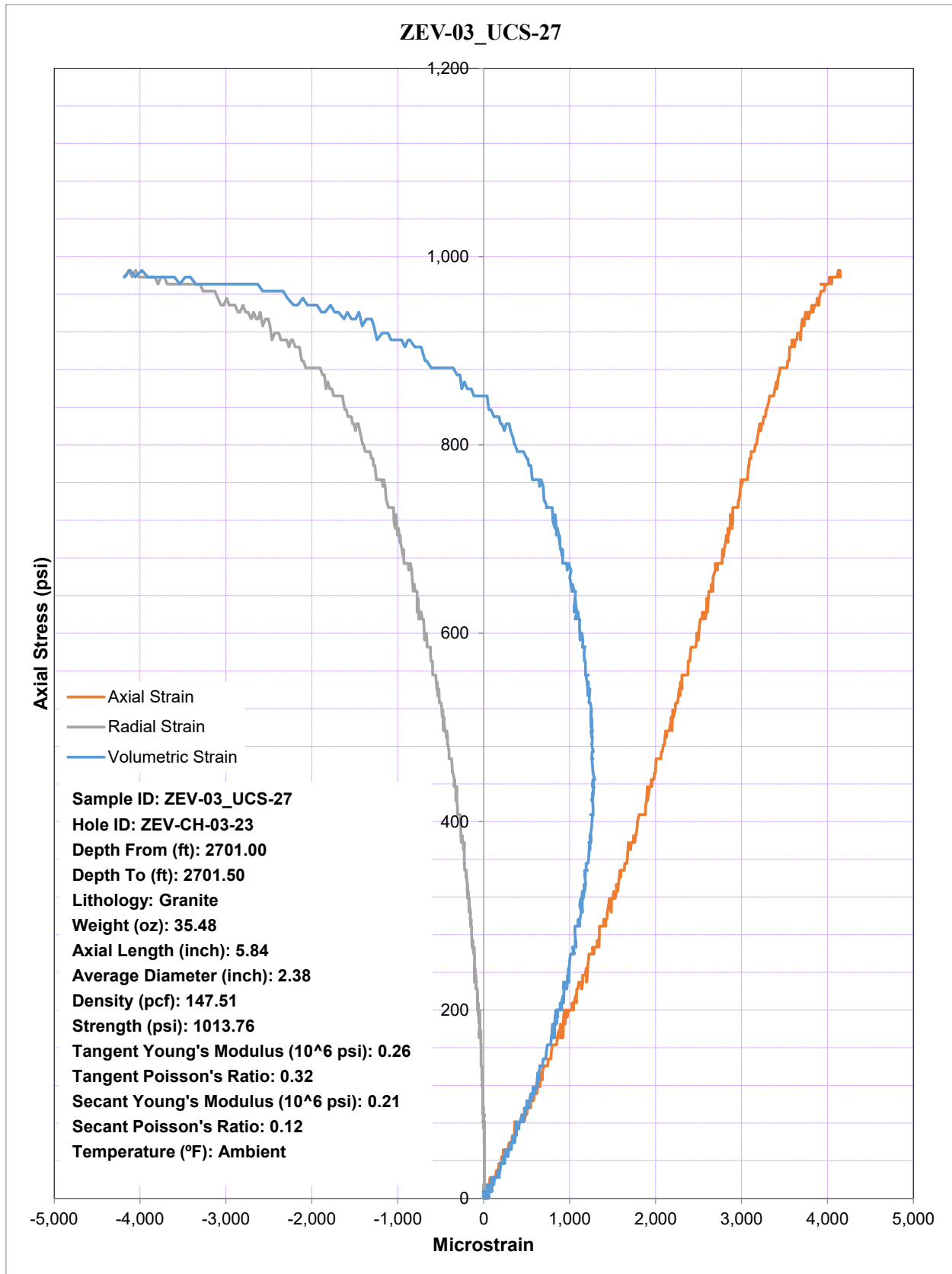


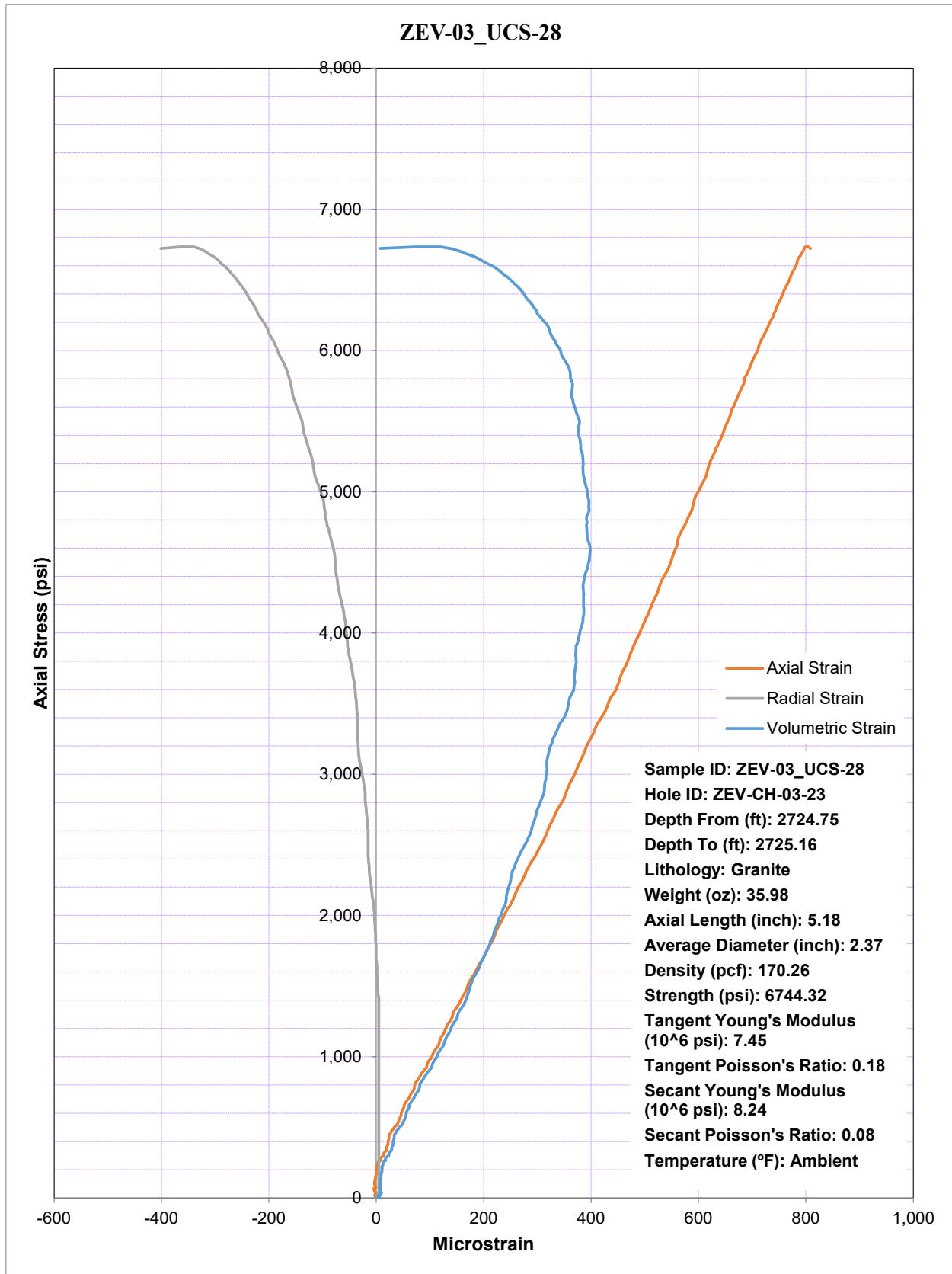


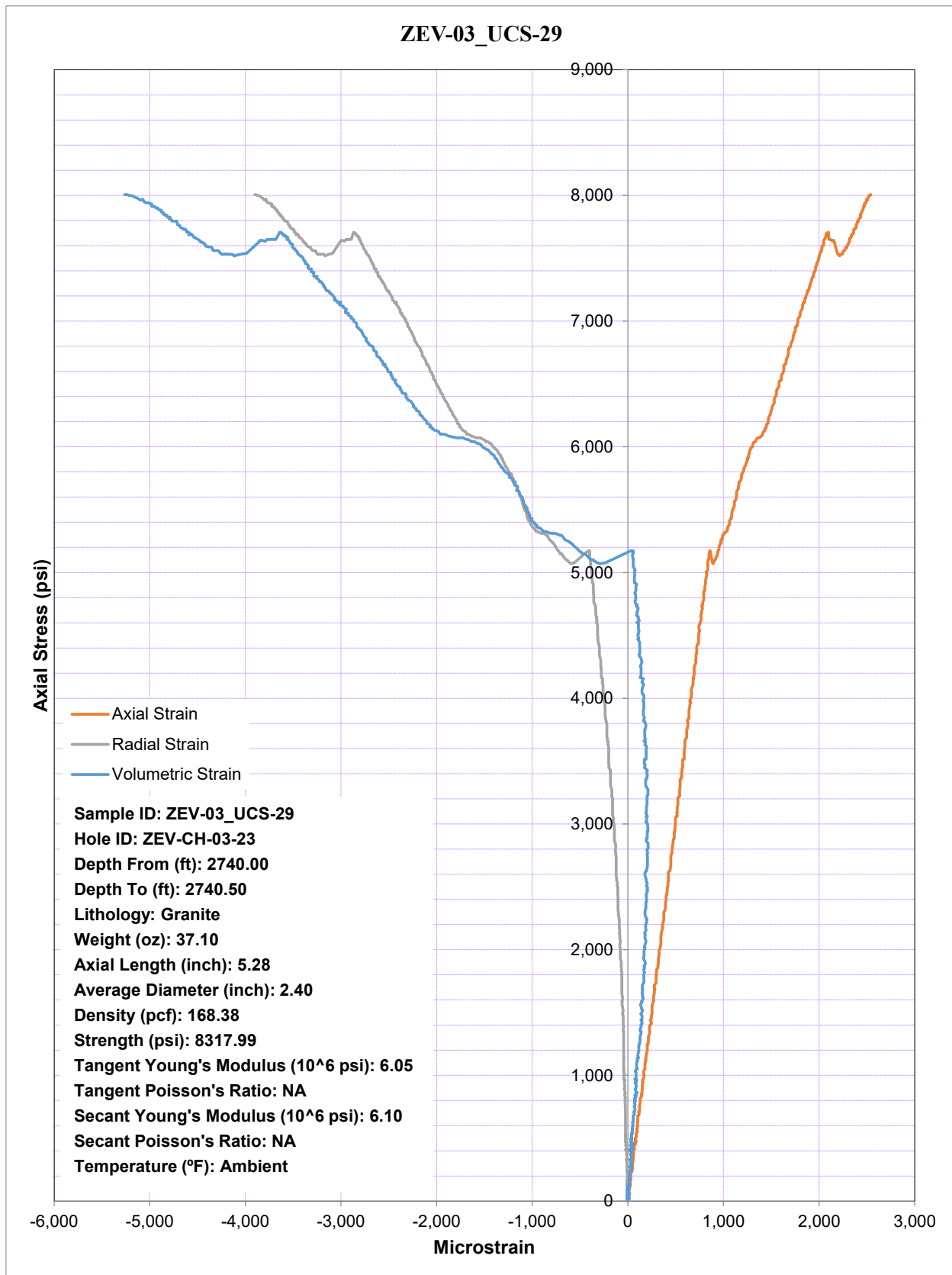


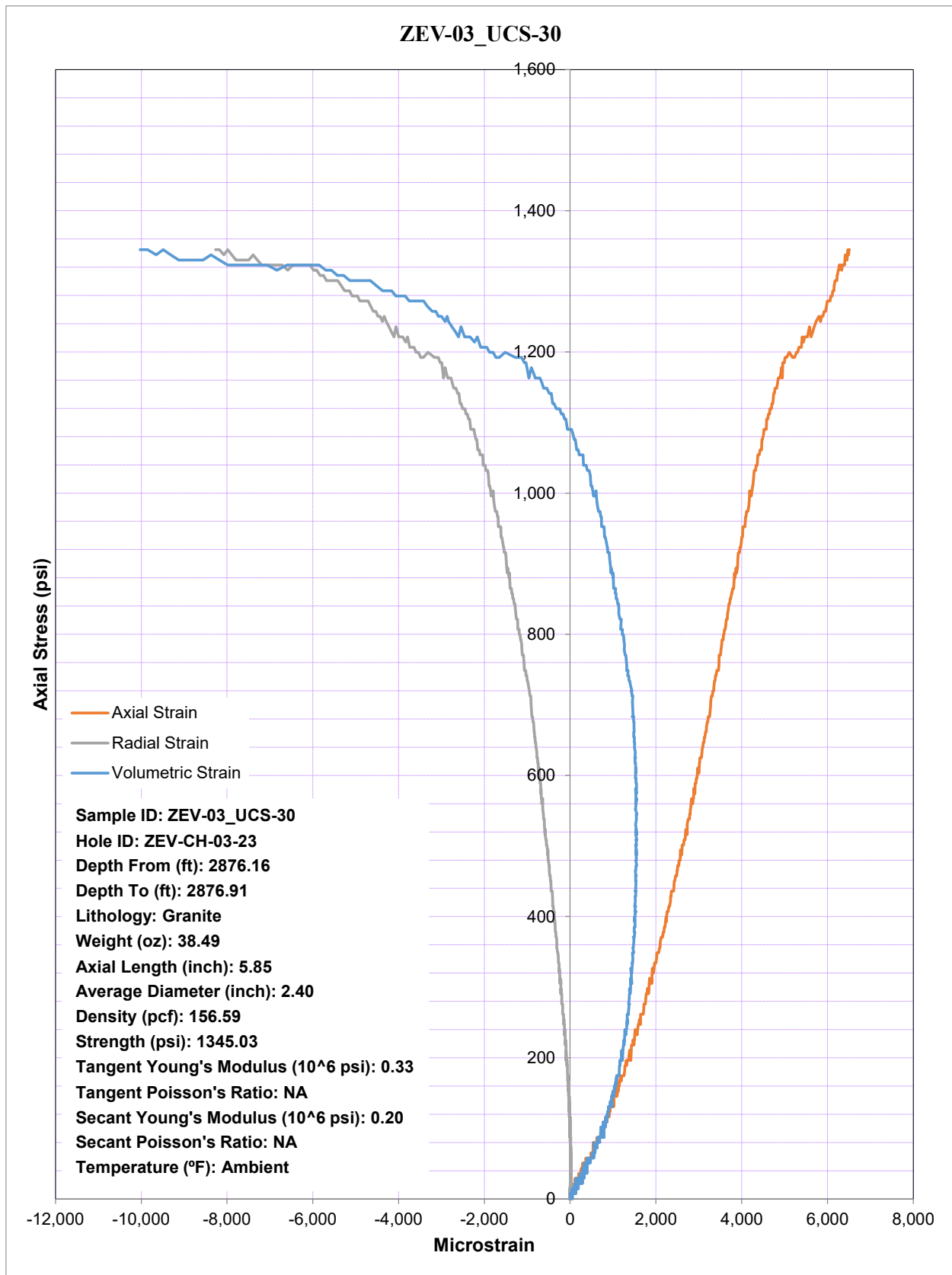


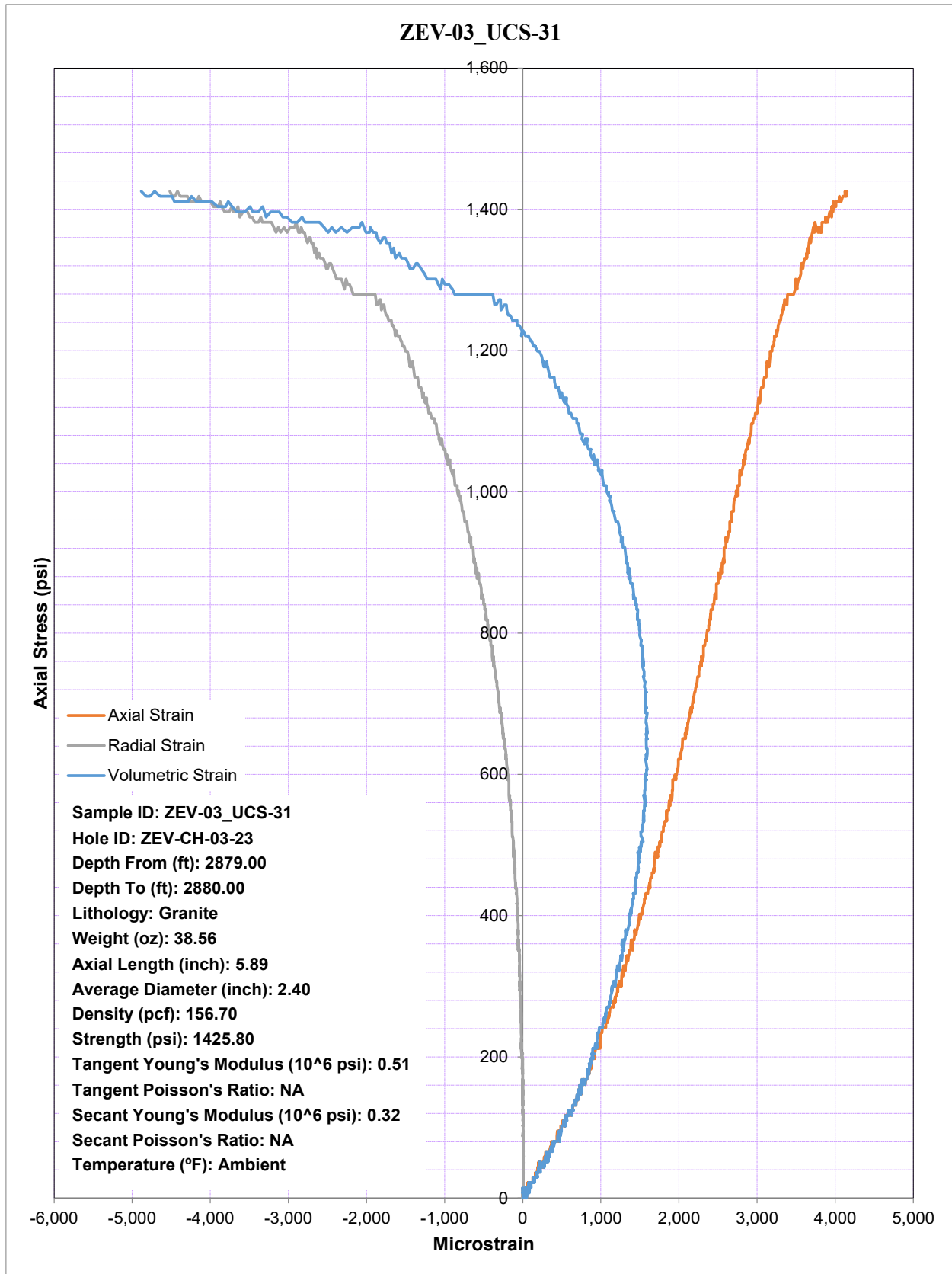


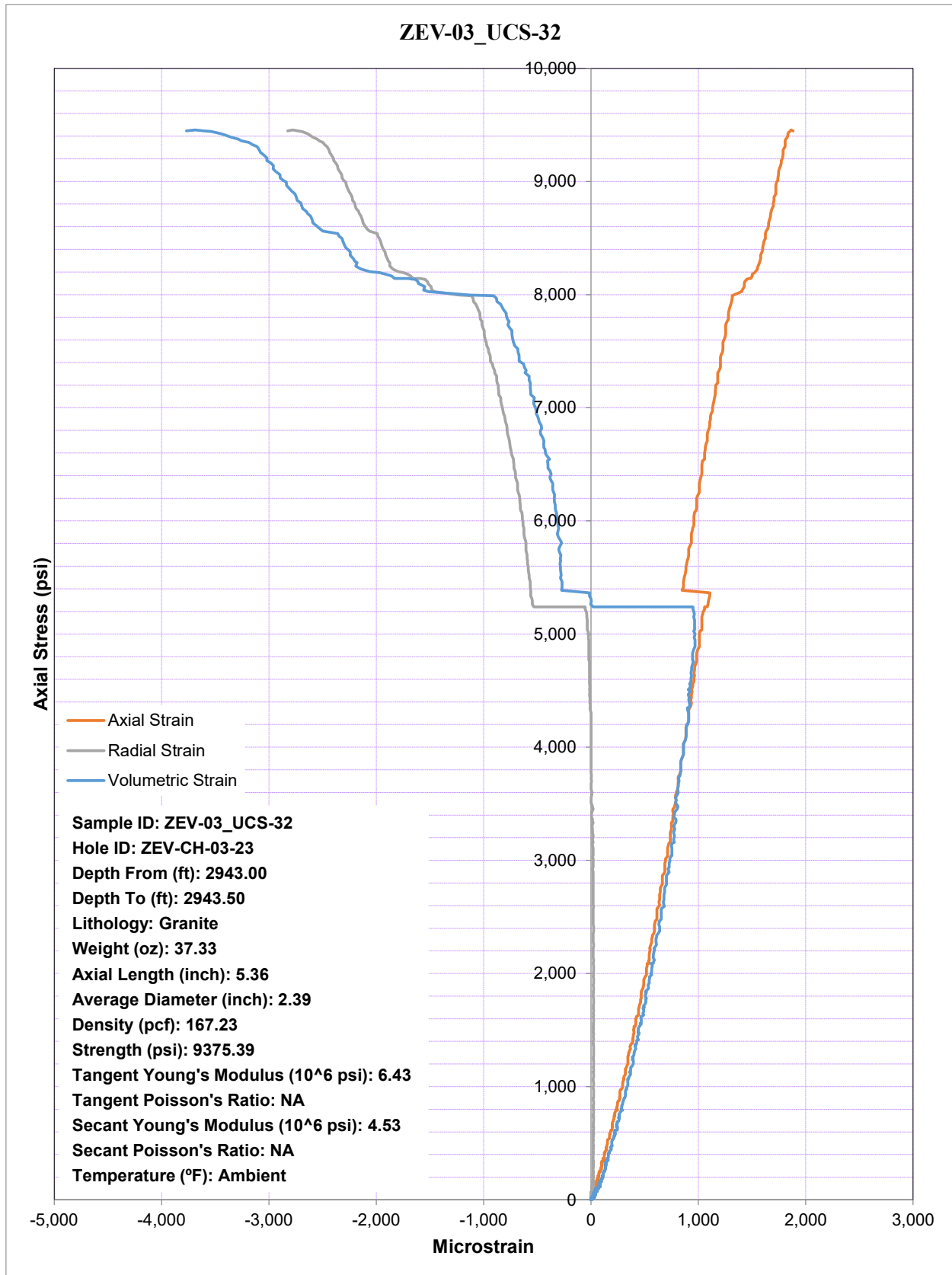


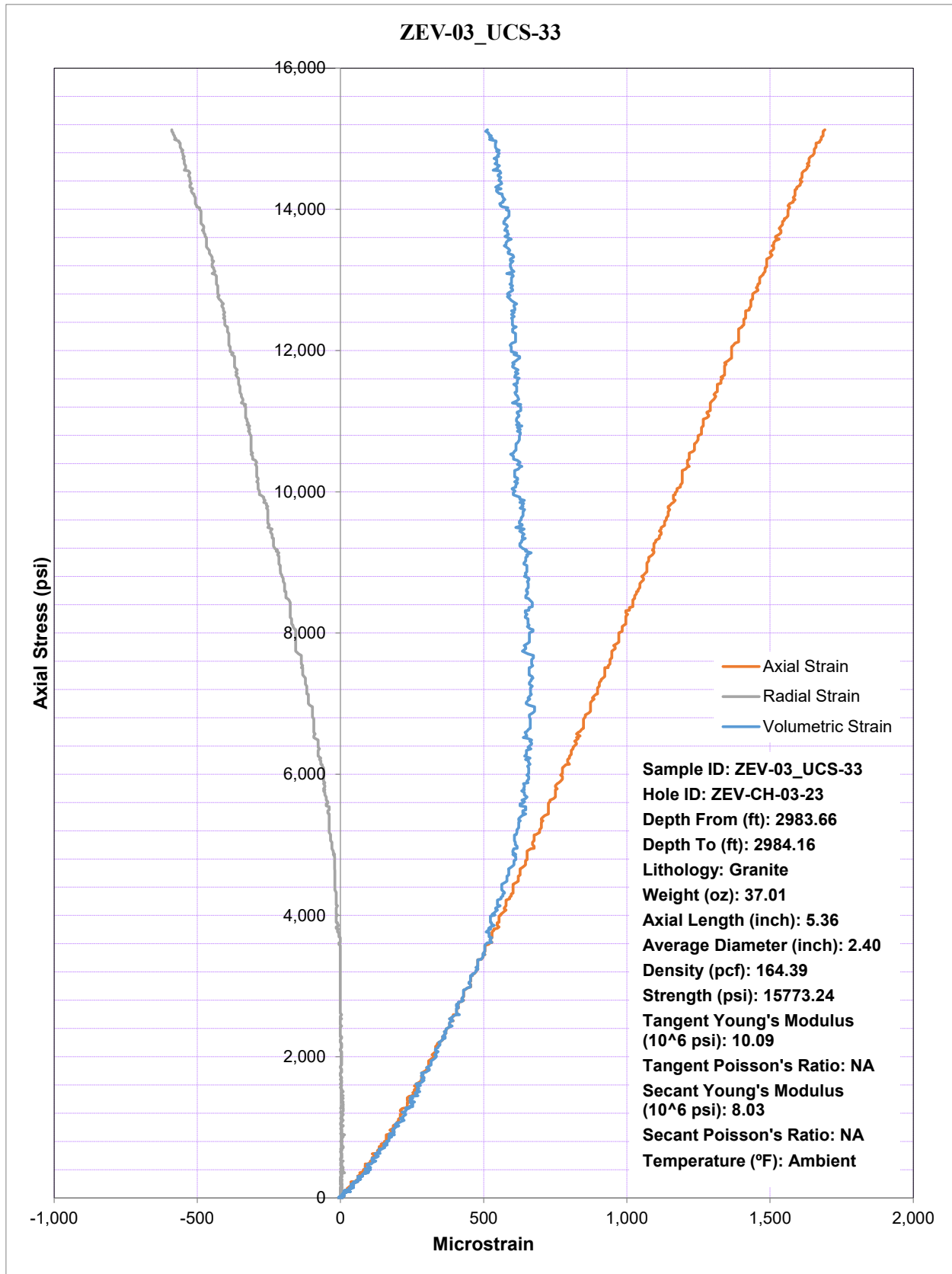












APPENDIX C

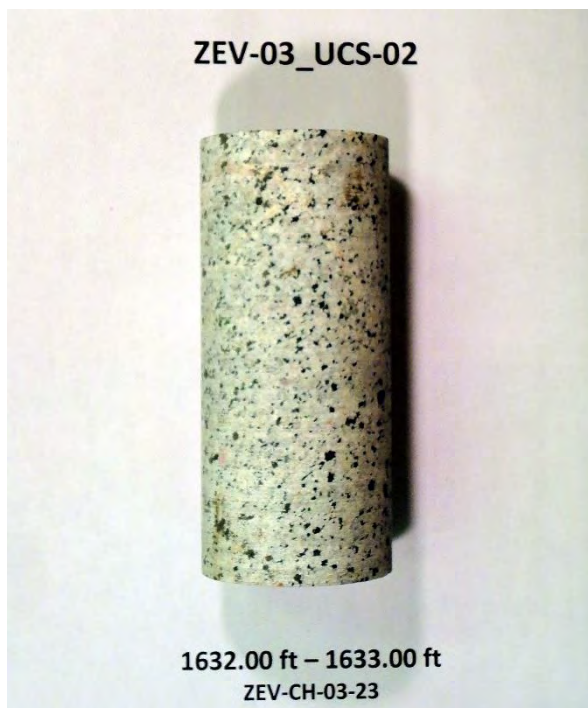
**BEFORE AND AFTER PHOTOGRAPHS
OF UNIAXIAL COMPRESSIVE STRENGTH TEST SPECIMENS**



ZEV-03_UCS-01—Before



ZEV-03_UCS-01—After



ZEV-03_UCS-02—Before



ZEV-03_UCS-02—After

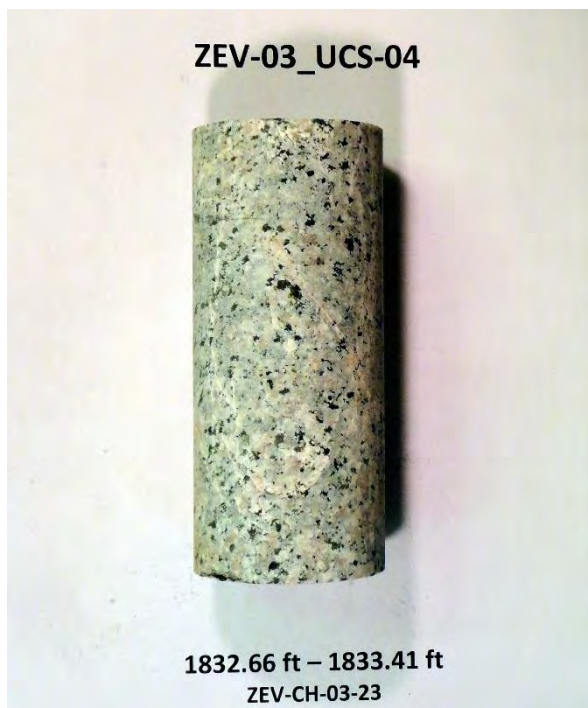
Figure C-1. Before and After Photographs of Uniaxial Compressive Strength Test Specimens



ZEV_03-UCS-03—Before



ZEV-03_UCS-03—After



ZEV-03_UCS-04—Before



ZEV-03_UCS-04—After

Figure C-1. Before and After Photographs of Uniaxial Compressive Strength Test Specimens (continued)



ZEV-03_UCS-05—Before



ZEV-03_UCS-05—After



ZEV-03_UCS-06—Before



ZEV-03_UCS-06—After

Figure C-1. Before and After Photographs of Uniaxial Compressive Strength Test Specimens (continued)



ZEV-03_UCS-07—Before



ZEV-03_UCS-07—After



ZEV-03_UCS-08—Before



ZEV-03_UCS-08—After

Figure C-1. Before and After Photographs of Uniaxial Compressive Strength Test Specimens (continued)



ZEV-03_UCS-09—Before



ZEV-03_UCS-09—After



ZEV-03_UCS-10—Before



ZEV-03_UCS-10—After

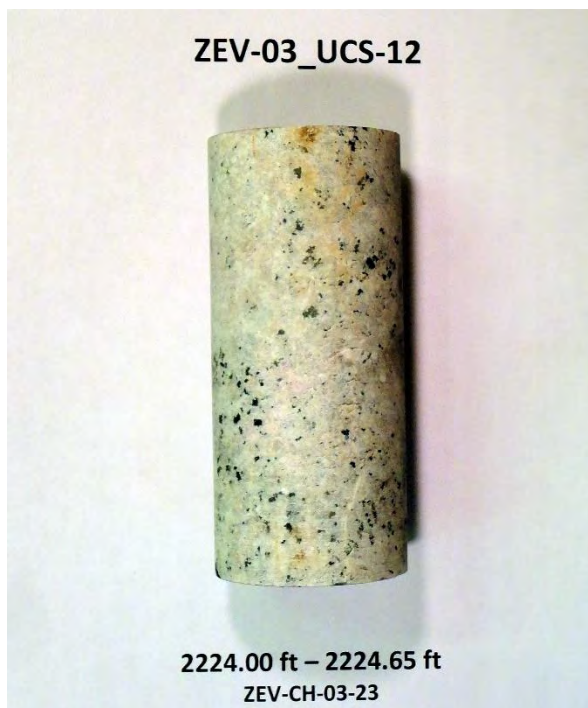
Figure C-1. Before and After Photographs of Uniaxial Compressive Strength Test Specimens (continued)



ZEV-03_UCS-11—Before



ZEV-03_UCS-11—After



ZEV-03_UCS-12—Before



ZEV-03_UCS-12—After

Figure C-1. Before and After Photographs of Uniaxial Compressive Strength Test Specimens (continued)



ZEV-03_UCS-13—Before



ZEV-03_UCS-13—After



ZEV-03_UCS-14—Before



ZEV-03_UCS-14—After

Figure C-1. Before and After Photographs of Uniaxial Compressive Strength Test Specimens



ZEV-03_UCS-15—Before



ZEV-03_UCS-15—After



ZEV-03_UCS-16—Before

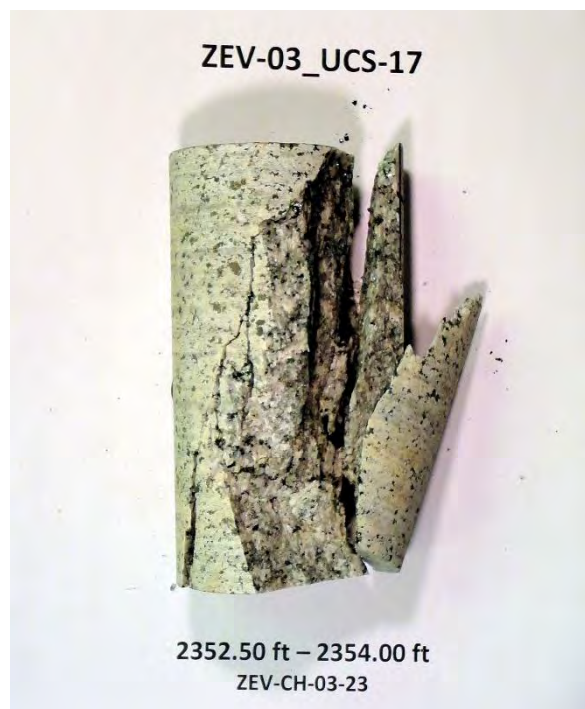


ZEV-03_UCS-16—After

Figure C-1. Before and After Photographs of Uniaxial Compressive Strength Test Specimens *(continued)*



ZEV-03_UCS-17—Before



ZEV-03_UCS-17—After



ZEV-03_UCS-18—Before

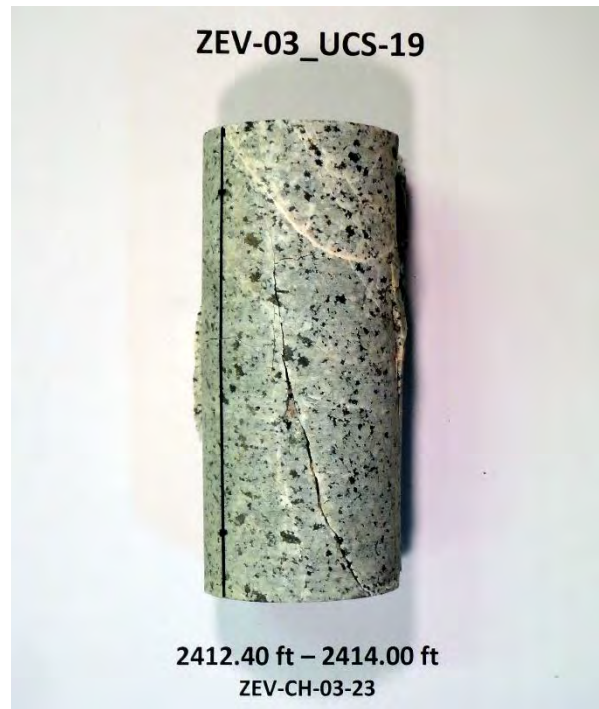


ZEV-03_UCS-18—After

Figure C-1. Before and After Photographs of Uniaxial Compressive Strength Test Specimens *(continued)*



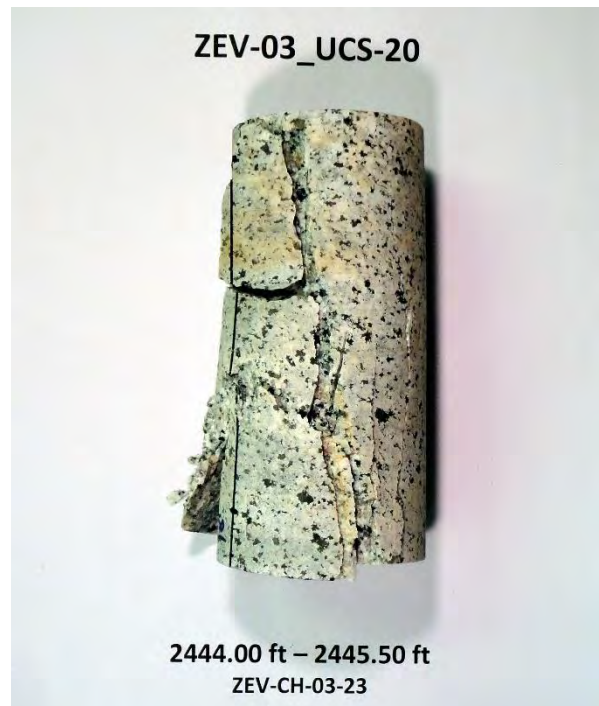
ZEV-03_UCS-19—Before



ZEV-03_UCS-19—After



ZEV-03_UCS-20—Before



ZEV-03_UCS-20—After

Figure C-1. Before and After Photographs of Uniaxial Compressive Strength Test Specimens (continued)



ZEV-03_UCS-21—Before



ZEV-03_UCS-21—After



ZEV-03_UCS-22—Before



ZEV-03_UCS-22—After

Figure C-1. Before and After Photographs of Uniaxial Compressive Strength Test Specimens (continued)



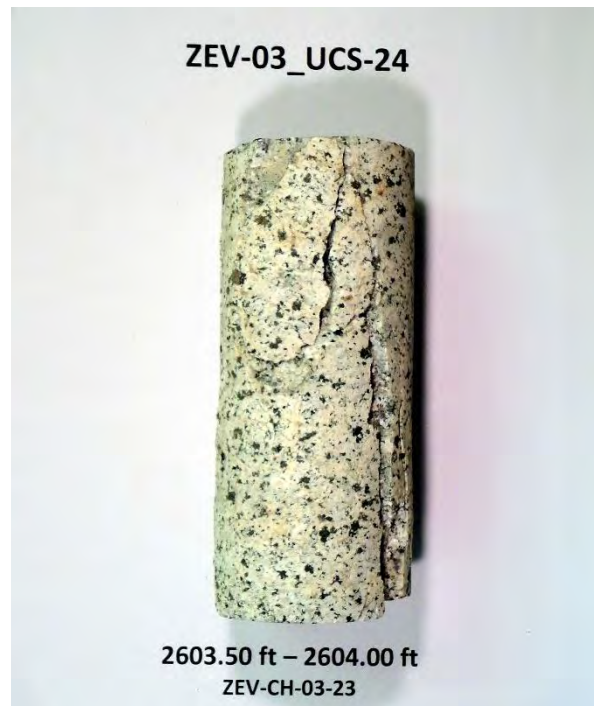
ZEV-03_UCS-23—Before



ZEV-03_UCS-23—After



ZEV-03_UCS-24—Before



ZEV-03_UCS-24—After

Figure C-1. Before and After Photographs of Uniaxial Compressive Strength Test Specimens (continued)



ZEV-03_UCS-25—Before



ZEV-03_UCS-25—After



ZEV-03_UCS-26—Before

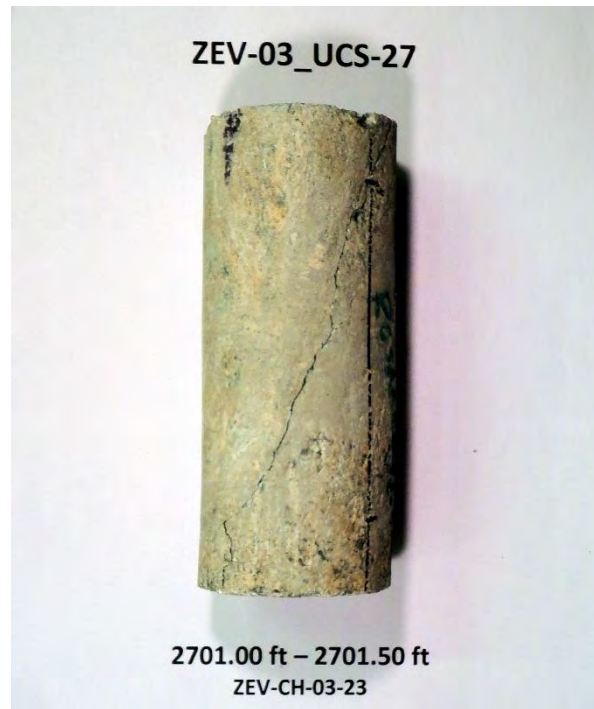


ZEV-03_UCS-26—After

Figure C-1. Before and After Photographs of Uniaxial Compressive Strength Test Specimens (continued)



ZEV-03_UCS-27—Before



ZEV-03_UCS-27—After



ZEV-03_UCS-28—Before



ZEV-03_UCS-28—After

Figure C-1. Before and After Photographs of Uniaxial Compressive Strength Test Specimens *(continued)*



ZEV-03_UCS-29—Before



ZEV-03_UCS-29—After



ZEV-03_UCS-30—Before



ZEV-03_UCS-30—After

Figure C-1. Before and After Photographs of Uniaxial Compressive Strength Test Specimens *(continued)*



ZEV-03_UCS-31—Before



ZEV-03_UCS-31—After

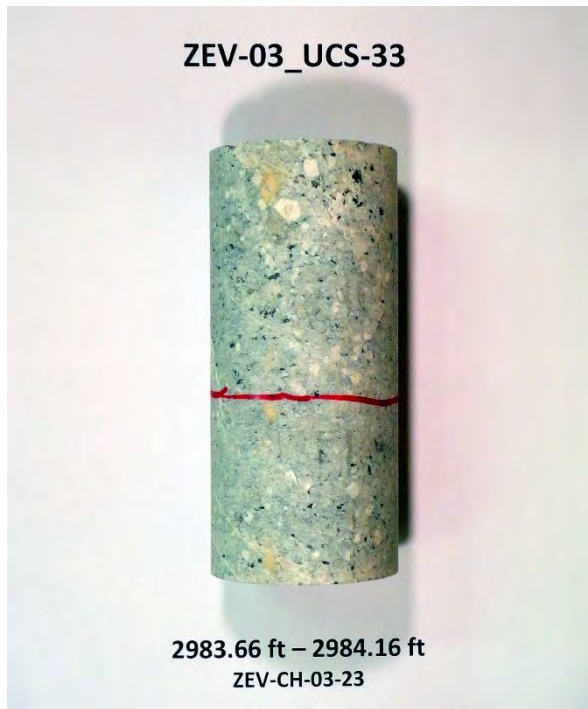


ZEV-03_UCS-32—Before



ZEV-03_UCS-32—After

Figure C-1. Before and After Photographs of Uniaxial Compressive Strength Test Specimens *(continued)*



ZEV-03_UCS-33—Before



ZEV-03_UCS-33—After

Figure C-1. Before and After Photographs of Uniaxial Compressive Strength Test Specimens (*concluded*)

APPENDIX D

TRIAXIAL COMPRESSION TEST DATASHEET



AGAPITO ASSOCIATES, INC.
2913 Hill Avenue, #B
Grand Junction, CO 81504 USA
970-242-4220

TRIAXIAL COMPRESSION TESTS

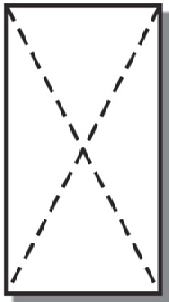
CLIENT	Lane PES
JOB NUMBER	951-20
DATE	October 4, 2023

HOLE NUMBER	ZEV-CH-03-23
MOISTURE CONDITION	As received
TEMPERATURE	Ambient

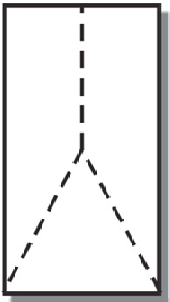
Specimen No.	Hole No.	Interval		Length of Interval (ft)	Weight (oz)	Diameter		Axial Length (in)	Length-to-Diameter Ratio	Area (in ²)	Volume (in ³)	Density (pcf)	Specific Gravity
		From (ft)	To (ft)			D ₁ (in)	D ₂ (in)						
ZEV-03_TRI-01	ZEV-CH-03-23	1,898.70	1,899.70	1.00	36.87	2.38	2.38	5.35	2.3	4.43	23.71	168	2.69
ZEV-03_TRI-02	ZEV-CH-03-23	1,901.20	1,902.00	0.80	36.31	2.38	2.38	5.24	2.2	4.44	23.25	169	2.70
ZEV-03_TRI-03	ZEV-CH-03-23	1,902.00	1,902.80	0.80	34.88	2.37	2.38	5.04	2.1	4.43	22.34	169	2.70
ZEV-03_TRI-04	ZEV-CH-03-23	2,246.00	2,247.70	1.70	35.41	2.40	2.40	5.16	2.2	4.51	23.25	164	2.64
ZEV-03_TRI-05	ZEV-CH-03-23	2,246.00	2,247.70	1.70	34.89	2.40	2.40	5.08	2.1	4.51	22.93	164	2.63
ZEV-03_TRI-06	ZEV-CH-03-23	2,246.00	2,247.70	1.70	36.11	2.40	2.40	5.24	2.2	4.51	23.64	165	2.64
ZEV-03_TRI-07	ZEV-CH-03-23	2,468.70	2,470.00	1.30	34.06	2.39	2.39	4.97	2.1	4.48	22.27	165	2.65
ZEV-03_TRI-08	ZEV-CH-03-23	2,468.70	2,470.00	1.30	34.58	2.39	2.39	4.99	2.1	4.49	22.41	167	2.67
ZEV-03_TRI-09	ZEV-CH-03-23	2,468.70	2,470.00	1.30	33.90	2.39	2.39	5.00	2.1	4.48	22.37	164	2.62
ZEV-03_TRI-10	ZEV-CH-03-23	2,689.00	2,689.75	0.75	37.86	2.39	2.39	5.39	2.3	4.48	24.14	169	2.71
ZEV-03_TRI-11	ZEV-CH-03-23	2,684.00	2,685.00	1.00	37.32	2.39	2.39	5.38	2.3	4.48	24.09	167	2.68
ZEV-03_TRI-12	ZEV-CH-03-23	2,694.75	2,695.25	0.50	38.09	2.39	2.39	5.44	2.3	4.49	24.42	168	2.70
ZEV-03_TRI-13	ZEV-CH-03-23	2,722.50	2,723.00	0.50	41.10	2.40	2.42	5.82	2.4	4.55	26.50	167	2.68
ZEV-03_TRI-14	ZEV-CH-03-23	2,723.33	2,724.16	0.83	37.14	2.40	2.40	5.29	2.2	4.52	23.91	168	2.69
ZEV-03_TRI-15	ZEV-CH-03-23	2,723.33	2,724.16	0.83	36.65	2.40	2.40	5.19	2.2	4.52	23.47	169	2.70
ZEV-03_TRI-16	ZEV-CH-03-23	2,777.66	2,778.33	0.67	33.39	2.40	2.40	5.19	2.2	4.53	23.52	153	2.46

Specimen No.	Lithological Description	Confining Pressure (psi)	Failure Load (lb)	Failure Stress (psi)	Failure Mode Notes	Comments
ZEV-03_TRI-01	Granite	1,500	120,410	27,157	Shear	
ZEV-03_TRI-02	Granite	2,500	202,450	45,641	Axial / Shear	
ZEV-03_TRI-03	Granite	1,500	153,620	34,683	Shear	
ZEV-03_TRI-04	Granite	2,500	227,815	50,516	Shear	
ZEV-03_TRI-05	Granite	1,500	213,135	47,212	Axial / Shear	
ZEV-03_TRI-06	Granite	2,500	269,100	59,658	Axial	
ZEV-03_TRI-07	Granite	1,500	135,535	30,236	Shear	
ZEV-03_TRI-08	Granite	2,500	167,430	37,313	Shear	
ZEV-03_TRI-09	Granite	1,500	144,955	32,387	Axial	
ZEV-03_TRI-10	Granite	2,500	170,725	38,095	Axial	
ZEV-03_TRI-11	Granite	1,500	133,270	29,762	Shear	Sample failed mostly along pre-existing fracture
ZEV-03_TRI-12	Granite	2,500	133,840	29,827	Shear	
ZEV-03_TRI-13	Granite	1,500	-	-	-	Sample broken during preparation
ZEV-03_TRI-14	Granite	2,500	77,475	17,136	Shear	Sample failed mostly along pre-existing fracture
ZEV-03_TRI-15	Granite	1,500	109,320	24,185	Shear	Sample failed mostly along pre-existing fracture
ZEV-03_TRI-16	Granite	2,500	12,370	2,732	Axial	Sample had many pre-existing fractures and was partially decomposed

Failure Mode Sketches



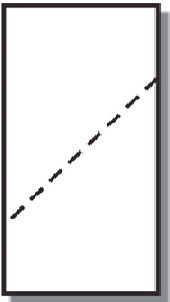
Cone



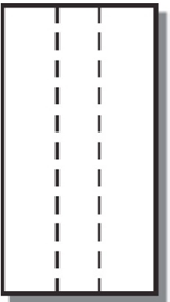
Cone and Axial Fracture/ Columnar



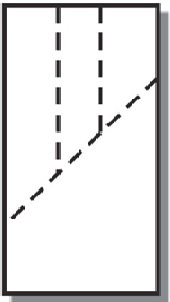
Cone and Shear



Shear



Axial Fracture/ Columnar



Shear and Axial Fracture/ Columnar

APPENDIX E

BEFORE AND AFTER PHOTOGRAPHS OF TRIAXIAL COMPRESSION TEST SPECIMENS



ZEV-03_TRI-01—Before



ZEV-03_TRI-01—After



ZEV-03_TRI-02—Before



ZEV-03_TRI-02—After

Figure E-1. Before and After Photographs of Triaxial Compressive Strength Test Specimens



ZEV-03_TRI-03—Before



ZEV-03_TRI-03—After



ZEV-03_TRI-04—Before



ZEV-03_TRI-04—After

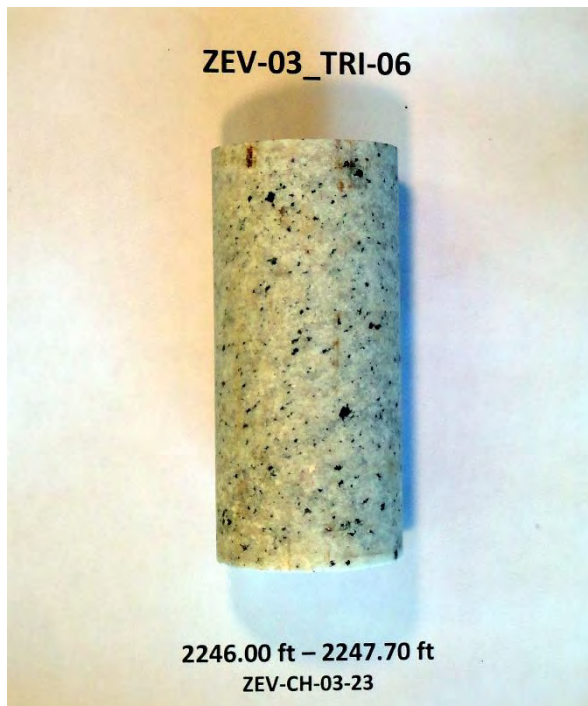
Figure E-1. Before and After Photographs of Triaxial Compressive Strength Test Specimens *(continued)*



ZEV-03_TRI-05—Before



ZEV-03_TRI-05—After



ZEV-03_TRI-06—Before



ZEV-03_TRI-06—After

Figure E-1. Before and After Photographs of Triaxial Compressive Strength Test Specimens *(continued)*



ZEV-03_TRI-07—Before



ZEV-03_TRI-07—After



ZEV-03_TRI-08—Before

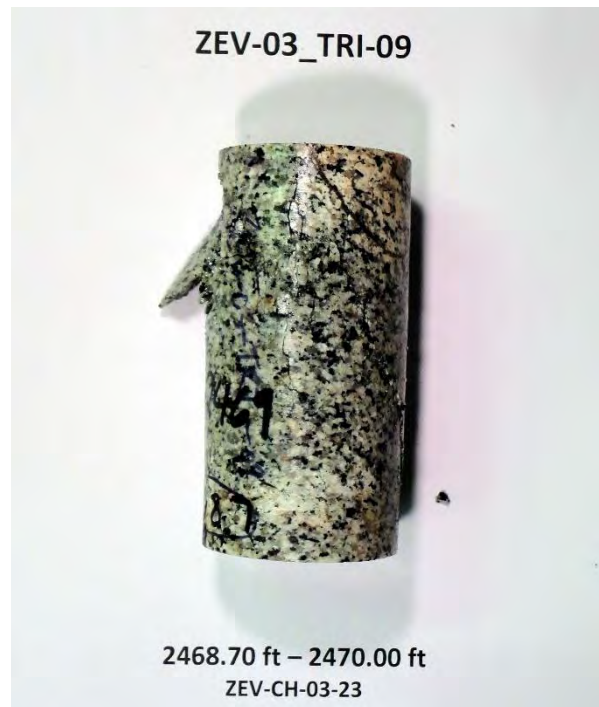


ZEV-03_TRI-08—After

Figure E-1. Before and After Photographs of Triaxial Compressive Strength Test Specimens *(continued)*



ZEV-03_TRI-09—Before



ZEV-03_TRI-09—After



ZEV-03_TRI-10—Before



ZEV-03_TRI-10—After

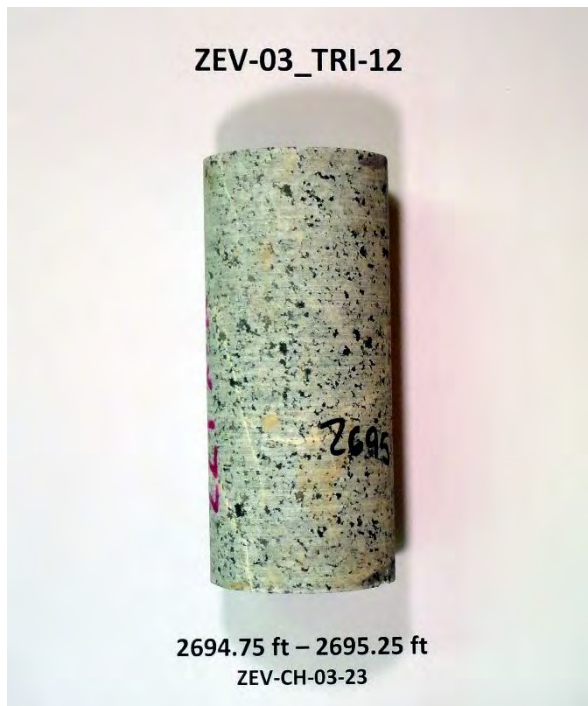
Figure E-1. Before and After Photographs of Triaxial Compressive Strength Test Specimens *(continued)*



ZEV-03_TRI-11—Before



ZEV-03_TRI-11—After



ZEV-03_TRI-12—Before



ZEV-03_TRI-12—After

Figure E-1. Before and After Photographs of Triaxial Compressive Strength Test Specimens *(continued)*



ZEV-03_TRI-14—Before



ZEV-03_TRI-14—After



ZEV-03_TRI-15—Before



ZEV-03_TRI-15—After

Figure E-1. Before and After Photographs of Triaxial Compressive Strength Test Specimens *(continued)*



ZEV-03_TRI-16—Before



ZEV-03_TRI-16—After

Figure E-1. Before and After Photographs of Triaxial Compressive Strength Test Specimens (*concluded*)

APPENDIX F

SPLITTING TENSILE STRENGTH (BRAZILIAN) TEST DATASHEET



AGAPITO ASSOCIATES, INC.
2913 Hill Avenue, #B
Grand Junction, CO 81504 USA
970-242-4220

INDIRECT TENSILE STRENGTH
(BRAZILIAN) TESTS

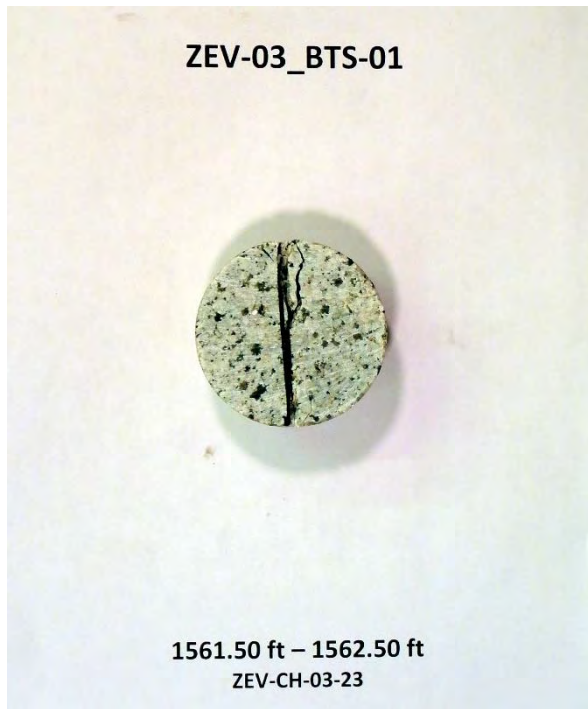
CLIENT	Lane PES	HOLE NUMBER	ZEV-CH-03-23
JOB NUMBER	951-20	MOISTURE CONDITION	As Received
DATE	October 4, 2023	TEMPERATURE	Ambient

Specimen No.	Hole No.	Depth		Length of Interval (ft)	Weight (oz)	Diameter		Axial Length (inch)	Area (inch ²)	Density (pcf)	Specific Gravity
		From (ft)	To (ft)			D ₁ (in)	D ₂ (in)				
ZEV-03_BTS-01	ZEV-CH-03-23	1,561.50	1,562.50	1.00	9.09	2.39	2.39	1.33	4.49	165	2.6
ZEV-03_BTS-02	ZEV-CH-03-23	1,633.08	1,634.41	1.33	9.06	2.37	2.37	1.32	4.43	167	2.7
ZEV-03_BTS-03	ZEV-CH-03-23	1,722.08	1,723.16	1.08	8.81	2.36	2.36	1.33	4.38	164	2.6
ZEV-03_BTS-04	ZEV-CH-03-23	1,878.70	1,879.60	0.90	9.11	2.38	2.37	1.35	4.43	164	2.6
ZEV-03_BTS-05	ZEV-CH-03-23	1,913.80	1,914.60	0.80	8.94	2.37	2.37	1.31	4.40	168	2.7
ZEV-03_BTS-06	ZEV-CH-03-23	2,019.90	2,020.40	0.50	8.46	2.40	2.40	1.21	4.51	168	2.7
ZEV-03_BTS-07	ZEV-CH-03-23	2,074.60	2,076.00	1.40	8.92	2.40	2.40	1.28	4.51	166	2.7
ZEV-03_BTS-08	ZEV-CH-03-23	2,092.70	2,093.80	1.10	9.38	2.40	2.40	1.34	4.51	168	2.7
ZEV-03_BTS-09	ZEV-CH-03-23	2,111.40	2,112.00	0.60	8.32	2.40	2.40	1.19	4.52	167	2.7
ZEV-03_BTS-10	ZEV-CH-03-23	2,175.20	2,176.00	0.80	8.89	2.40	2.40	1.28	4.52	166	2.7
ZEV-03_BTS-11	ZEV-CH-03-23	2,193.50	2,195.00	1.50	8.63	2.40	2.40	1.27	4.51	162	2.6
ZEV-03_BTS-12	ZEV-CH-03-23	2,218.00	2,219.00	1.00	8.55	2.39	2.39	1.25	4.49	164	2.6
ZEV-03_BTS-13	ZEV-CH-03-23	2,254.20	2,255.50	1.30	8.79	2.39	2.40	1.28	4.50	165	2.6
ZEV-03_BTS-14	ZEV-CH-03-23	2,276.40	2,277.00	0.60	8.73	2.40	2.40	1.25	4.51	167	2.7
ZEV-03_BTS-15	ZEV-CH-03-23	2,306.60	2,308.20	1.60	7.86	2.39	2.39	1.15	4.49	165	2.6
ZEV-03_BTS-16	ZEV-CH-03-23	2,321.00	2,322.00	1.00	8.54	2.39	2.39	1.22	4.49	168	2.7
ZEV-03_BTS-17	ZEV-CH-03-23	2,352.50	2,354.00	1.50	8.99	2.39	2.39	1.30	4.49	167	2.7
ZEV-03_BTS-18	ZEV-CH-03-23	2,384.50	2,386.00	1.50	9.10	2.39	2.39	1.30	4.50	168	2.7
ZEV-03_BTS-19	ZEV-CH-03-23	2,412.40	2,414.00	1.60	8.10	2.39	2.40	1.17	4.50	166	2.7
ZEV-03_BTS-20	ZEV-CH-03-23	2,438.40	2,440.60	2.20	8.82	2.38	2.38	1.30	4.45	164	2.6
ZEV-03_BTS-21	ZEV-CH-03-23	2,482.83	2,483.25	0.42	9.20	2.39	2.39	1.35	4.48	164	2.6
ZEV-03_BTS-22	ZEV-CH-03-23	2,526.83	2,527.25	0.42	8.99	2.39	2.39	1.30	4.48	166	2.7
ZEV-03_BTS-23	ZEV-CH-03-23	2,559.83	2,560.00	0.17	9.64	2.40	2.40	1.38	4.51	167	2.7
ZEV-03_BTS-24	ZEV-CH-03-23	2,578.83	2,579.58	0.75	9.19	2.40	2.40	1.31	4.51	168	2.7
ZEV-03_BTS-25	ZEV-CH-03-23	2,654.00	2,654.50	0.50	8.91	2.39	2.39	1.28	4.49	167	2.7
ZEV-03_BTS-26	ZEV-CH-03-23	2,677.58	2,678.41	0.83	8.80	2.38	2.38	1.30	4.44	165	2.6
ZEV-03_BTS-27	ZEV-CH-03-23	2,707.00	2,707.50	0.50	8.52	2.38	2.39	1.26	4.47	164	2.6
ZEV-03_BTS-28	ZEV-CH-03-23	2,740.00	2,740.50	0.50	8.96	2.40	2.40	1.30	4.51	165	2.6
ZEV-03_BTS-29	ZEV-CH-03-23	2,759.91	2,760.33	0.42	8.82	2.40	2.40	1.29	4.53	163	2.6
ZEV-03_BTS-30	ZEV-CH-03-23	2,939.58	2,940.00	0.42	8.63	2.38	2.39	1.26	4.46	165	2.6
ZEV-03_BTS-31	ZEV-CH-03-23	2,946.50	2,947.00	0.50	9.39	2.40	2.40	1.35	4.51	167	2.7

Specimen No.	Lithological Description	Failure Load (lb)	Splitting Tensile Strength (psi)	Failure Mode Notes
ZEV-03_BTS-01	Granite	6,815	1,366	Tensile - Center
ZEV-03_BTS-02	Granite	6,602	1,338	Tensile - Center
ZEV-03_BTS-03	Granite	4,563	927	Tensile - Center
ZEV-03_BTS-04	Granite	1,935	384	Shear - Along pre existing fractures
ZEV-03_BTS-05	Granite	5,655	1,163	Tensile - Center, Secondary along pre-existing fracture
ZEV-03_BTS-06	Granite	4,677	1,028	Tensile - Center
ZEV-03_BTS-07	Granite	5,449	1,127	Tensile - Center
ZEV-03_BTS-08	Granite	5,651	1,122	Tensile - Center
ZEV-03_BTS-09	Granite	5,653	1,258	Tensile - Off center, Partially along pre existing fracture
ZEV-03_BTS-10	Granite	2,894	600	Tensile - Off center, Partially along pre existing fracture
ZEV-03_BTS-11	Granite	2,292	478	Tensile - Center, Partially along pre existing fractures
ZEV-03_BTS-12	Granite	6,432	1,370	Tensile - Center
ZEV-03_BTS-13	Granite	5,632	1,172	Tensile - Center
ZEV-03_BTS-14	Granite	3,843	816	Tensile - Center
ZEV-03_BTS-15	Granite	5,999	1,393	Tensile - Center
ZEV-03_BTS-16	Granite	5,475	1,194	Tensile - Center
ZEV-03_BTS-17	Granite	6,151	1,265	Tensile - Center
ZEV-03_BTS-18	Granite	7,440	1,521	Tensile - Center
ZEV-03_BTS-19	Granite	2,157	491	Tensile - Failed partially along pre existing fracture
ZEV-03_BTS-20	Granite	6,288	1,293	Tensile - Center
ZEV-03_BTS-21	Granite	-	-	Sample broke before testing
ZEV-03_BTS-22	Granite	5,114	1,047	Tensile - Center
ZEV-03_BTS-23	Granite	8,503	1,640	Tensile - Slight off center. Multiple secondary fractures along
ZEV-03_BTS-24	Granite	6,083	1,232	Tensile - Slight off center.
ZEV-03_BTS-25	Granite	7,396	1,535	Tensile - Slight off center
ZEV-03_BTS-26	Granite	6,800	1,405	Tensile - Center
ZEV-03_BTS-27	Granite	6,987	1,481	Tensile - Center
ZEV-03_BTS-28	Granite	5,150	1,053	Tensile - Slight off center
ZEV-03_BTS-29	Granite	2,639	542	Tensile - Center
ZEV-03_BTS-30	Granite	3,887	822	Tensile - Center; Secondary failure along PE fracture
ZEV-03_BTS-31	Granite	7,537	1,485	Tensile - Slight off center

APPENDIX G

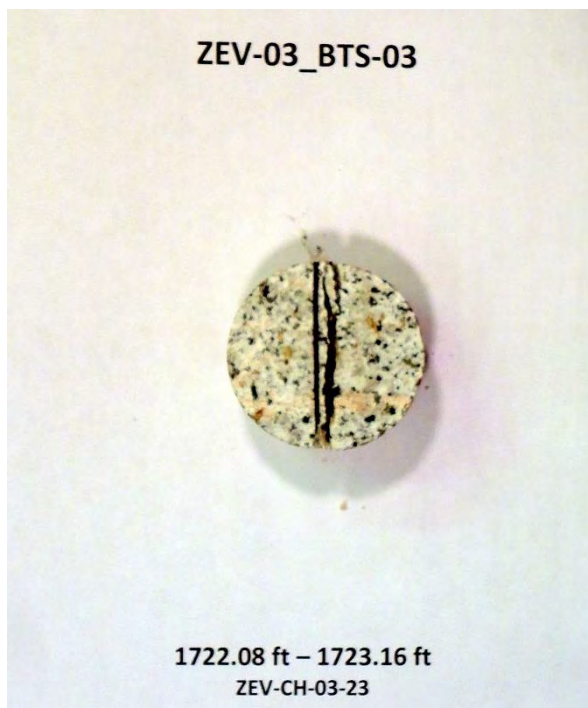
AFTER PHOTOGRAPHS OF BRAZILIAN TEST SPECIMENS



ZEV-03_BTS-01—After



ZEV-03_BTS-02—After

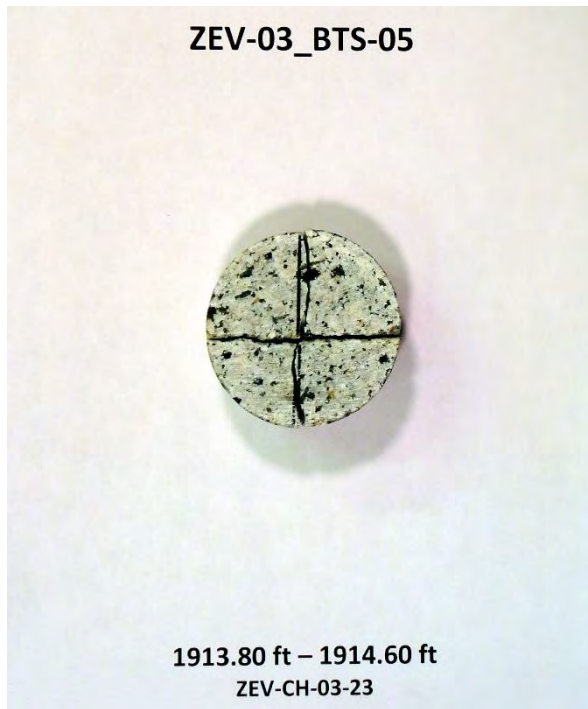


ZEV-03_BTS-03—After

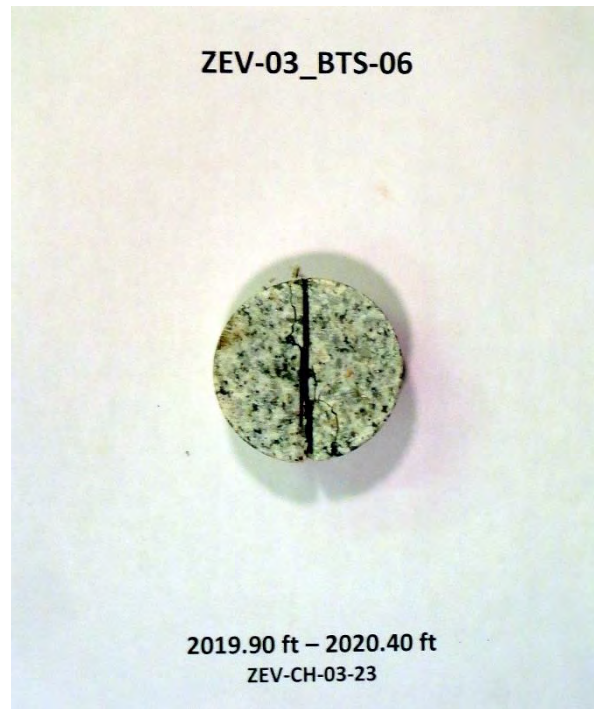


ZEV-03_BTS-04—After

Figure G-1. After Photographs of Brazilian Test Specimens



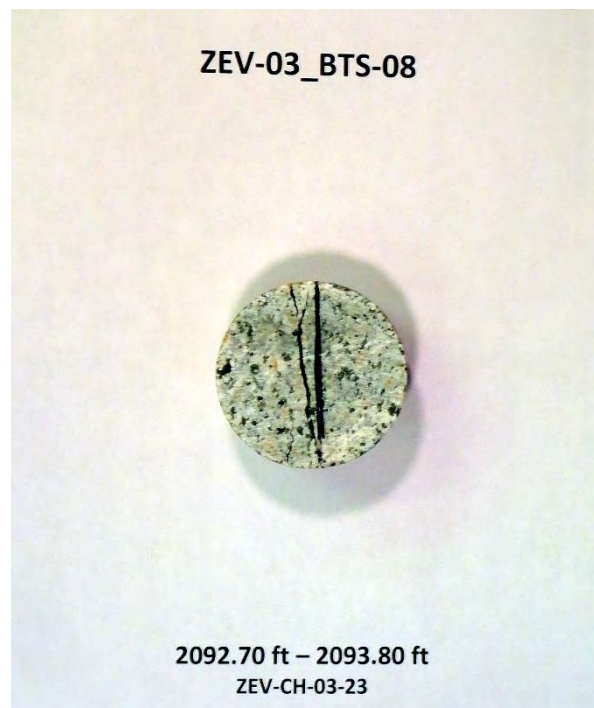
ZEV-03_BTS-05—After



ZEV-03_BTS-06—After

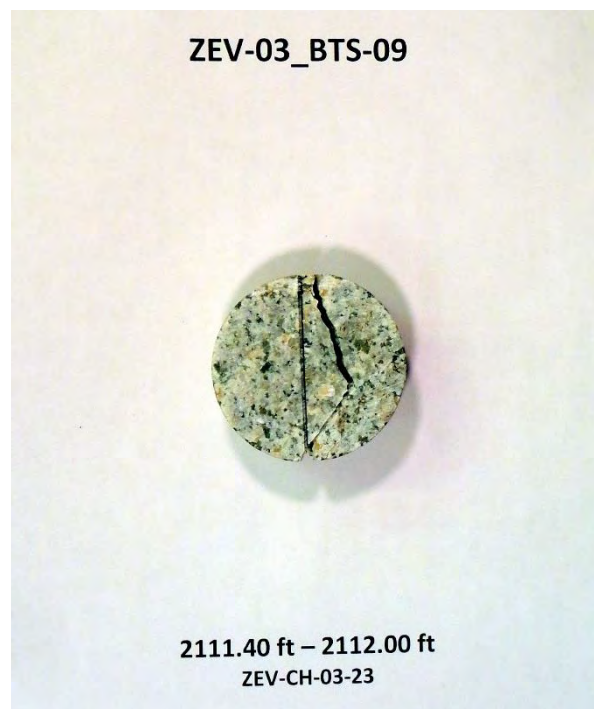


ZEV-03_BTS-07—After

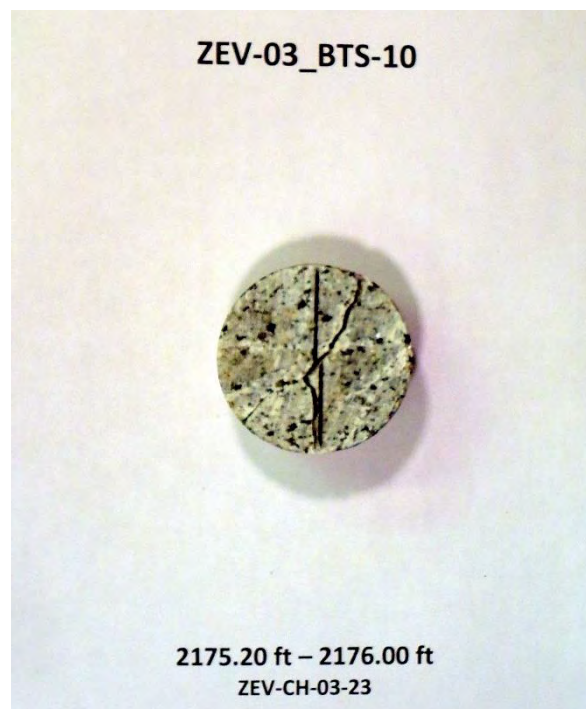


ZEV-03_BTS-08—After

Figure G-1. After Photographs of Brazilian Test Specimens *(continued)*



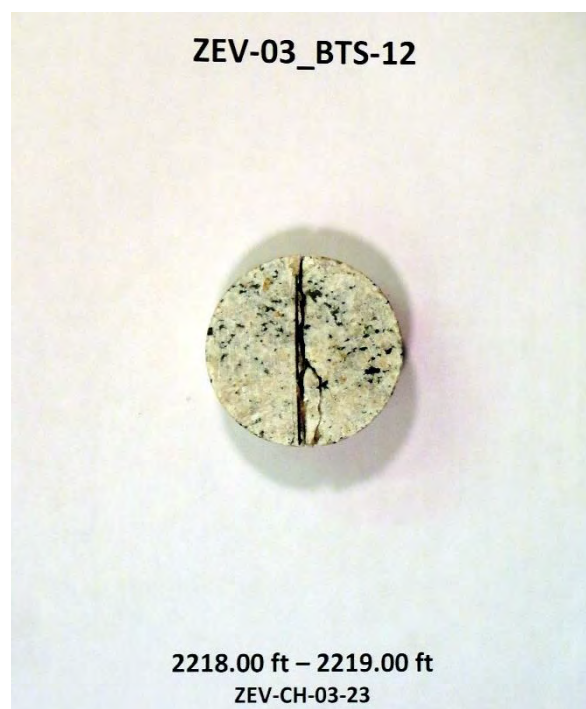
ZEV-03_BTS-09—After



ZEV-03_BTS-10—After



ZEV-03_BTS-11—After

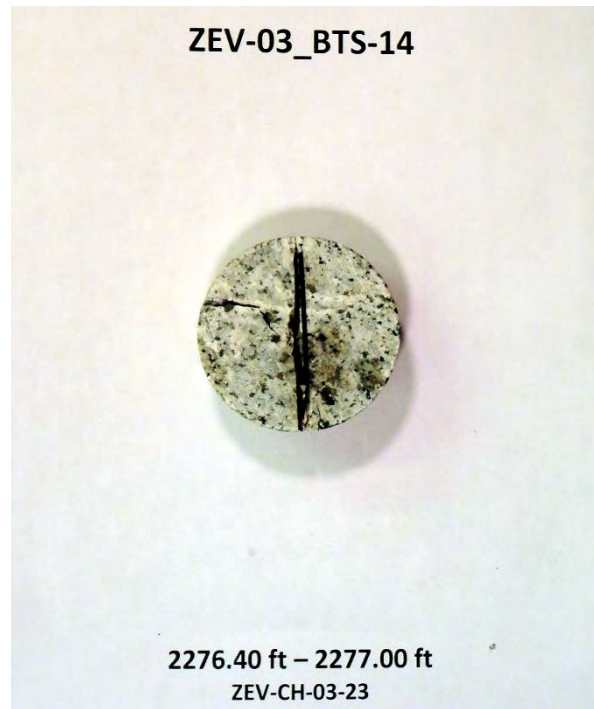


ZEV-03_BTS-12—After

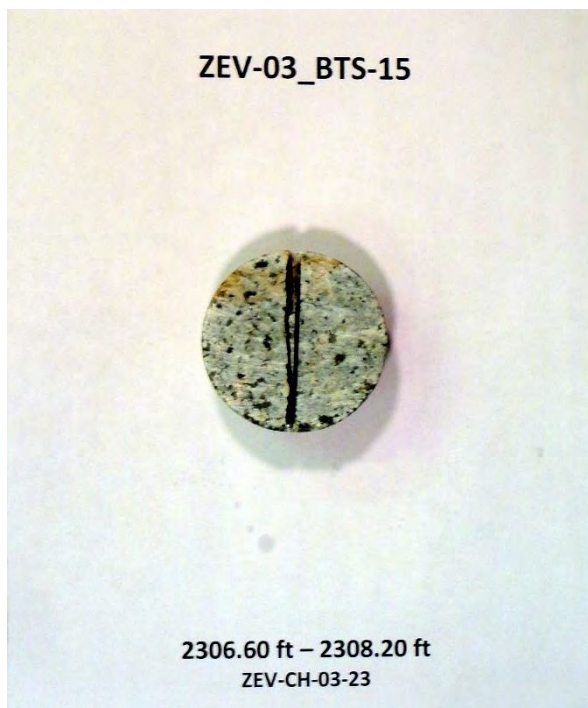
Figure G-1. After Photographs of Brazilian Test Specimens *(continued)*



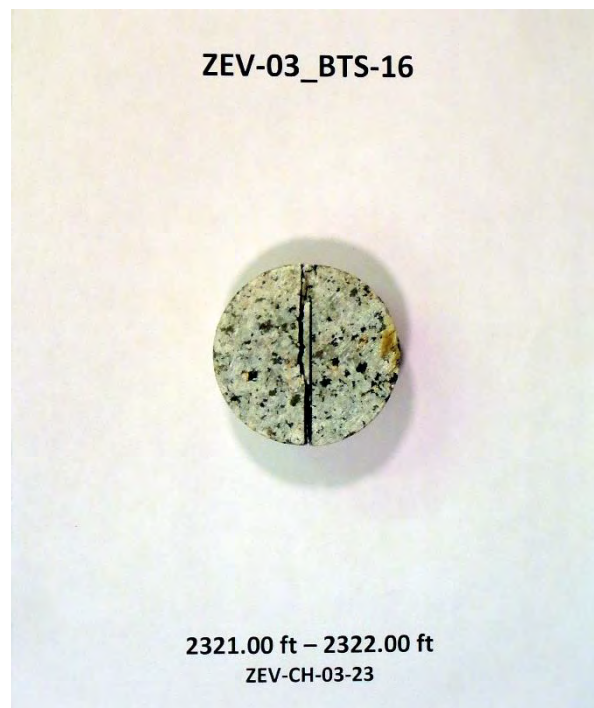
ZEV-03_BTS-13—After



ZEV-03_BTS-14—After



ZEV-03_BTS-15—After

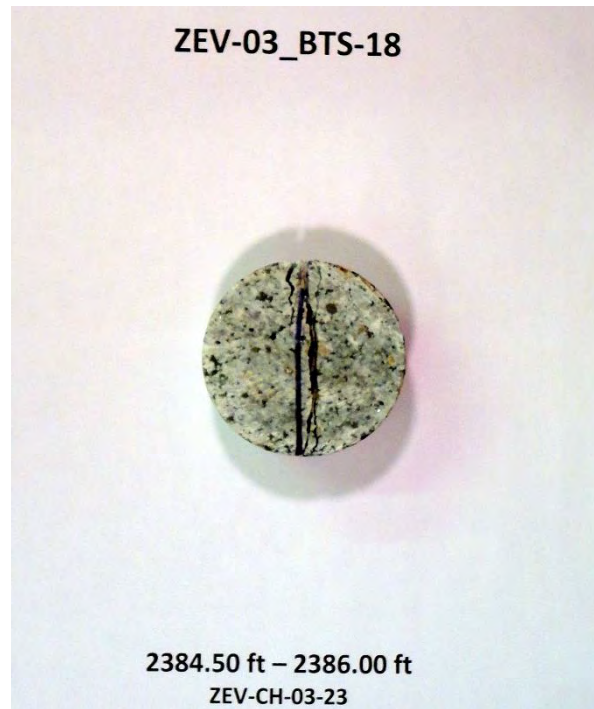


ZEV-03_BTS-16—After

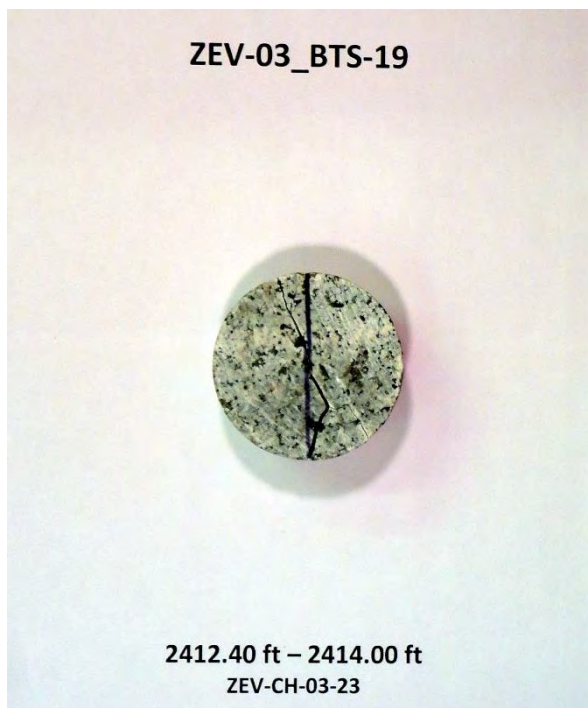
Figure G-1. After Photographs of Brazilian Test Specimens *(continued)*



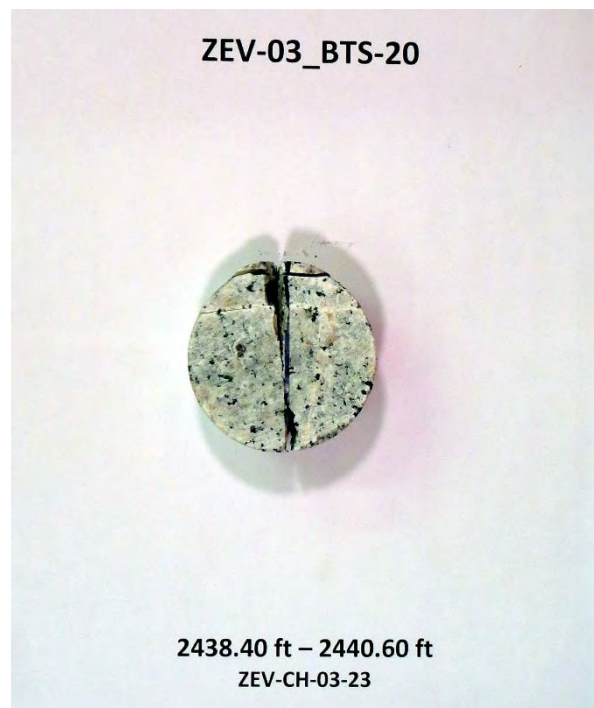
ZEV-03_BTS-17—After



ZEV-03_BTS-18—After

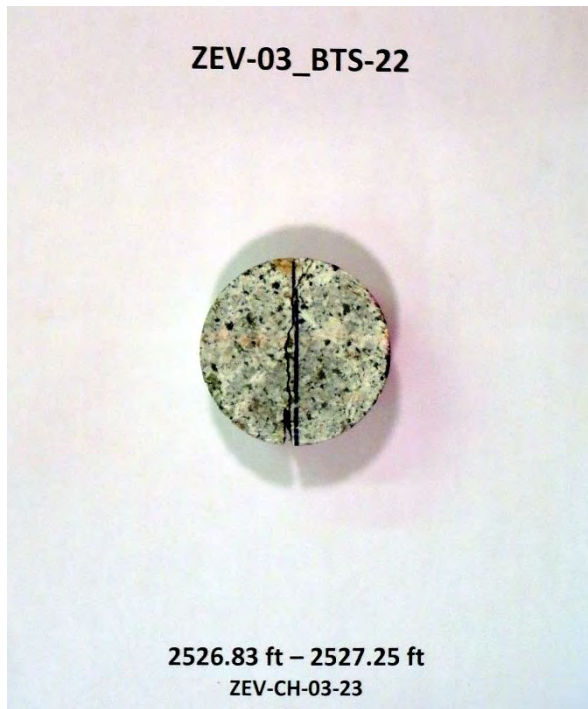


ZEV-03_BTS-19—After

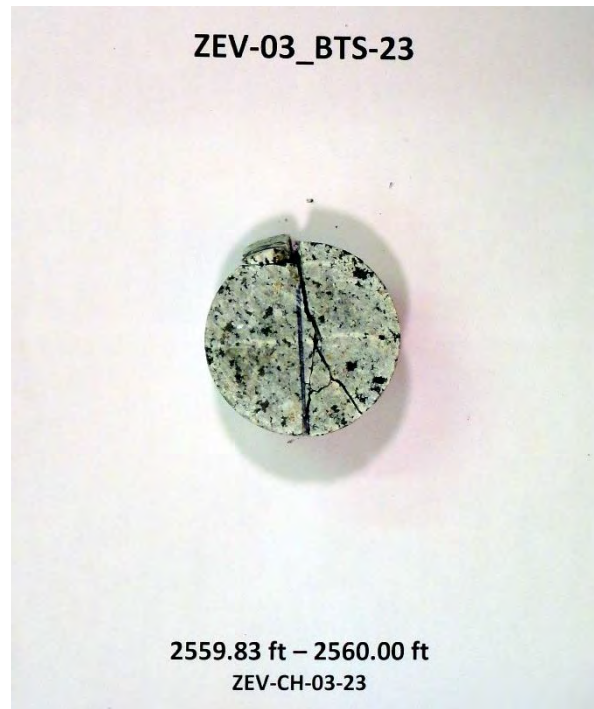


ZEV-03_BTS-20—After

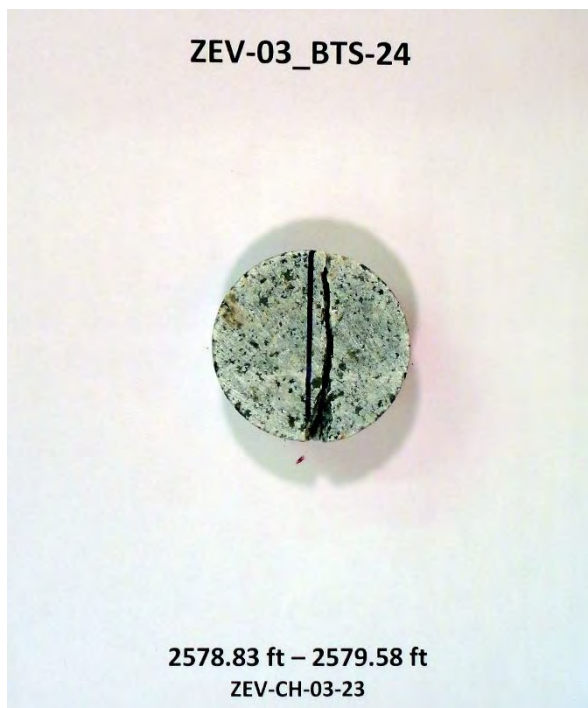
Figure G-1. After Photographs of Brazilian Test Specimens *(continued)*



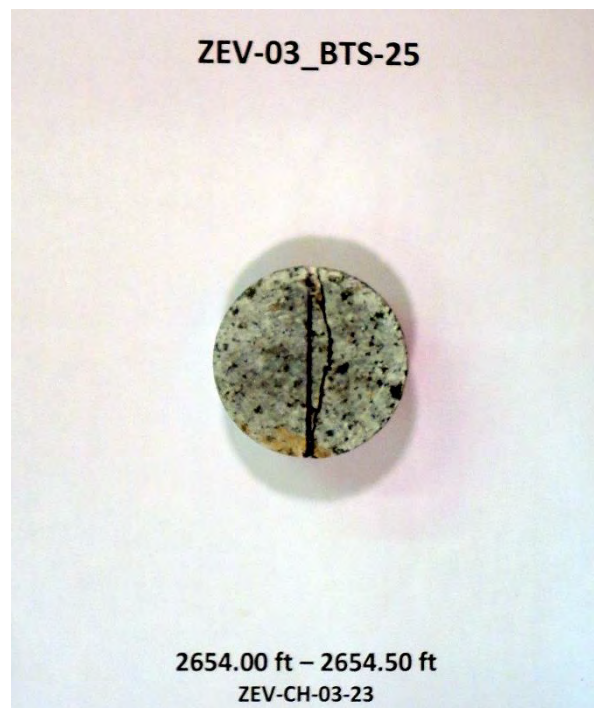
ZEV-03_BTS-22—After



ZEV-03_BTS-23—After

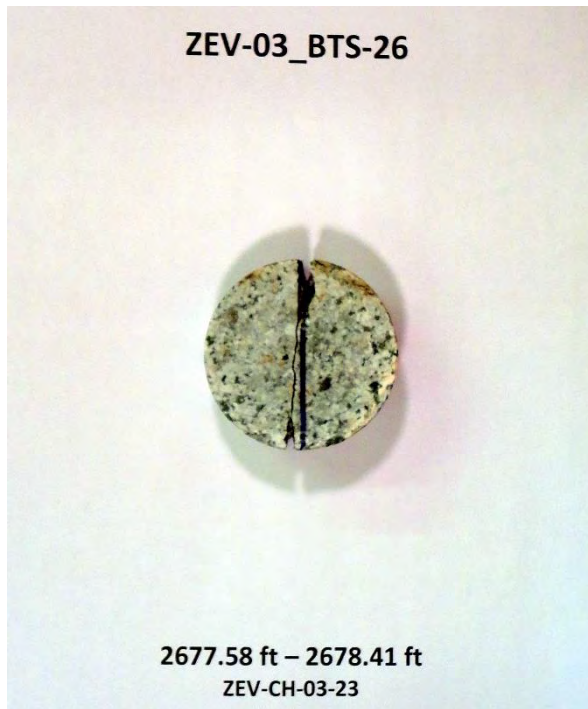


ZEV-03_BTS-24—After

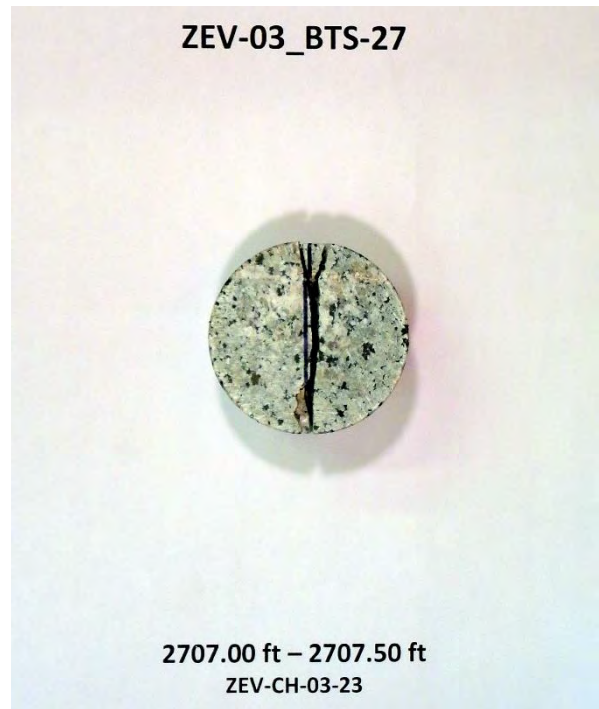


ZEV-03_BTS-25—After

Figure G-1. After Photographs of Brazilian Test Specimens *(continued)*



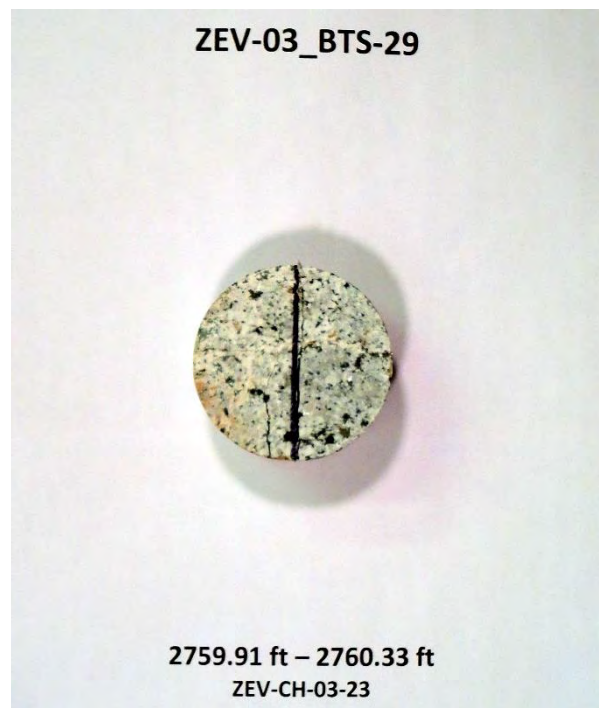
ZEV-03_BTS-26—After



ZEV-03_BTS-27—After



ZEV-03_BTS-28—After

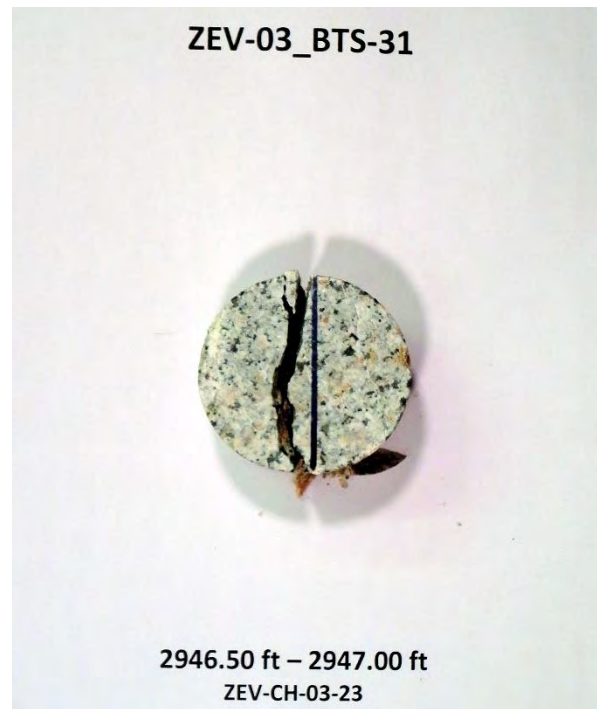


ZEV-03_BTS-29—After

Figure G-1. After Photographs of Brazilian Test Specimens *(continued)*



ZEV-03_BTS-30—After



ZEV-03_BTS-31—After

Figure G-1. After Photographs of Brazilian Test Specimens *(concluded)*

APPENDIX H

POINT LOAD TEST DATASHEET



AGAPITO ASSOCIATES, INC.
2913 Hill Avenue, #B
Grand Junction, CO 81504 USA
970-242-4220

POINT LOAD TESTS

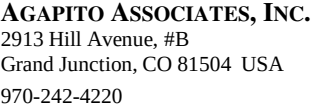
CLIENT	Lane PES
JOB NUMBER	951-20
DATE	October 4, 2023

HOLE NUMBER	ZEV-CH-03-23
MOISTURE CONDITION	As Received
TEMPERATURE	Ambient

Sample No.	Hole ID	Depth From (ft)	Depth To (ft)	Failure Gauge Pressure (psi)	"Failure Load" P (lb)	Test Orientation (A/D)	D (inch)	W (inch)	D _e (inch)	D _e ² (inch ²)	Uncorrected Point Load Index, I _s (psi)	Size Correction Factor, F	Corrected Point Load Index, I _{s(50)} (psi)	UCS (psi)	Lithology
ZEV-03_PLT-01	ZEV-CH-03-23	1539.25	1540.50	196	340	A	2.40	1.02	1.77	3.13	109	0.95	104	2,175	Granite
ZEV-03_PLT-02	ZEV-CH-03-23	1576.00	1577.00	2,574	4,463	A	2.39	1.10	1.83	3.35	1,331	0.97	1,288	27,051	Granite
ZEV-03_PLT-03	ZEV-CH-03-23	1598.75	1599.66	1,755	3,043	A	2.38	1.20	1.91	3.64	835	0.99	824	17,300	Granite
ZEV-03_PLT-04	ZEV-CH-03-23	1616.33	1617.00	2,405	4,170	A	2.39	1.18	1.90	3.60	1,160	0.98	1,140	23,946	Granite
ZEV-03_PLT-05	ZEV-CH-03-23	1617.08	1618.00	2,276	3,946	A	2.39	1.16	1.88	3.54	1,116	0.98	1,093	22,956	Granite
ZEV-03_PLT-06	ZEV-CH-03-23	1618.18	1619.00	1,043	1,808	A	2.39	1.20	1.91	3.65	495	0.99	488	10,256	Granite
ZEV-03_PLT-07	ZEV-CH-03-23	1701.25	1701.75	2,686	4,657	A	2.37	1.18	1.89	3.56	1,307	0.98	1,282	26,928	Granite
ZEV-03_PLT-08	ZEV-CH-03-23	1701.83	1702.33	1,443	2,502	A	2.37	1.12	1.84	3.39	739	0.97	717	15,056	Granite
ZEV-03_PLT-09	ZEV-CH-03-23	1778.16	1779.00	1,875	3,251	A	2.35	1.18	1.88	3.54	919	0.98	900	18,906	Granite
ZEV-03_PLT-10	ZEV-CH-03-23	1851.00	1851.70	924	1,602	A	2.39	1.18	1.90	3.60	445	0.98	438	9,194	Granite
ZEV-03_PLT-11	ZEV-CH-03-23	1869.30	1869.70	1,929	3,344	A	2.39	1.18	1.90	3.60	930	0.98	914	19,201	Granite
ZEV-03_PLT-12	ZEV-CH-03-23	1869.70	1870.10	1,881	3,261	A	2.39	1.20	1.91	3.66	892	0.99	880	18,484	Granite
ZEV-03_PLT-13	ZEV-CH-03-23	1921.60	1922.00	2,295	3,979	A	2.37	1.14	1.86	3.45	1,153	0.97	1,123	23,586	Granite
ZEV-03_PLT-14	ZEV-CH-03-23	1922.00	1922.40	3,021	5,238	A	2.37	1.16	1.87	3.51	1,492	0.98	1,459	30,638	Granite
ZEV-03_PLT-15	ZEV-CH-03-23	1922.40	1922.80	2,567	4,451	A	2.37	1.24	1.94	3.75	1,188	0.99	1,179	24,755	Granite
ZEV-03_PLT-16	ZEV-CH-03-23	2020.40	2021.00	2,212	3,835	A	2.39	1.20	1.91	3.66	1,047	0.99	1,034	21,717	Granite
ZEV-03_PLT-17	ZEV-CH-03-23	2077.00	2077.60	2,190	3,797	A	2.40	1.16	1.88	3.54	1,072	0.98	1,050	22,056	Granite
ZEV-03_PLT-18	ZEV-CH-03-23	2105.50	2106.00	1,330	2,306	A	2.40	1.20	1.92	3.67	629	0.99	621	13,041	Granite
ZEV-03_PLT-19	ZEV-CH-03-23	2111.40	2112.00	1,827	3,168	A	2.40	1.10	1.83	3.37	941	0.97	912	19,145	Granite
ZEV-03_PLT-20	ZEV-CH-03-23	2175.20	2176.00	737	1,278	A	2.40	1.18	1.90	3.61	354	0.98	349	7,322	Granite
ZEV-03_PLT-21	ZEV-CH-03-23	2196.00	2196.60	397	688	A	2.40	1.16	1.88	3.54	194	0.98	190	3,999	Granite
ZEV-03_PLT-22	ZEV-CH-03-23	2218.00	2219.00	2,293	3,976	A	2.39	1.18	1.90	3.60	1,105	0.98	1,086	22,816	Granite
ZEV-03_PLT-23	ZEV-CH-03-23	2237.00	2237.70	3,001	5,203	A	2.40	1.22	1.93	3.72	1,398	0.99	1,385	29,088	Granite
ZEV-03_PLT-24	ZEV-CH-03-23	2254.20	2255.50	1,653	2,866	A	2.39	1.20	1.91	3.66	783	0.99	773	16,231	Granite
ZEV-03_PLT-25	ZEV-CH-03-23	2271.60	2272.65	631	1,094	A	2.40	1.18	1.90	3.60	304	0.98	299	6,275	Granite
ZEV-03_PLT-26	ZEV-CH-03-23	2306.60	2308.20	1,427	2,474	A	2.39	1.14	1.86	3.48	712	0.98	695	14,585	Granite
ZEV-03_PLT-27	ZEV-CH-03-23	2321.00	2322.00	2,292	3,974	A	2.39	1.14	1.86	3.47	1,144	0.98	1,116	23,433	Granite
ZEV-03_PLT-28	ZEV-CH-03-23	2352.50	2354.00	2,138	3,707	A	2.39	1.18	1.90	3.59	1,031	0.98	1,014	21,295	Granite
ZEV-03_PLT-29	ZEV-CH-03-23	2389.20	2390.00	2,521	4,371	A	2.39	1.20	1.91	3.66	1,195	0.99	1,180	24,770	Granite
ZEV-03_PLT-30	ZEV-CH-03-23	2417.00	2418.50	2,119	3,674	A	2.37	1.18	1.89	3.56	1,033	0.98	1,013	21,271	Granite
ZEV-03_PLT-31	ZEV-CH-03-23	2451.60	2453.00	354	614	A	2.39	1.24	1.94	3.78	162	0.99	161	3,391	Granite
ZEV-03_PLT-32	ZEV-CH-03-23	2478.00	2478.40	2,672	4,633	A	2.39	1.24	1.94	3.78	1,227	0.99	1,220	25,618	Granite
ZEV-03_PLT-33	ZEV-CH-03-23	2533.41	2533.83	669	1,160	A	2.40	1.26	1.96	3.85	302	1.00	301	6,323	Granite
ZEV-03_PLT-34	ZEV-CH-03-23	2577.83	2578.33	3,018	5,233	A	2.40	1.28	1.98	3.91	1,339	1.00	1,342	28,174	Granite
ZEV-03_PLT-35	ZEV-CH-03-23	2602.75	2603.33	470	815	A	2.40	1.24	1.95	3.80	215	1.00	214	4,488	Granite
ZEV-03_PLT-36	ZEV-CH-03-23	2631.83	2632.33	944	1,637	A	2.40	1.20	1.91	3.67	447	0.99	441	9,261	Granite
ZEV-03_PLT-37	ZEV-CH-03-23	2654.66	2655.25	1,975	3,424	A	2.39	1.22	1.93	3.72	922	0.99	913	19,171	Granite
ZEV-03_PLT-38	ZEV-CH-03-23	2684.00	2685.00	1,397	2,422	A	2.39	1.20	1.91	3.65	664	0.99	655	13,760	Granite
ZEV-03_PLT-39	ZEV-CH-03-23	2721.16	2721.66	2,408	4,175	A	2.40	1.14	1.87	3.48	1,198	0.98	1,170	24,567	Granite
ZEV-03_PLT-40	ZEV-CH-03-23	2751.75	2752.25	170	295	A	2.40	1.30	1.99	3.97	74	1.01	75	1,569	Granite
ZEV-03_PLT-41	ZEV-CH-03-23	2835.58	2836.00	2,295	3,979	A	2.40	1.26	1.96	3.85	1,034	1.00	1,033	21,688	Granite
ZEV-03_PLT-42	ZEV-CH-03-23	2853.00	2853.58	1,819	3,154	A	2.40	1.24	1.95	3.79	833	0.99	828	17,398	Granite
ZEV-03_PLT-43	ZEV-CH-03-23	2879.00	2880.00	83	144	A	2.40	1.36	2.04	4.15	35	1.02	35	740	Granite
ZEV-03_PLT-44	ZEV-CH-03-23	2939.58	2940.00	1,220	2,115	A	2.38	1.22	1.92	3.70	571	0.99	565	11,870	Granite
ZEV-03_PLT-45	ZEV-CH-03-23	2949.25	2949.50	660	1,144	A	2.40	1.20	1.91	3.67	312	0.99	308	6,475	Granite

APPENDIX I

SLAKE DURABILITY TEST DATASHEET



HOLE NUMBER	ZEV-CH-03-23
MOISTURE CONDITION	As Received
OVEN TEMPERATURE	230 ± 10 °F

CLIENT	Lane PES
JOB NUMBER	951-20
DATE	October 4, 2023

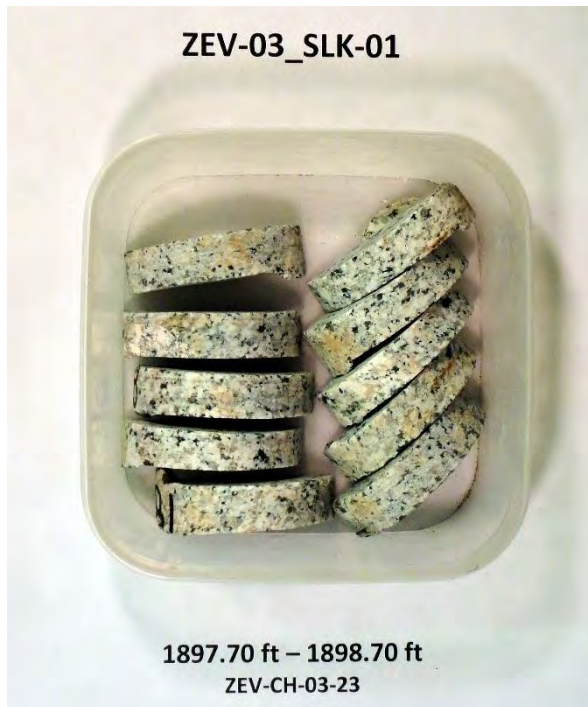
A	Weight of drum plus sample at natural moisture content (oz)
B	Weight of drum plus oven-dried sample before first cycle (oz)
C	Weight of drum (oz)

D	Weight of drum plus oven-dried sample before second cycle (oz)
W_F	Weight of drum plus oven-dried sample after second cycle (oz)

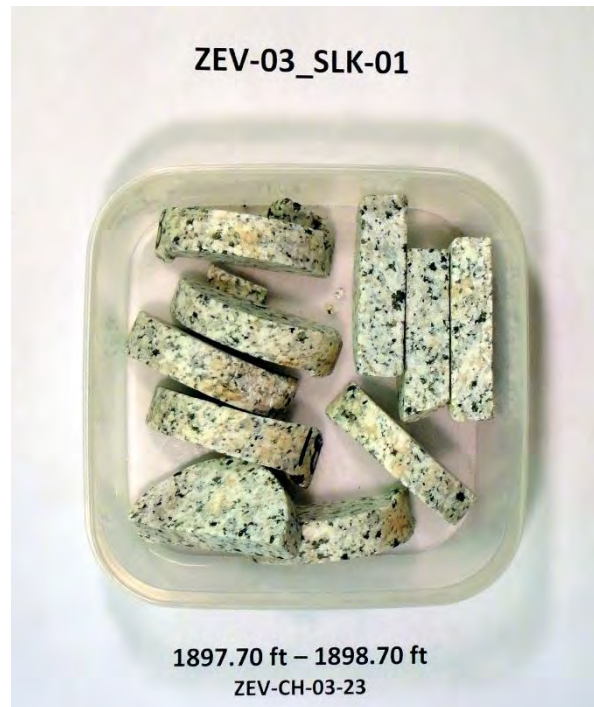
Specimen No.	Hole ID	Interval		Length of Interval (ft)	A (oz)	B (oz)	C (oz)	D (oz)	Water Temperature Cycle 1		W _F (oz)	Water Temperature Cycle 2		Moisture Content (%)	Slake Durability Index I _d (2)	Retained Materials Description	
		From (ft)	To (ft)						Before (°F)	After (°F)		Before (°F)	After (°F)			Lithology	Type*
ZEV-03_SLK-01	ZEV-CH-03-23	1,897.70	1,898.70	1.00	70.86	70.80	53.49	70.75	68.5	71.6	70.74	70.4	76.0	0.3%	99.7	Granite	I
ZEV-03_SLK-02	ZEV-CH-03-23	2,021.70	2,023.10	1.40	71.00	70.96	53.50	70.92	68.5	71.6	70.90	70.4	76.0	0.3%	99.7	Granite	I
ZEV-03_SLK-03	ZEV-CH-03-23	2,074.60	2,076.00	1.40	69.91	69.87	52.76	69.82	68.5	71.6	69.80	70.4	76.0	0.2%	99.6	Granite	I
ZEV-03_SLK-04	ZEV-CH-03-23	2,092.70	2,093.80	1.10	69.75	69.70	53.74	69.62	70.2	72.5	69.6	72.5	77.8	0.3%	99.5	Granite	I
ZEV-03_SLK-05	ZEV-CH-03-23	2,112.00	2,113.00	1.00	74.04	74.00	55.00	73.95	70.2	72.5	73.93	72.5	77.8	0.2%	99.7	Granite	I
ZEV-03_SLK-06	ZEV-CH-03-23	2,151.00	2,151.60	0.60	71.81	71.76	55.41	71.70	70.2	72.5	71.66	72.5	77.8	0.3%	99.4	Granite	I
ZEV-03_SLK-07	ZEV-CH-03-23	2,187.00	2,188.04	1.04	72.64	72.60	55.49	71.81	71.6	72.0	71.46	72.8	74.6	0.2%	93.4	Granite	II
ZEV-03_SLK-08	ZEV-CH-03-23	2,245.00	2,246.00	1.00	69.45	69.42	53.49	69.38	63.8	73.7	69.36	68.6	73.2	0.2%	99.7	Granite	I
ZEV-03_SLK-09	ZEV-CH-03-23	2,277.50	2,279.00	1.50	70.84	70.80	53.49	70.67	63.8	73.7	70.63	68.6	73.2	0.3%	99.0	Granite	I
ZEV-03_SLK-10	ZEV-CH-03-23	2,306.60	2,308.20	1.60	70.51	70.48	52.76	70.43	63.8	73.7	70.40	68.6	73.2	0.2%	99.6	Granite	I
ZEV-03_SLK-11	ZEV-CH-03-23	2,335.00	2,335.80	0.80	72.36	72.32	53.73	72.21	64.4	67.7	72.17	73.2	77.5	0.2%	99.2	Granite	I
ZEV-03_SLK-12	ZEV-CH-03-23	2,359.70	2,360.50	0.80	72.68	72.65	55.00	72.58	64.4	67.7	72.55	73.2	77.5	0.2%	99.4	Granite	I
ZEV-03_SLK-13	ZEV-CH-03-23	2389.20	2390.00	0.80	73.20	73.17	55.41	73.13	64.4	67.7	73.11	73.2	77.5	0.2%	99.7	Granite	I
ZEV-03_SLK-14	ZEV-CH-03-23	2,417.00	2,418.50	1.50	72.25	72.21	55.50	72.17	67.5	68.9	72.16	77.5	77.8	0.2%	99.7	Granite	I
ZEV-03_SLK-15	ZEV-CH-03-23	2,451.60	2,453.00	1.40	71.24	71.21	53.49	71.08	65.8	71.8	71.02	68.3	74.3	0.2%	98.9	Granite	I
ZEV-03_SLK-16	ZEV-CH-03-23	2,486.00	2,487.00	1.00	70.11	70.07	53.50	69.96	65.8	71.8	69.92	68.3	74.3	0.3%	99.1	Granite	I
ZEV-03_SLK-17	ZEV-CH-03-23	2,532.00	2,532.75	0.75	70.03	69.92	52.76	69.22	65.8	71.8	68.96	68.3	74.3	0.6%	94.4	Granite	II
ZEV-03_SLK-18	ZEV-CH-03-23	2,562.83	2,563.58	0.75	72.08	72.05	53.74	71.97	71.8	75.0	71.93	74.3	77.4	0.2%	99.3	Granite	I
ZEV-03_SLK-19	ZEV-CH-03-23	2,604.50	2,605.41	0.91	72.21	72.10	55.00	71.09	71.8	75.0	70.55	74.3	77.4	0.6%	90.9	Granite	II
ZEV-03_SLK-20	ZEV-CH-03-23	2,648.00	2,648.66	0.66	73.82	73.74	55.41	73.47	71.8	75.0	73.36	74.3	77.4	0.4%	97.9	Granite	I
ZEV-03_SLK-21	ZEV-CH-03-23	2,674.50	2,675.50	1.00	72.29	72.26	55.49	72.22	74.5	74.5	72.20	77.4	77.6	0.1%	99.6	Granite	I
ZEV-03_SLK-22	ZEV-CH-03-23	2,717.08	2,717.58	0.50	70.84	70.80	53.49	70.76	63.3	68.9	70.72	64.2	70.8	0.2%	99.5	Granite	I
ZEV-03_SLK-23	ZEV-CH-03-23	2,757.00	2,758.00	1.00	71.14	71.08	53.50	70.82	63.3	68.9	70.64	64.2	70.8	0.3%	97.5	Granite	I
ZEV-03_SLK-24	ZEV-CH-03-23	2,798.58	2,799.58	1.00	70.51	70.39	52.76	69.78	63.3	68.9	69.13	64.2	70.8	0.7%	92.8	Granite	II
ZEV-03_SLK-25	ZEV-CH-03-23	2,867.00	2,867.58	0.58	70.66	70.55	53.73	64.04	68.8	72.1	61.23	70.8	74.2	0.7%	44.6	Granite	II
ZEV-03_SLK-26	ZEV-CH-03-23	2,917.77	2,918.33	0.56	72.60	72.26	55.01	60.98	68.8	72.1	59.13	70.8	74.2	2.0%	23.9	Granite	III
ZEV-03_SLK-27	ZEV-CH-03-23	2,983.66	2,984.16	0.50	71.97	71.92	55.41	71.81	68.8	72.1	71.75	70.8	74.2	0.3%	99.0	Granite	I
* Type I = Retained pieces remained virtually unchanged II = Retained materials consist of large and small pieces III = Retained materials is exclusively small fragments																	

APPENDIX J

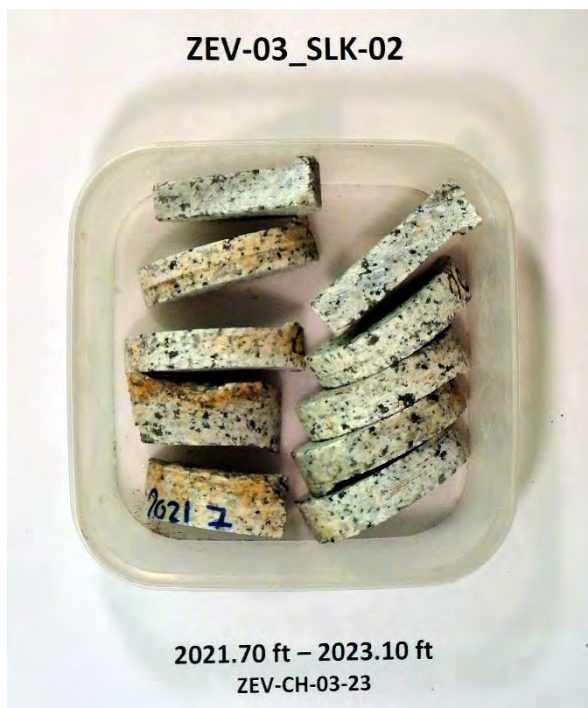
BEFORE AND AFTER PHOTOGRAPHS OF SLAKE DURABILITY TEST SPECIMENS



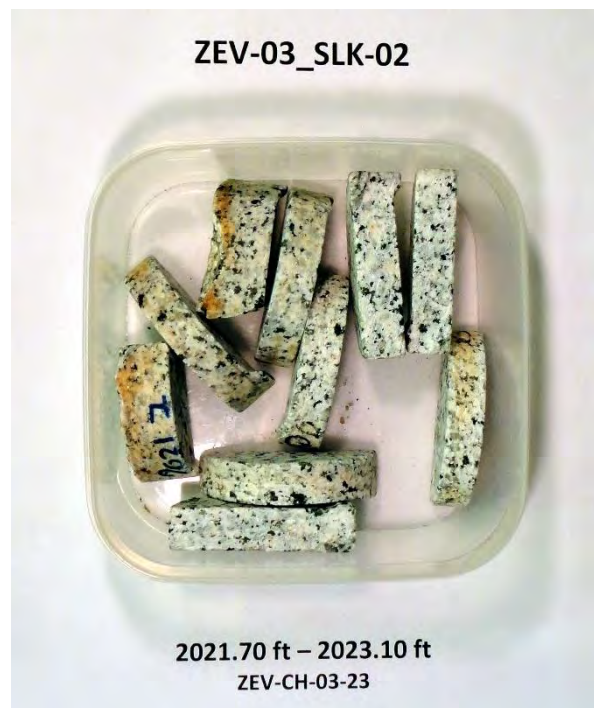
ZEV-03_SLK-01—Before



ZEV-03_SLK-01—After

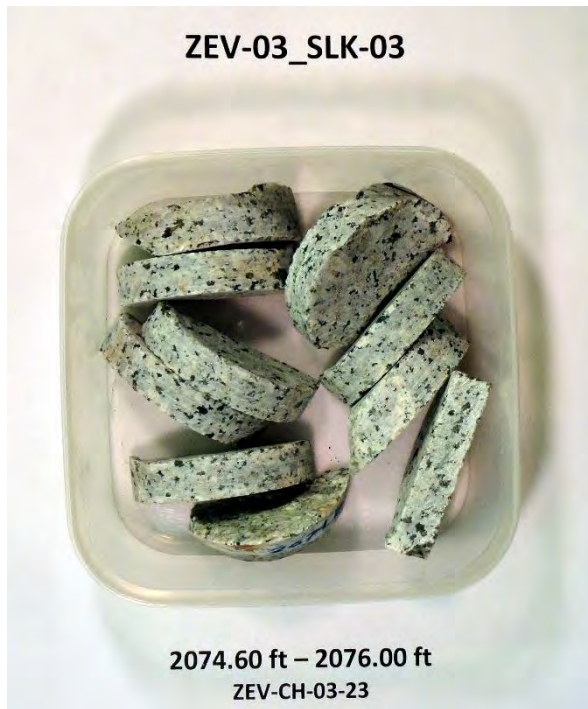


ZEV-03_SLK-02—Before

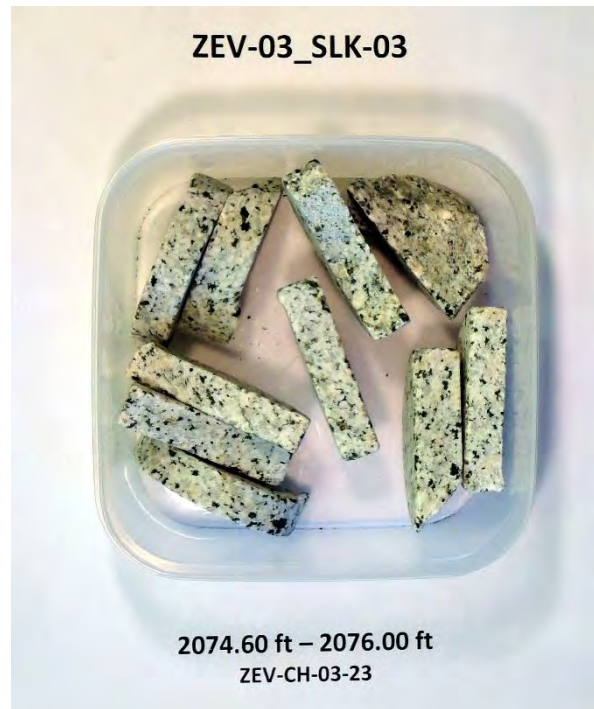


ZEV-03_SLK-02—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens



ZEV-03_SLK-03—Before



ZEV-03_SLK-03—After



ZEV-03_SLK-04—Before



ZEV-03_SLK-04—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



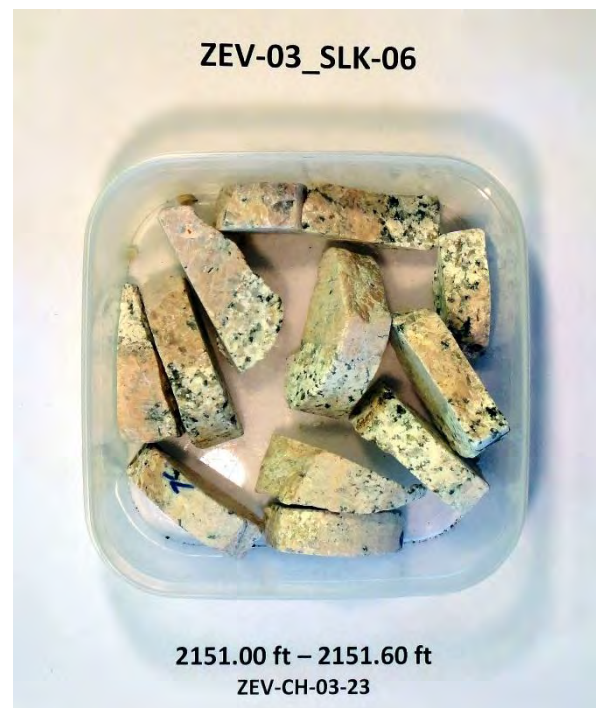
ZEV-03_SLK-05—Before



ZEV-03_SLK-05—After



ZEV-03_SLK-06—Before

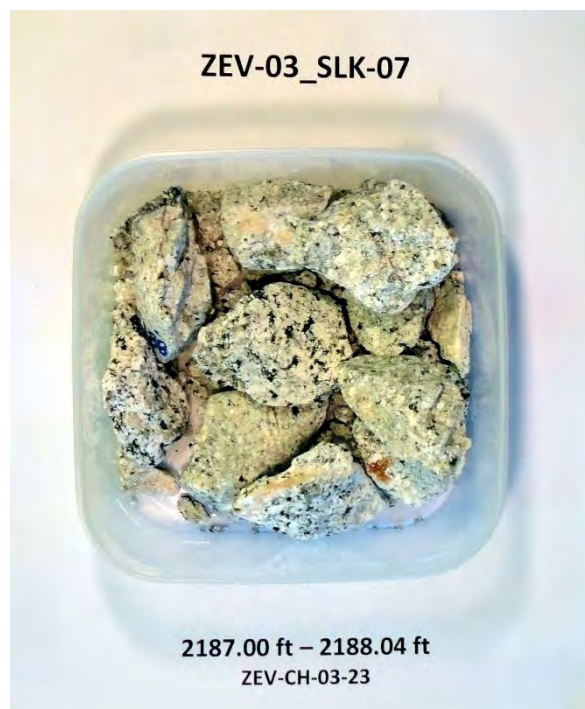


ZEV-03_SLK-06—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



ZEV-03_SLK-07—Before



ZEV-03_SLK-07—After



ZEV-03_SLK-08—Before

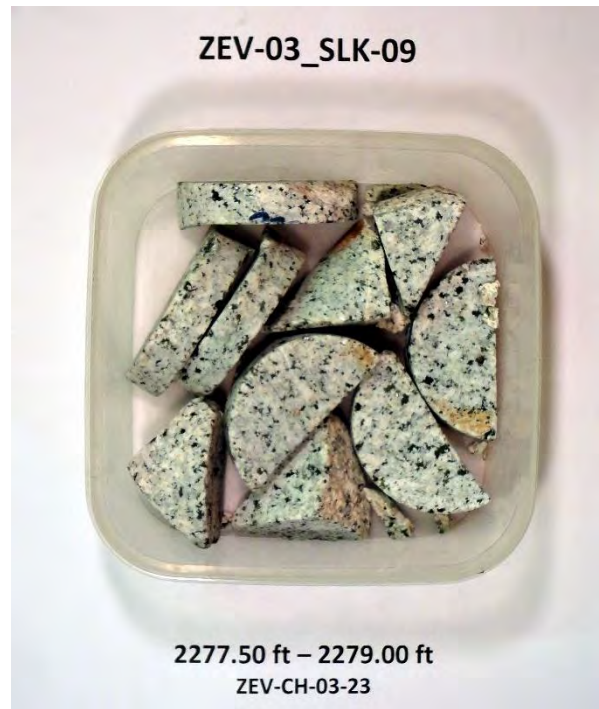


ZEV-03_SLK-08—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



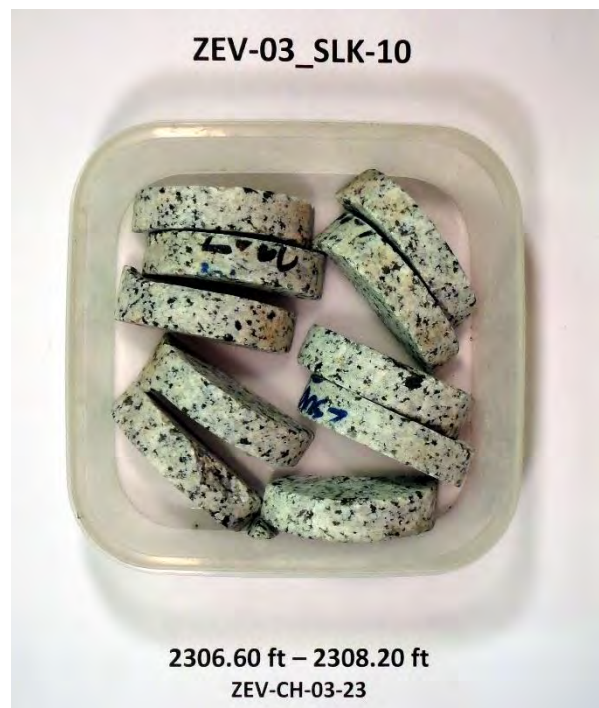
ZEV-03_SLK-09—Before



ZEV-03_SLK-09—After



ZEV-03_SLK-10—Before

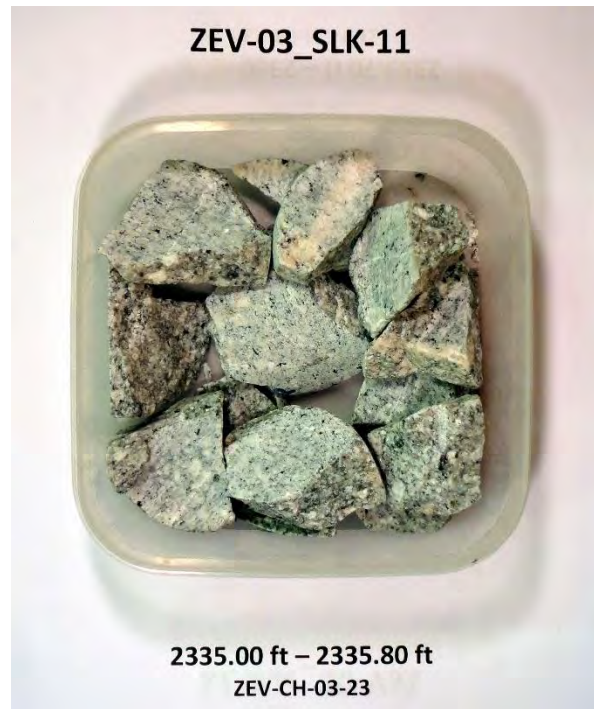


ZEV-03_SLK-10—After

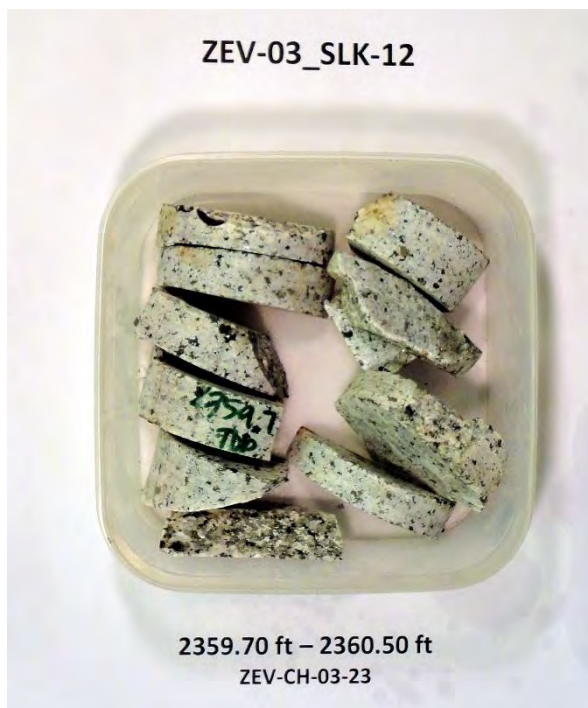
Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



ZEV-03_SLK-11—Before



ZEV-03_SLK-11—After



ZEV-03_SLK-12—Before

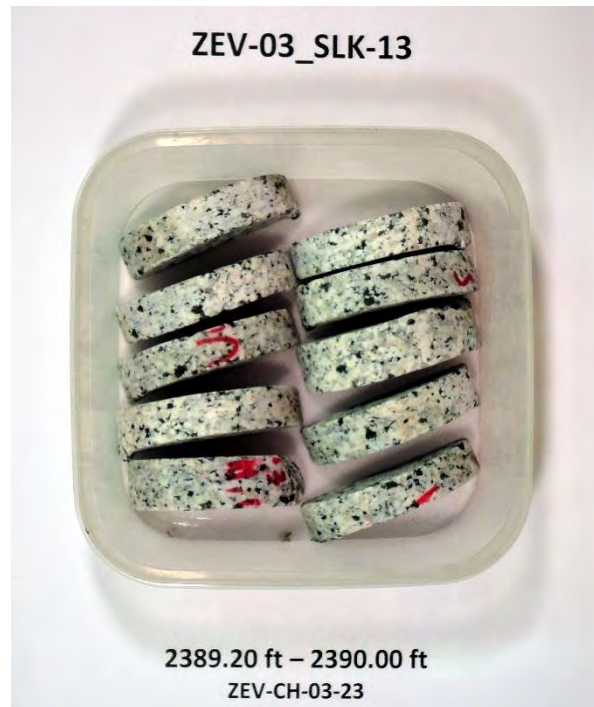


ZEV-03_SLK-12—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



ZEV-03_SLK-13—Before



ZEV-03_SLK-13—After



ZEV-03_SLK-14—Before

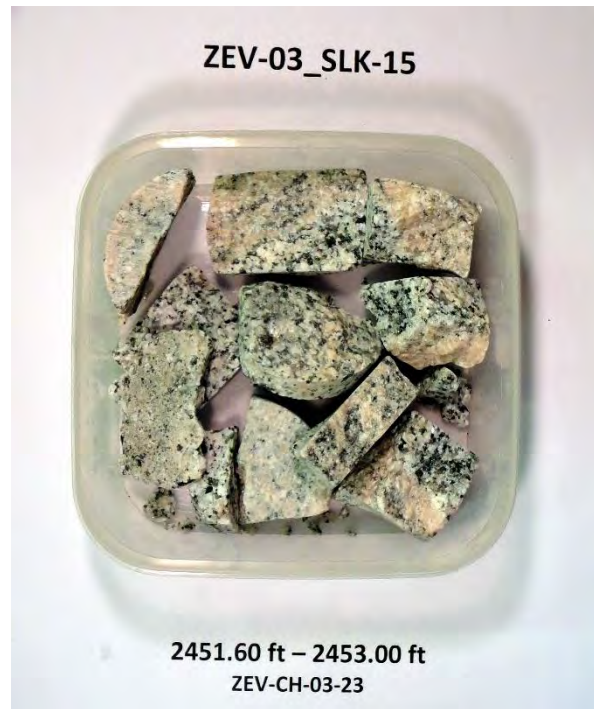


ZEV-03_SLK-14—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



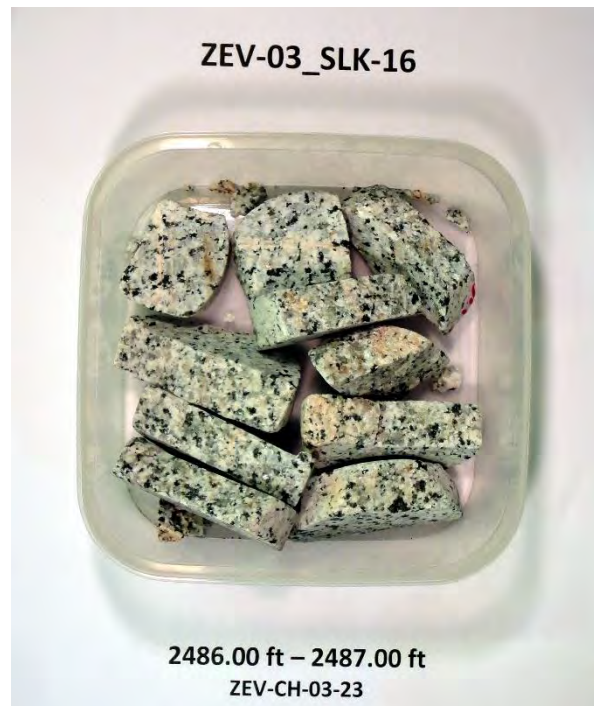
ZEV-03_SLK-15—Before



ZEV-03_SLK-15—After



ZEV-03_SLK-16—Before

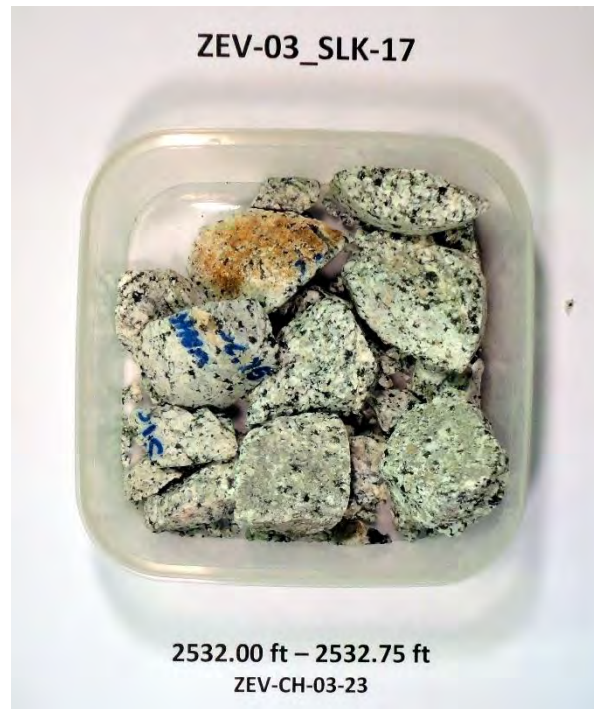


ZEV-03_SLK-16—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



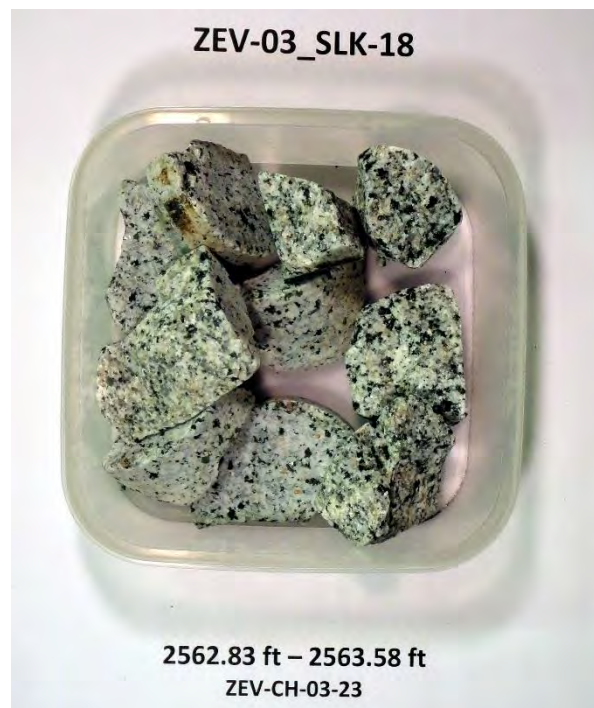
ZEV-03_SLK-17—Before



ZEV-03_SLK-17—After



ZEV-03_SLK-18—Before

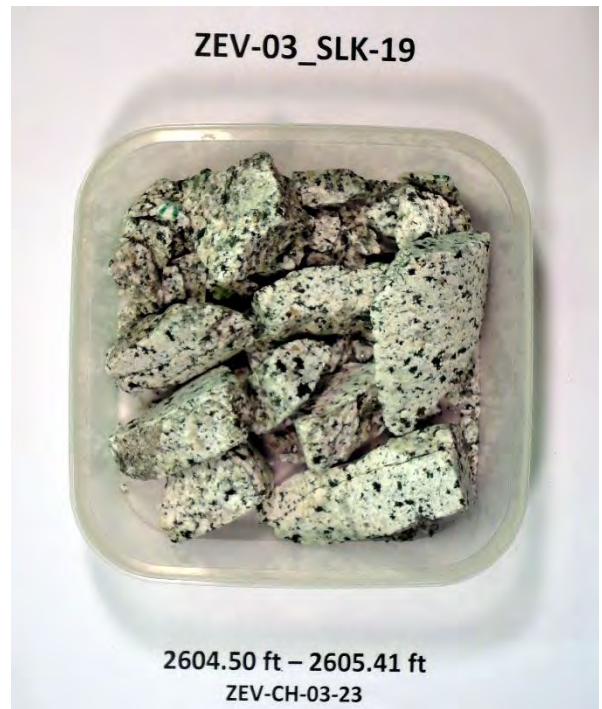


ZEV-03_SLK-18—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



ZEV-03_SLK-19—Before



ZEV-03_SLK-19—After



ZEV-03_SLK-20—Before

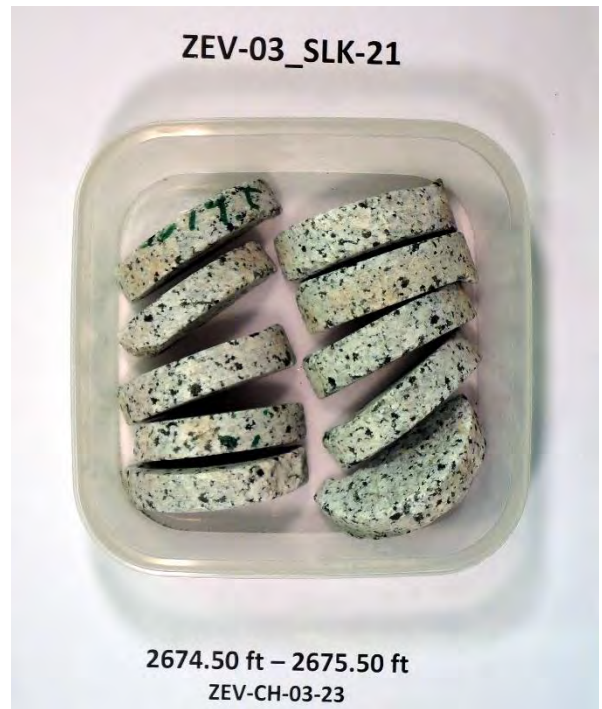


ZEV-03_SLK-20—After

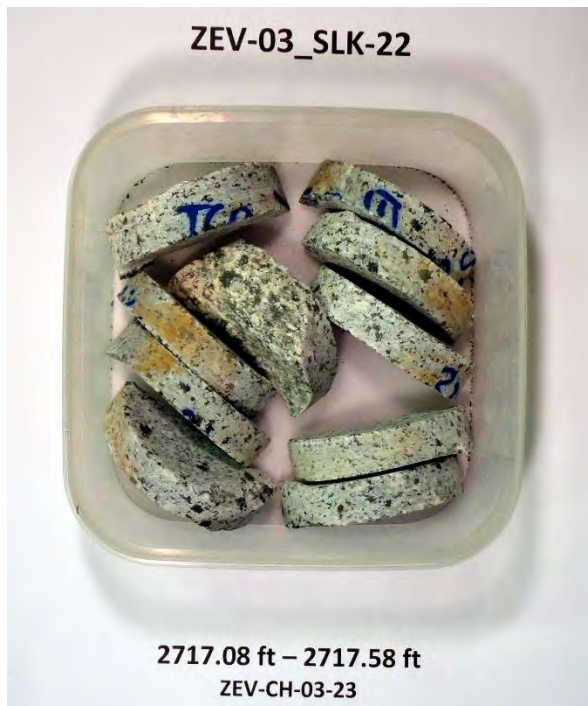
Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



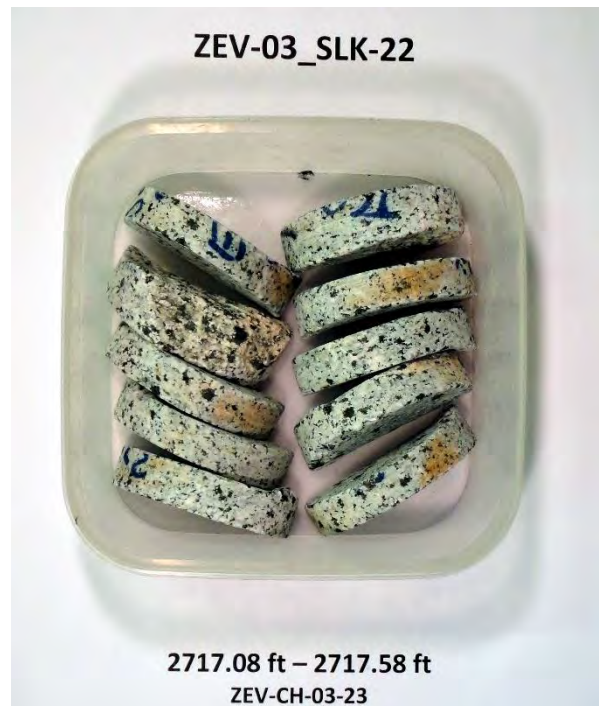
ZEV-03_SLK-21—Before



ZEV-03_SLK-21—After



ZEV-03_SLK-22—Before



ZEV-03_SLK-22—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



ZEV-03_SLK-23—Before



ZEV-03_SLK-23—After



ZEV-03_SLK-24—Before

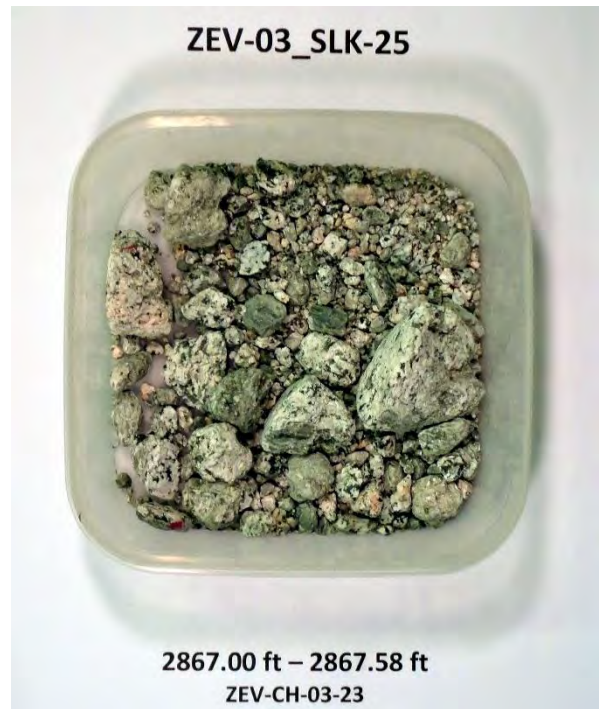


ZEV-03_SLK-24—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



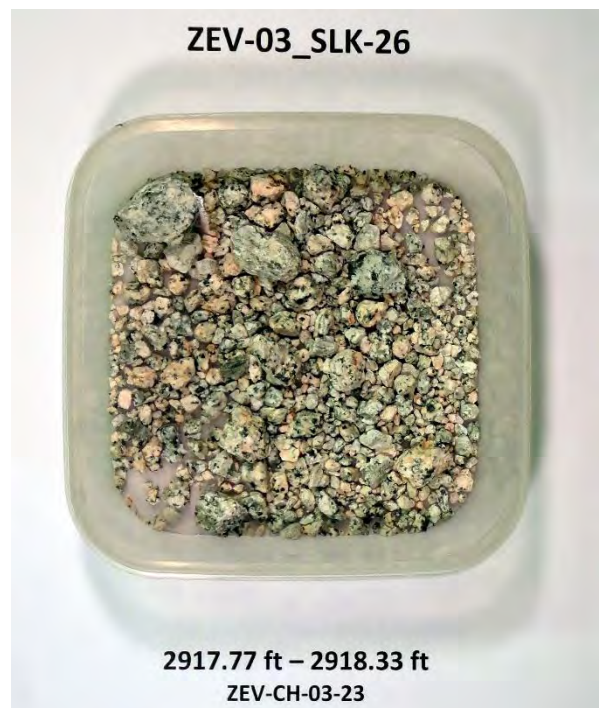
ZEV-03_SLK-25—Before



ZEV-03_SLK-25—After

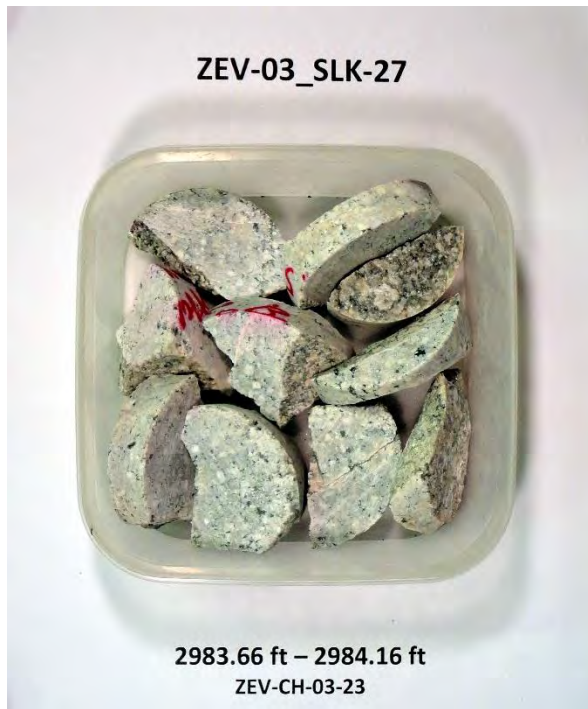


ZEV-03_SLK-26—Before

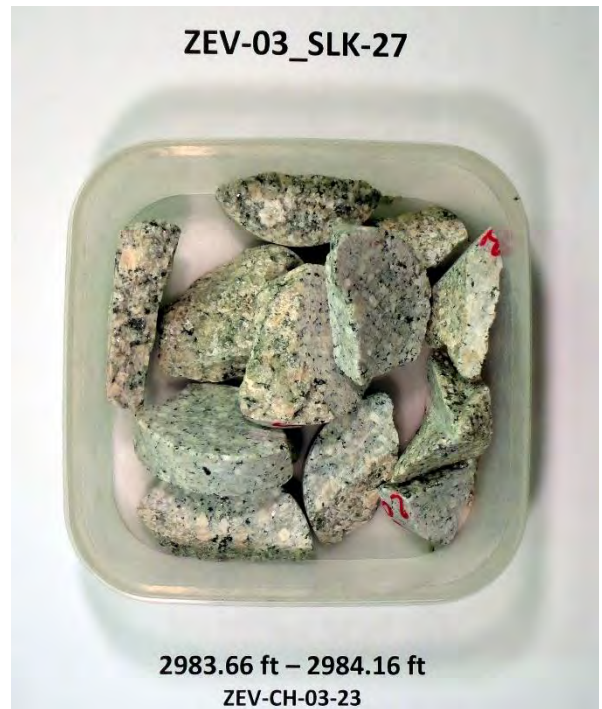


ZEV-03_SLK-26—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens *(continued)*



ZEV-03_SLK-27—Before



ZEV-03_SLK-27—After

Figure J-1. Before and After Photographs of Slake Durability Test Specimens (*concluded*)

APPENDIX D

ROCK MASS RATINGS

Depth		Elevation (MSL)		Interval Thickness (ft)	Lithology	Strength of Intact Rock (psi)	Estimated Strength of Intact Rock (psi)	Estimated Strength of Intact Rock (psi)	Rock Quality Designation (RQD)	Fracture Spacing (ft)	Number of Joint Sets	Condition of Discontinuities										Rock Mass Rating (RMR) Classification										Q-system										Rock Mass Rating (RMR _{Ho})	Geological Strength Index (GSI)		
From (ft)	To (ft)	From (ft)	To (ft)									Discontinuity Length (ft)	Aperture Thickness (mm)	Joint Roughness (J)	Roughness (RMR)	Joint Alteration (Q)	Infilling (RMR)	Weathering / Alteration	Joint Water (Q)	Groundwater Conditions	Strength of Intact Rock Rating	Strength of Intact Rock Rating	RQD Rating	Spacing of Discontinuities s Rating	Discontinuity Length Rating	Aperture Thickness Rating	Discontinuity Roughness Rating	Discontinuity Filling Rating	Weathering / Alteration Rating	Groundwater Rating	Rock Mass Rating (RMR)	RMR Classification	Joint Set Number (J _s)	Joint Roughness Number (J _r)	Joint Alteration Number (J _a)	Joint Water Reduction (J _w)	Estimated Vertical Stress (psi)	Stress Condition	Stress Reduction Factor (SRF)	Q-value	Q-value Classification				
1752	1762	708	708	10	Qz Monzonite	9,966			52	0.63	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	13	8	2	6	1	6	5	10	58	Fair Rock	6	1	1	1	2021	High Stress, Jointed Rock	4	2,167	Poor	63	58		
1762	1772	708	708	10	Qz Monzonite				R4	10,875	39	0.37	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	8	8	2	6	1	6	5	10	53	Fair Rock	6	1	1	1	2032	High Stress, Jointed Rock	4	1,425	Poor	58	53
1772	1782	708	778	10	Qz Monzonite	17,820				69	0.83	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Softening or low friction clay	Soft filling <5mm	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	12	0	13	10	2	6	1	2	5	10	61	Good Rock	6	1	4	1	2044	High Stress, Jointed Rock	2	1,438	Poor	66	61	
1782	1792	778	768	10	Qz Monzonite				R4	10,875	42	0.63	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	8	8	2	6	1	6	5	10	53	Fair Rock	6	1	1	1	2055	High Stress, Jointed Rock	4	1,798	Poor	58	53
1792	1802	768	758	10	Qz Monzonite				R4	10,875	56	0.40	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Tightly heaved, hard	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	13	8	2	6	1	6	5	10	58	Fair Rock	6	1	0.75	1	2067	High Stress, Jointed Rock	4	3,111	Poor	63	58
1802	1812	758	748	10	Qz Monzonite				R4	10,875	24	0.33	Two joint sets plus random joints	10	2	Smooth, planar	Smooth	Tightly heaved, hard	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	3	8	2	1	1	4	5	10	41	Fair Rock	6	1	1	1	2078	High Stress, Jointed Rock	4	1,333	Poor	46	41
1812	1822	748	738	10	Qz Monzonite				R4	10,875	74	0.38	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	13	8	2	6	1	6	5	10	58	Fair Rock	6	1	1	1	2090	High Stress, Jointed Rock	4	3,083	Poor	63	58
1822	1832	738	728	10	Qz Monzonite	10,296			10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	7	0	13	8	2	6	1	6	5	10	58	Fair Rock	6	1	1	1	2101	High Stress, Jointed Rock	4	2,542	Poor	63	58					
1832	1842	728	718	10	Diorite	12,474			R5	25,375	15	0.43	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	7	0	3	8	2	6	1	6	5	10	49	Fair Rock	6	1	1	1	2113	High Stress, Jointed Rock	4	8,625	Very Poor	54	49
1842	1852	718	708	10	Qz Monzonite				R4	10,875	29	0.20	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	12	8	8	2	6	1	6	5	10	58	Fair Rock	6	1	1	1	2124	Medium Stress	1	4,833	Fair	63	58
1852	1862	708	698	10	Qz Monzonite				R4	10,875	25	0.38	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	8	8	2	6	1	6	5	10	53	Fair Rock	6	1	1	1	2136	High Stress, Jointed Rock	4	1,042	Poor	58	53
1862	1872	698	688	10	Qz Monzonite	18,920			R4	10,875	50	0.45	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	12	0	13	8	2	6	1	6	5	10	63	Good Rock	6	1	1	1	2147	High Stress, Jointed Rock	2	4,167	Poor	68	63
1872	1882	688	678	10	Qz Monzonite				R4	10,875	34	0.25	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	8	8	2	6	1	6	5	10	53	Fair Rock	6	1	1	1	2159	High Stress, Jointed Rock	4	1,417	Poor	58	53
1882	1892	678	668	10	Qz Monzonite				R4	10,875	16	0.33	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	3	8	2	6	1	6	5	10	46	Fair Rock	6	1	1	1	2170	High Stress, Jointed Rock	6	6,667	Very Poor	53	46
1892	1902	668	658	10	Qz Monzonite				R3	4,550	10	0.56	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Slightly altered joint walls	None	Moderately altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	4	3	8	2	6	1	6	3	10	43	Fair Rock	6	1	2	1	2182	High Stress, Jointed Rock	14	8,068	Extremely Poor	48	43
1902	1912	658	648	10	Qz Monzonite				R3	4,550	41	0.59	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Unaltered	Medium inflow, occasional outwash of joint fillings (many drips "rain")	Wet	0	4	8	8	2	6	1	6	6	7	48	Fair Rock	6	1	1	0.66	2193	High Stress, Jointed Rock	14	8,322	Very Poor	56	51
1912	1922	648	638	10	Qz Monzonite				R4	10,875	51	0.33	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Slightly altered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	13	8	2	6	1	6	5	10	56	Fair Rock	6	1	2	1	2205	High Stress, Jointed Rock	4	1,063	Poor	61	56
1922	1932	638	628	10	Qz Monzonite				R4	10,875	71	0.77	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	13	10	2	6	1	6	5	10	60	Fair Rock	6	1	1	1	2216	High Stress, Jointed Rock	4	2,958	Poor	65	60
1932	1942	628	618	10	Qz Monzonite	17,783			R4	10,875	61	0.53	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Slightly altered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	12	0	13	8	2	6	1	6	5	10	61	Good Rock	6	1	1	1	2228	High Stress, Jointed Rock	2	2,542	Poor	66	61
1942	1952	618	608	10	Qz Monzonite				R4	10,875	56	0.56	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	13	8	2	6	1	6	5	10	56	Fair Rock	6	1	2	1	2239	High Stress, Jointed Rock	4	1,167	Poor	61	56
1952	1962	608	598	10	Qz Monzonite				R4	10,875	34	0.20	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Tightly heaved, hard	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	8	8	2	6	1	6	5	10	51	Fair Rock	6	1	0.75	1	2251	High Stress, Jointed Rock	4	1,889	Poor	56	51
1962	1972	598	588	10	Qz Monzonite	8,654			R4	10,875	11	0.22	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	7	0	3	8	2	6	1	6	6	10	49	Fair Rock	6	1	1	1	2262	High Stress, Jointed Rock	6	6,396	Very Poor	54	49
1972	1982	588	578	10	Qz Monzonite				R4	10,875	30	0.20	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	8	8	2	6	1	6	5	10	53	Fair Rock	6	1	1	1	2274	High Stress, Jointed Rock	4	1,258	Poor	58	53
1982	1992	578	568	10	Qz Monzonite				R4	10,875	52	0.67	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	13	10	2	6	1	6	5	10	60	Fair Rock	6	1	1	1	2285	High Stress, Jointed Rock	4	2,167	Poor	65	60
1992	2002	568	558	10	Qz Monzonite				R4	10,875	82	0.77	Three joint sets	10	Closed	Smooth, planar	Smooth	Tightly heaved, hard	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	17	10	2	6	1	6	5	10	62	Good Rock	9	1	0.75	1	2297	High Stress, Jointed Rock	4	3,037	Poor	67	62
2002	2012	558	548	10	Qz Monzonite	20,405			R4	10,875	80	0.83	Three joint sets	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	12	0	17	10	2	6	1	6	5	10	67	Good Rock	9	1	1	1	2308	High Stress, Jointed Rock	2	4,444	Fair	72	67
2012	2022	548	538	10	Qz Monzonite				R4	10,875	77	0.43	Three joint sets	10	Closed	Smooth, planar	Smooth	Tightly heaved, hard	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	17	8	2	6	1	6	5	10	60	Fair Rock	9	1	0.75	1	2320	High Stress, Jointed Rock	4	2,852	Poor	64	59
2022	2032	538	528	10	Qz Monzonite				R4	10,875	70	0.77	Three joint sets	10	Closed	Smooth, planar	Smooth	Slightly altered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	13	10	2	6	1	6	5	10	58	Fair Rock	9	1	2	1	2331	High Stress, Jointed Rock	4	6,972	Very Poor	63	58
2032	2042	528	518	10	Qz Monzonite	12,102			R4	10,875	100	2.00	Three joint sets	10	Closed	Smooth, planar	Smooth</																												

Depth (ft)	Elevation (MSL)		Interval Thickness (ft)	Lithology	Strength of Intact Rock (psi)	Estimated Strength of Intact Rock (psi)	Rock Quality Designation (RQR)	Fracture Spacing (ft)	Number of Joint Sets	Conditions of Discontinuities										Groundwater Conditions	Rock Mass Rating (RMR) Classification										Overview												
	From (ft)	To (ft)								Discontinuity Length (ft)	Aperture Thickness (mm)	Joint Roughness (Q)	Roughness (RMR)	Joint Alteration (Q)	Infilling (RMR)	Weathering / Alteration	Joint Water (Q)	Strength of Intact Rock Rating	Strength of Intact Rock Rating		RQD Rating	Spacing of Discontinuities in Rating	Discontinuity Length Rating	Aperture Thickness Rating	Discontinuity Roughness Rating	Discontinuity Filling Rating	Weathering / Alteration Rating	Groundwater Rating	Rock Mass Rating (RMR)	RMR Classification	Joint Set Number (J ₁)	Joint Roughness Number (J ₂)	Joint Alteration Number (J ₃)	Joint Water Reduction (J ₄)	Estimated Vertical Stress (psi)	Stress Condition	Stress Reduction Factor (SRF)	Q-value	Q-value Classification	Rock Mass Rating (RMR ₉₀)	Geological Strength Index (GSI)		
1750	760	820	810	10	Qtz Monzonite	24,560	72	1.25	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Slightly altered joint walls	Soft filling <5mm	Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	12	0	13	10	2	6	1	6	5	10	61	Good Rock	6	1	1	1	2018	Medium Stress	1	6,400	Fair	66	61		
1760	770	810	780	10	Qtz Monzonite		78	1.00	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	17	10	2	6	1	6	5	10	58	Fair Rock	6	1	1	1	2030	High Stress, Jointed Rock	4	3,250	Poor	49	58		
1770	780	800	790	10	Qtz Monzonite	14,036	69	0.71	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	7	0	13	10	2	6	1	6	5	10	60	Fair Rock	6	1	1	1	2041	High Stress, Jointed Rock	2	5,700	Fair	65	60		
1780	790	790	780	10	Qtz Monzonite		84	10.75	71	0.63	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	13	8	2	6	1	6	5	10	58	Fair Rock	6	1	1	1	2053	High Stress, Jointed Rock	4	2,950	Poor	63	58
1790	800	790	780	10	Diorite/Qtz Monzonite		70	0.29	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Tightly healed, hard	Hard filling <5mm	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	8	8	2	6	1	4	5	10	51	Fair Rock	6	1	0.75	1	2064	High Stress, Jointed Rock	4	2,000	Poor	56	51		
1800	810	770	760	10	Qtz Monzonite		84	10.75	30	0.45	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	8	8	2	6	1	6	5	10	54	Fair Rock	6	1	1	1	2076	High Stress, Jointed Rock	4	1,250	Poor	59	54
1810	820	760	750	10	Qtz Monzonite		84	10.75	30	0.37	Two joint sets plus random joints	10	Closed	Rough, irregular, planar	Slightly rough	Tightly healed, hard	None	Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	8	8	2	6	1	4	6	10	59	Fair Rock	6	1.5	0.75	1	2087	High Stress, Jointed Rock	4	5,417	Fair	64	59
1820	830	750	740	10	Qtz Monzonite		84	10.75	30	0.45	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Tightly healed, hard	Hard filling <5mm	Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	8	8	2	6	1	4	6	10	52	Fair Rock	6	1	0.75	1	2099	High Stress, Jointed Rock	4	1,667	Poor	57	52
1830	840	740	730	10	Qtz Monzonite		84	10.75	14	0.42	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Tightly healed, hard	Hard filling <5mm	Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	3	8	2	6	1	4	6	10	47	Fair Rock	6	1	0.75	1	2110	High Stress, Jointed Rock	4	0,778	Very Poor	62	47
1840	850	720	710	10	Qtz Monzonite		84	10.75	19	0.24	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	8	8	2	6	1	6	6	10	54	Fair Rock	6	1	1	1	2122	High Stress, Jointed Rock	4	1,875	Poor	59	54
1850	860	720	710	10	Qtz Monzonite	15,752	35	0.45	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	12	0	8	8	2	6	1	6	6	10	59	Fair Rock	6	1	1	1	2133	High Stress, Jointed Rock	2	2,917	Poor	64	59		
1860	870	710	700	10	Qtz Monzonite		84	10.75	27	0.33	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Decomposed	Dry excavations or minor inflow (humid or a few drips)	Damp	4	0	8	8	2	6	1	6	0	10	45	Fair Rock	6	1	1	1	2145	High Stress, Jointed Rock	12	0,375	Very Poor	50	45
1870	880	700	690	10	Qtz Monzonite	16,610	48	0.63	Two joint sets plus random joints	10	Closed	Smooth, undulating	Smooth	Unaltered joint walls	None	Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	12	0	8	8	2	6	1	6	6	10	59	Fair Rock	6	2	1	1	2156	High Stress, Jointed Rock	2	8,000	Fair	64	59		
1880	890	690	680	10	Qtz Monzonite	17,182	52	0.91	Two joint sets plus random joints	10	Closed	Sticksided, undulating	Sticksided	Slightly altered joint walls	Soft filling <5mm	Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	12	0	13	10	2	6	1	6	0	10	61	Good Rock	6	1.5	2	1	2168	High Stress, Jointed Rock	2	2,280	Fair	71	61		
1890	900	680	670	10	Qtz Monzonite		84	10.75	52	0.56	Two joint sets plus random joints	10	Closed	Smooth, undulating	Smooth	Slightly altered joint walls	Hard filling <5mm	Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	3	8	2	6	1	4	6	10	47	Fair Rock	6	2	2	2	2179	High Stress, Jointed Rock	4	0,792	Very Poor	52	47
1900	910	670	660	10	Qtz Monzonite		84	10.75	10	0.25	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	3	8	2	6	1	4	6	10	47	Fair Rock	6	1	1	1	2191	High Stress, Jointed Rock	4	0,280	Very Poor	52	47
1910	920	660	650	10	Qtz Monzonite		84	10.75	43	0.36	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Moderately altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	3	8	2	6	1	4	6	10	46	Fair Rock	6	1	1	1	2202	High Stress, Jointed Rock	4	0,417	Very Poor	55	46
1920	930	650	640	10	Qtz Monzonite		84	10.75	10	0.29	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Slightly altered joint walls	Hard filling <5mm	Moderately altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	3	8	2	6	1	4	3	10	44	Fair Rock	6	1	2	1	2214	High Stress, Jointed Rock	4	0,208	Very Poor	49	44
1930	940	640	630	10	Qtz Monzonite		84	10.75	10	0.32	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	3	8	2	6	1	6	6	10	49	Fair Rock	6	1	1	1	2225	High Stress, Jointed Rock	6	0,417	Very Poor	54	49
1940	950	630	620	10	Qtz Monzonite		84	10.75	42	0.30	Two joint sets plus random joints	10	Closed	Sticksided, undulating	Sticksided	Unaltered joint walls	None	Moderately altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	8	8	2	6	0	6	3	10	50	Fair Rock	6	1.5	1	1	2237	High Stress, Jointed Rock	4	2,688	Poor	55	50
1950	960	620	610	10	Qtz Monzonite	18,272	23	0.40	Two joint sets plus random joints	10	Closed	Rough or irregular, undulating	Slightly rough	Slightly altered joint walls	Hard filling <5mm	Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	12	0	3	8	2	6	3	4	6	10	24	Fair Rock	6	3	2	2	2248	High Stress, Jointed Rock	2	2,875	Poor	59	54		
1960	970	610	600	10	Qtz Monzonite		84	10.75	60	0.42	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	12	0	17	8	2	6	1	6	6	10	60	Good Rock	6	1	1	1	2260	High Stress, Jointed Rock	2	6,667	Fair	69	60
1970	980	600	590	10	Qtz Monzonite	14,146	55	1.11	Two joint sets plus random joints	10	Closed	Smooth, undulating	Smooth	Unaltered joint walls	None	Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	7	0	13	10	2	6	1	6	6	10	61	Good Rock	6	1	1	1	2271	High Stress, Jointed Rock	2	4,583	Fair	66	61		
1980	990	590	580	10	Qtz Monzonite	25,410	87	0.40	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	12	0	17	8	2	6	1	6	6	10	68	Good Rock	6	2	1	1	2283	High Stress, Jointed Rock	1	29,000	Good	73	68		
1990	1000	580	570	10	Qtz Monzonite		84	10.75	95	0.83	Three joint sets	10	Closed	Smooth, planar	Smooth	Slightly altered joint walls	Hard filling <5mm	Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	20	10	2	6	1	4	6	10	66	Good Rock	9	1	2	2	2294	High Stress, Jointed Rock	4	1,319	Poor	71	66
2000	1010	570	560	10	Qtz Monzonite	26,655	92	0.77	Three joint sets	10	Closed	Smooth, undulating	Smooth	Slightly altered joint walls	Hard filling <5mm	Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	12	0	20	10	2	6	1	4	6	10	71	Good Rock	9	2	2	2	2306	Medium Stress	1	10,222	Good	76	71		
2010	1020	560	550	10	Qtz Monzonite		84	10.75	50	0.50	Three joint sets	10	Closed	Rough or irregular, undulating	Slightly rough	Unaltered joint walls	None	Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	13	8	2	6	1	6	6	10	61	Good Rock	9	3	1	1	2317	High Stress, Jointed Rock	4	4,187	Fair	69	61
2020	1030	550	540	10	Qtz Monzonite		84	10.75	74	0.56	Three joint sets	10	Closed	Rough or irregular, undulating	Slightly rough	Unaltered joint walls	None	Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	13	8	2	6	3	6	6	10	61	Good Rock	9	3	1	1	2329	High Stress, Jointed Rock	4	6,167	Fair	66	61
2030	1040	540	530	10	Qtz Monzonite		84	10.75	55	0.34	Three joint sets	10	Closed	Smooth, undulating	Smooth	Slightly altered joint walls	Soft filling <5mm	Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	13	8	2	6	1	2	6	10	55	Fair Rock	9	2	2	2	2340	High Stress, Jointed Rock	4	1,528	Fair	60	55
2040	1050	530	520	10	Qtz Monzonite	27,231	100	0.83	Three joint sets	10	Closed	Smooth, planar	Smooth	Slightly altered joint walls	Soft filling <5mm	Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	12	0	20	15	2	6	1	2	6	10	69	Good Rock	9	1	2	1	2352	Medium Stress	1						

Depth		Elevation (MSL)		Interval Thickness (ft)	Lithology	Strength of Intact Rock (psi)	Estimated Strength of Intact Rock (psi)	Estimated Strength of Intact Rock (psi)	Rock Quality Designation (RQD)	Fracture Spacing (ft)	Number of Joint Sets	Condition of Discontinuities										Groundwater Conditions	Rock Mass Rating (RMR) Classification										Q-system													
From (ft)	To (ft)	From (ft)	To (ft)									Discontinuity Length (ft)	Aperture Thickness (mm)	Joint Roughness (Q)	Roughness (RMR)	Joint Alteration (Q)	Infilling (RMR)	Weathering / Alteration	Strength of Intact Rock Rating	Strength of Intact Rock Rating	RQD Rating		Spacing of Discontinuities Rating	Discontinuity Length Rating	Aperture Thickness Rating	Discontinuity Roughness Rating	Discontinuity Filling Rating	Weathering / Alteration Rating	Groundwater Rating	Rock Mass Rating (RMR)	RMR Classification	Joint Set Number (J ₁)	Joint Roughness Number (J ₂)	Joint Alteration Number (J ₃)	Joint Water Reduction (J ₄)	Estimated Vertical Stress (psi)	Stress Condition	Stress Reduction Factor (SRF)	Q-value	Q-value Classification	Rock Mass Rating (RMR ₉₀)	Geological Strength Index (GSI)				
1751	1761	819	809	10	Qtz Monzonite				4.50	16	0.40	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	4	3	8	2	6	1	6	5	10	45	Fair Rock	6	1	1	1	2019	High Stress, Jointed Rock	12	4,222	Very Poor	50	45		
1761	1771	799	789	10	Qtz Monzonite				10,875	R4	10,875	20	0.42	Two joint sets plus random joints	10	Closed	Rough or irregular, undulating	Slightly rough	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	4	3	8	2	6	1	6	5	10	50	Fair Rock	6	3	1	1	2031	High Stress, Jointed Rock	4	1,250	Poor	55	50
1771	1781	799	789	10	Qtz Monzonite	19,800													Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	12	0	3	8	2	6	3	6	5	10	55	Fair Rock	6	3	1	1	2042	High Stress, Jointed Rock	2	6,000	Fair	60	55		
1781	1791	789	779	10	Qtz Monzonite				R4	10,875	22	0.33	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Moderately altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	3	8	2	6	1	6	5	10	46	Fair Rock	6	1	1	1	2054	High Stress, Jointed Rock	4	4,917	Very Poor	51	46	
1791	1801	779	769	10	Qtz Monzonite				R4	10,875	20	0.33	Two joint sets plus random joints	10	Closed	Rough or irregular, undulating	Slightly rough	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	3	8	2	6	3	6	5	10	50	Fair Rock	6	1.5	1	1	2065	High Stress, Jointed Rock	4	2,125	Poor	55	50	
1801	1811	769	759	10	Qtz Monzonite				R4	10,875	12	0.33	Two joint sets plus random joints	10	Closed	Rough, irregular, planar	Slightly rough	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	3	8	2	6	3	6	5	10	50	Fair Rock	6	1.5	1	1	2077	High Stress, Jointed Rock	4	4,790	Very Poor	56	50	
1811	1821	759	749	10	Qtz Monzonite				R4	10,875	18	0.38	Two joint sets plus random joints	10	Closed	Rough or irregular, undulating	Slightly rough	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	3	8	2	6	3	6	5	10	50	Fair Rock	6	1.5	1	1	2088	High Stress, Jointed Rock	4	1,125	Poor	49	44	
1821	1831	749	739	10	Qtz Monzonite				R4	10,875	33	1.00	Two joint sets plus random joints	10	Closed	Rough, irregular, planar	Slightly rough	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	8	10	2	6	3	2	5	10	53	Fair Rock	6	1.5	2	1	2100	High Stress, Jointed Rock	4	1,831	Poor	58	53	
1831	1841	739	729	10	Qtz Monzonite	14,797													Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	12	0	8	10	2	6	3	6	5	10	62	Good Rock	6	1.5	1	1	2111	High Stress, Jointed Rock	2	2,875	Fair	67	62		
1841	1851	729	719	10	Qtz Monzonite				R4	10,875	29	0.63	Two joint sets plus random joints	10	Closed	Rough or irregular, undulating	Slightly rough	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	3	8	2	6	3	6	5	10	49	Fair Rock	6	3	1	1	2123	High Stress, Jointed Rock	4	2,875	Poor	56	51	
1851	1861	719	709	10	Qtz Monzonite	9,636													Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	7	0	3	8	2	6	3	6	5	10	47	Fair Rock	6	3	2	1	2134	High Stress, Jointed Rock	4	4,938	Very Poor	52	47		
1861	1871	709	699	10	Qtz Monzonite				R4	10,875	16	0.42	Two joint sets plus random joints	10	Closed	Rough or irregular, undulating	Slightly rough	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	3	8	2	6	1	6	5	10	54	Fair Rock	6	2	1	1	2146	High Stress, Jointed Rock	2	3,500	Poor	59	54	
1871	1881	699	689	10	Qtz Monzonite	79,360													Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	12	0	3	8	2	6	1	6	6	10	54	Fair Rock	6	1	1	1	2157	High Stress, Jointed Rock	2	1,167	Poor	59	54		
1881	1891	689	679	10	Discrete Qtz Monzonite				R4	10,875	36	0.59	Two joint sets plus random joints	10	Closed	Smooth, undulating	Smooth	Unaltered joint walls	None	Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	8	10	2	6	1	6	6	10	54	Fair Rock	6	2	1	1	2169	High Stress, Jointed Rock	4	3,000	Poor	59	54	
1891	1901	679	669	10	Discrete Qtz Monzonite				R4	10,875	30	0.48	Two joint sets plus random joints	10	Closed	Smooth, undulating	Smooth	Unaltered joint walls	None	Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	8	10	2	6	1	6	6	10	54	Fair Rock	6	2	1	1	2180	High Stress, Jointed Rock	4	4,833	Very Poor	54	49	
1901	1911	669	659	10	Qtz Monzonite														Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	7	0	13	8	2	6	3	6	5	10	61	Good Rock	6	3	1	1	2192	High Stress, Jointed Rock	6	4,667	Fair	66	61		
1911	1921	659	649	10	Qtz Monzonite				R4	10,875	30	0.48	Two joint sets plus random joints	10	Closed	Smooth, undulating	Smooth	Unaltered joint walls	None	Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	13	10	2	6	3	6	6	10	61	Good Rock	6	3	1	1	2203	High Stress, Jointed Rock	4	1,456	Poor	57	52	
1921	1931	649	639	10	Qtz Monzonite	32,098													Unaltered	Dry excavations or minor inflow (humid or a few drips)	Damp	12	0	8	8	2	6	3	6	6	10	61	Good Rock	6	3	1	1	2215	Medium Stress	1	15,000	Good	66	61		
1931	1941	639	629	10	Qtz Monzonite				R4	4,550	57	0.83	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Slightly altered joint walls	None	Highly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	4	13	10	2	6	1	4	1	10	51	Fair Rock	6	1	2	1	2226	High Stress, Jointed Rock	14	4,339	Very Poor	56	51	
1941	1951	629	619	10	Qtz Monzonite				R4	10,875	40	0.82	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	3	8	2	6	1	6	5	10	50	Fair Rock	6	4	1	1	2238	High Stress, Jointed Rock	4	1,667	Poor	55	50	
1951	1961	619	609	10	Qtz Monzonite				R4	10,875	10	0.48	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	3	8	2	6	1	6	5	10	48	Fair Rock	6	1	1	1	2249	High Stress, Jointed Rock	4	4,333	Very Poor	53	48	
1961	1971	609	599	10	Qtz Monzonite				R4	10,875	40	0.91	Two joint sets plus random joints	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	3	8	2	6	1	6	5	10	56	Fair Rock	6	3	1	1	2261	High Stress, Jointed Rock	4	4,617	Poor	54	49	
1971	1981	599	589	10	Qtz Monzonite				R4	10,875	90	1.67	Two joint sets plus random joints	10	Closed	Rough or irregular, undulating	Slightly rough	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	20	10	2	6	3	6	5	10	69	Good Rock	6	3	1	1	2272	High Stress, Jointed Rock	4	11,250	Good	74	69	
1981	1991	589	579	10	Qtz Monzonite				R4	10,875	67	0.83	Two joint sets plus random joints	10	Closed	Rough or irregular, undulating	Slightly rough	Unaltered joint walls	None	Moderately altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	13	10	2	6	3	6	3	10	60	Fair Rock	6	3	1	1	2284	High Stress, Jointed Rock	4	4,375	Fair	65	60	
1991	2001	579	569	10	Qtz Monzonite				R4	10,875	47	0.67	Two joint sets plus random joints	10	Closed	Rough or irregular, undulating	Slightly rough	Unaltered joint walls	None	Moderately altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	8	10	2	6	1	6	3	10	53	Fair Rock	9	1	1	1	2295	High Stress, Jointed Rock	4	1,286	Poor	58	53	
2001	2011	569	559	10	Qtz Monzonite				R4	10,875	10	0.29	Three joint sets	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	3	8	2	6	1	6	5	10	48	Fair Rock	9	1	1	1	2307	High Stress, Jointed Rock	4	4,278	Very Poor	53	48	
2011	2021	559	549	10	Qtz Monzonite	20,362													Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	12	0	8	8	2	6	3	6	5	10	60	Fair Rock	9	1.5	1	1	2318	High Stress, Jointed Rock	2	2,500	Poor	65	60		
2021	2031	549	539	10	Qtz Monzonite				R4	10,875	50	1.00	Three joint sets	10	Closed	Rough, irregular, planar	Slightly rough	Unaltered joint walls	None	Slightly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	7	13	10	2	6	1	6	3	10	58	Fair Rock	9	1	1	1	2330	High Stress, Jointed Rock	4	1,289	Poor	63	58	
2031	2041	539	529	10	Qtz Monzonite				R4	4,550	50	1.00	Three joint sets	10	Closed	Smooth, planar	Smooth	Unaltered joint walls	None	Highly altered	Dry excavations or minor inflow (humid or a few drips)	Damp	0	4	13	10	2	6	1	4	1	10	53	Fair Rock	9	1	1	1	2341	High Stress, Jointed Rock	14	4,397	Very Poor	58	53	
2041	2051	529	519	10	Qtz Monzonite																																									

APPENDIX E

IST2D STRESS MEASUREMENTS

DRAFT

**IN-SITU STRESS TESTING
CORE HOLE ZEV-CH-03-23**

WILLOW ROCK - DAWN ROAD PROJECT SITE, ROSAMOND CALIFORNIA

Prepared for:

Lane Power and Energy Solutions

February 12, 2024

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DRAFT**IN-SITU STRESS TESTING
CORE HOLE ZEV-CH-03-23**

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1 INTRODUCTION

In September 2023, Agapito Associates, Inc. (Agapito) performed downhole in-situ stress testing for Lane Power and Energy Solutions at the Willow Rock – Dawn Road Project site near Rosamond, California. The purpose of the work was to measure the in-situ stress field within the quartz monzonite rock at target cavern construction horizons. A total of four tests were attempted in the quartz monzonite. Two of the tests were unsuccessful, owing to one oversized pilot hole and one attempt, in which the IST tool did not set into the pilot hole correctly; two of the tests were successful and provided usable stress relief data.

Testing was conducted in core hole ZEV-CH-03-23 between the depths of 2,027.4 and 2,037.4 feet (ft). The overcore testing was conducted by Agapito in cooperation with Crux Subsurface, Inc. drilling personnel, who performed the core drilling for all the stress tests.

The measurements were obtained using the in-situ stress measurement tool (IST2D) and downhole technique developed by Sigra, Pty. (Sigra) of Brisbane, Australia. The IST technique measures the rock stresses in the plane perpendicular to the core hole, and because core holes are generally vertical, this technique provides the principal stresses in the horizontal plane at the testing depth.

Rock samples from the two successful overcores were tested in Agapito's laboratory in Grand Junction, Colorado, to determine the unconfined compressive strength (UCS), Young's modulus E (in the axial direction), and Poisson's ratio (ν). The horizontal stresses were determined based on the elastic properties and the measured changes in hole diameter resulting from stress relief during overcoring.

The effects of horizontal stress on structural stability have been identified as potentially important design factors at the project site. The orientation, magnitude, and ratio of the maximum and minimum components of the pre-mining stress field, along with the intact rock mass strength and jointing characteristics, determine the extent of the impacts of horizontal stress.

2 TESTING METHODOLOGY

The IST2D tool is a pilot hole overcoring device that automatically measures the deformation across the diameter of a 25 to 27 mm pilot hole at six locations during overcoring. It is used with the Boart Longyear HQ wireline coring system. The tool's orientation is determined by triaxial magnetometers and accelerometers which are read when the tool is locked in the pilot hole and the drill rods are withdrawn 6 meters (m) above the tool, so as to free it from the effects of magnetic field.

The overcore samples are tested to determine the core's mechanical properties relating to Young's Moduli and Poisson's Ratios as well as uniaxial compressive strength. The analysis is in terms of axisymmetric anisotropic elastic, though not necessarily linearly elastic behavior. The derivation of core properties is described by Gray, Zhao, and Liu (2018).

There is one solution for six diameter change measurements, six solutions for five diameter change measurements, fifteen solutions for four diameter change measurements, and twenty solutions for three diameter change measurements. Thus, there exists forty-two potential solutions. The best

solution is one that uses the most diameter change measurements, provided the traces of diameter changes are consistent throughout the overcore process.

The solution to the sets of equations describing the stress and deformation is arrived at by using a least squares procedure. A value of the RMS (Root Mean Square) error between the theoretical best fit and actual measurements is presented with each solution.

3 FIELD TESTING PROGRAM

Testing was conducted using a specially modified set of wireline coring components (setting tools) and the IST stress tool. The setting tools were brought to the site in conjunction with Agapito's field work and assembled immediately prior to testing. Testing was conducted by one Agapito employee with the assistance of the drillers and other project personnel.

The data measured by the IST and used for analysis consisted of the measurement time and voltages for the six measurement pins, three magnetometers, and three inclinometer accelerometers. The datalogger contained within the IST was configured to record the hole diameter at 4-second intervals and orientation data at 20-second intervals for each overcore test. The response of the stress tool measurement pins was calibrated prior to the field program to determine the (linear) relationship between voltage and diameter. The pin response was checked periodically to verify proper operation. The magnetometers and accelerometers determined the orientation of the tool with respect to Magnetic North and the tool inclination within the hole. Each overcore test was performed in the following sequence, as shown in Figure 1.

1. After coring proceeded to a pre-determined depth (Figure 1 [a]), the core was pulled and a special counterbore assembly was lowered down the hole by wireline (Figure 1 [b]). The counterbore bit grinds any core stump left at the bottom of the hole and makes a counterbore to center the 25-mm-diameter pilot hole within the HQ (100-mm) bore. The counterbore bit is locked into the outer barrel such that thrust is transmitted to the bit, and an adapter, installed in the drill string, permits the counterbore bit to rotate with the rods.
2. Once the counterbore was drilled, the counterbore assembly was removed from the hole by wireline, and the pilot hole assembly (thruster) was lowered down the hole (Figure 1 [c]). Thrust on the pilot hole bit is provided by a water-pressure-actuated piston within the assembly, and rotation is transmitted to the assembly through the special adapter installed in Step 1. Thus, the rods rotate during drilling of the pilot hole, but do not provide thrust. Completion of the 500-mm-deep pilot hole is accompanied by a drop in water pressure as the piston advances past water-bypass ports at the end of its stroke.
3. Upon completion of the pilot hole, the hole was flushed for several minutes to clear cuttings. The thruster assembly was then removed, and the IST tool was carefully lowered down the hole by wireline (Figure 1 [d]). The IST was guided into the pilot hole by the shape of the counterbore, and wedges on the end of the IST secured it within the pilot hole as the downward load of hydraulic pressure acted on the IST. The launching tool was then pulled out of the hole, and the inner core barrel lowered down the hole by wireline.
4. After the inner core barrel latched, the rods were pulled up 20 ft so that magnetometers within the IST could stabilize without magnetic interference from the metallic rods

(Figure 1 [e]). The rods were held in this position for a minimum of 5 minutes to ensure that a satisfactory data sample was recorded.

5. With the inner barrel latched in and directional orientation assured, overcoring of the IST proceeded (Figures 1 [f] and 1 [g]). Rotation and feed rate were somewhat lower than in normal coring operations to minimize vibration as the IST was overcored.
6. At the completion of the overcore run, the inner barrel was pulled out of the hole by wireline, and with it, the core and IST. The core was extracted from the inner barrel into a core tray, and the core around the IST pins was photographed. Finally, the IST was removed from the core and the battery powering the IST was removed. The IST was then connected to a laptop computer and the pin, magnetometer, and accelerometer data were downloaded and reviewed (Figure 1 [h]). Depending on the expected competence of the strata in the next core run, the process was immediately repeated, or further coring was undertaken to locate a suitable test horizon.

Stress testing in core hole ZEV-CH-03-23 began on September 4, 2023, at a depth of 2027.4 ft. A total of four tests were attempted in the core hole to a depth of 2037.0 ft. A summary of the overcoring test attempts is given in Table 1. Two tests were successful: overcores 0052-70 and 0054-70. When possible, the recovered cores for each test were photographed to document rock type and condition, particularly cores that contained fractures which may have influenced the overcore data. Photographs of the recovered core for the two successful tests, including the relative location of the measurement pins are shown in Figures 2 and 3.

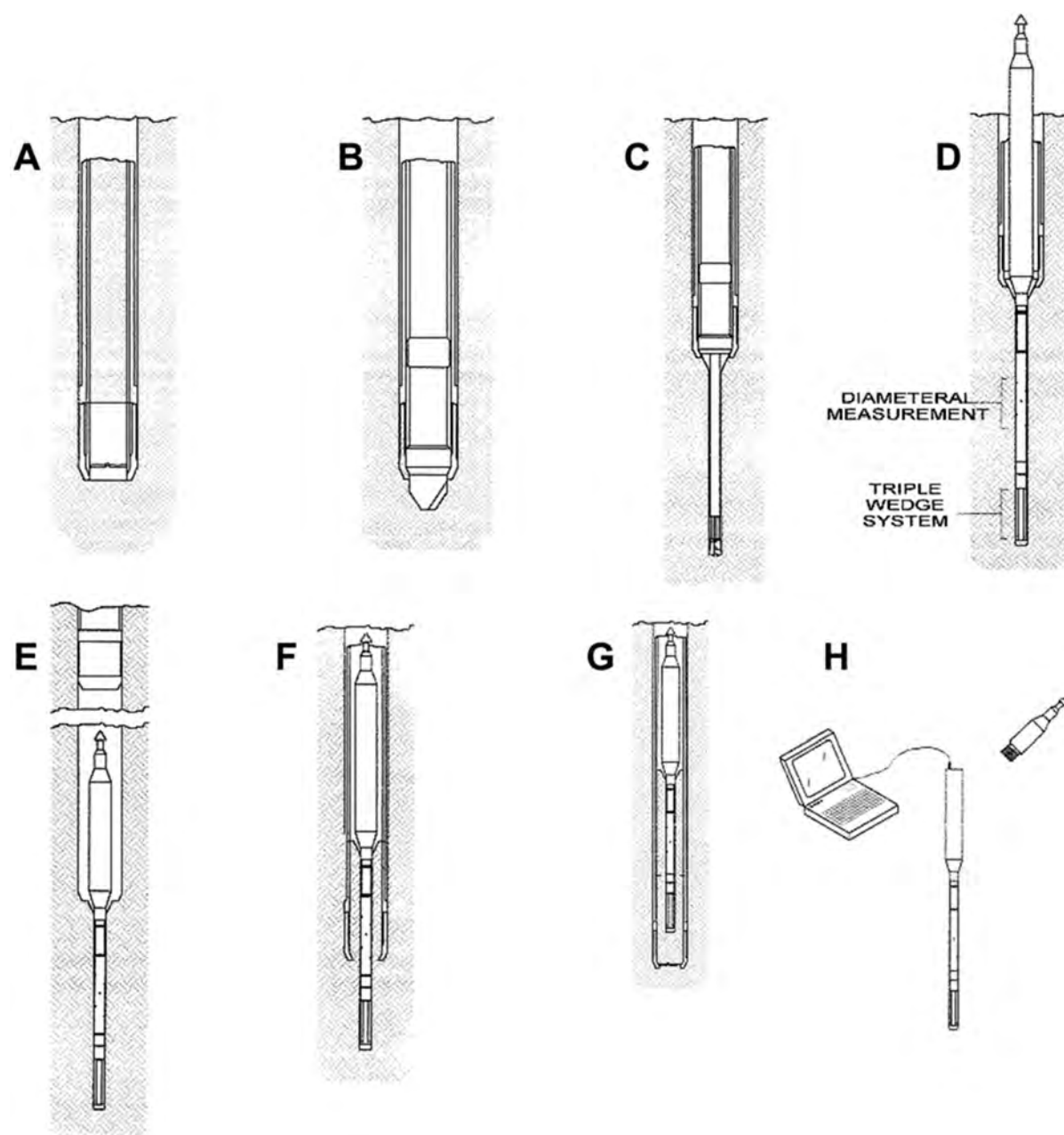


Figure 1. IST Field Stress Testing Sequence

Table 1. Summary of IST Test Attempts

Test Number	Overcore Number	Test Date	Successful Unsuccessful	Rock Condition	Depth (ft)
1	0036-076	9/5/2023	Unsuccessful	Solid	2027.4
2	0052-070	9/5/2023	Successful	Solid	2030.0
3	0054-070	9/5/2023	Successful	Solid	2033.4
4	0056-070	9/6/2023	Unsuccessful	Solid	2037.0



Figure 2. Photograph of Recovered Core from Overcore 0052-070

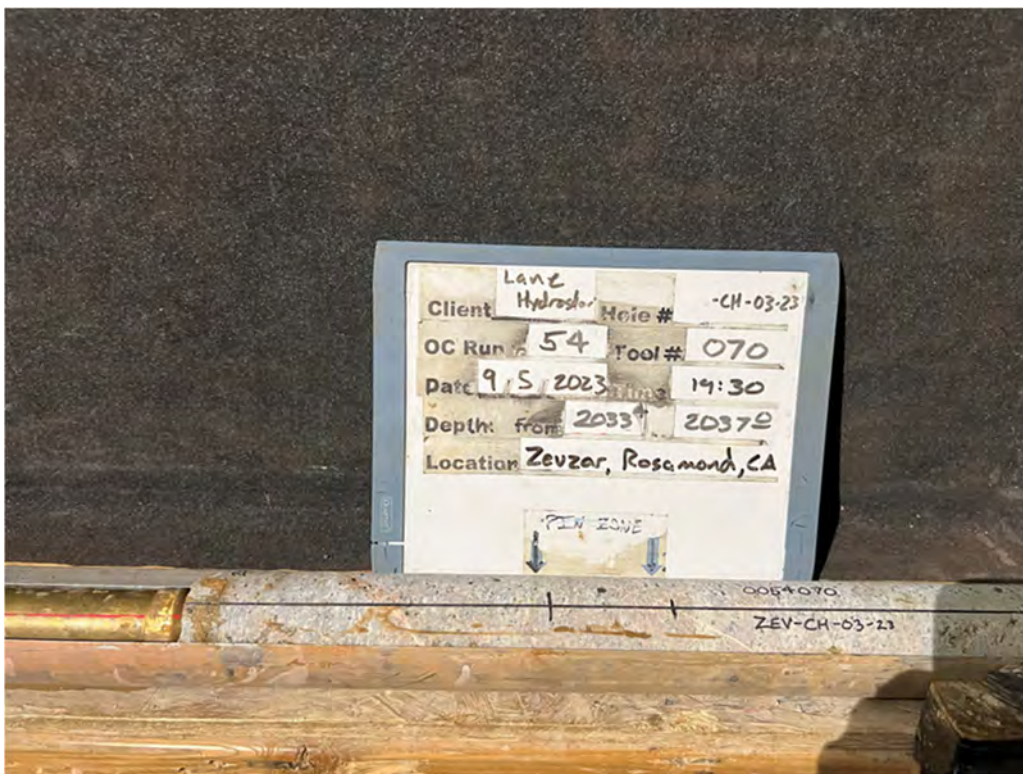


Figure 3. Photograph of Recovered Core from Overcore 0054-070

4 PHYSICAL PROPERTY TESTING

4.1 Physical Property Testing

Rock samples from both successful overcores were tested in Agapito's laboratory in Grand Junction, Colorado, to determine unconfined compressive strength (UCS), Young's modulus, and Poisson's ratio (in the axial direction).

4.2 Axial Testing

A summary of the laboratory test results is included in the Appendix following this report. The Appendix also contains the stress-strain plots for each test. Tests Run-0052-070 and Run-0054-070 correspond to overcores 0052-070 and 0054-070, respectively. The calculated values of Young's modulus (E) were 10.38×10^6 psi for overcore 0052-070, and 9.31×10^6 psi for overcore 0054-070. The Poisson's ratio was 0.32 for overcore 0052-070 and 0.19 for overcore 0054-070. The UCS was 26,468 psi for overcore 0052-070 and 12.696 psi for overcore 0054-070.

4.3 Radial Testing

Ideally, the value of E should be determined in the plane (generally horizontal) of the measurement direction, perpendicular to the borehole axis. However, because of the core size and geometry, samples used for conventional compression tests in the laboratory must be loaded along the borehole (vertical) axis. Additionally, the granitic rock being tested is presumed to be isotropic, with similar elastic properties in directions parallel and perpendicular to the plane of testing. Therefore, it was determined that radial testing was unnecessary for this project.

5 CALCULATION OF HORIZONTAL STRESSES

Plots of the pin data versus time were examined to determine the change in diameter that occurred during overcoring. In general, the diameter decreases slightly just before overcoring due to a stress concentration ahead of the core bit, then increases significantly and stabilizes as the bit relieves the horizontal stress acting on the core. The diametral change is calculated as the difference between the diameter measured just before the stress concentration decreases and the stable diameter just after the stress relief increases. Because the pins are offset, the overcore responses for the six pins occur in sequence at different times, depending upon the bit advance speed. Most of the pin traces for the successful tests showed normal response during the approximately 5-minute interval, while the core bit drilled past the pins, and could be used to calculate the stresses. The data for some pins were inconsistent and were excluded from the calculations.

Periodically, dummy runs were conducted on the surface to ensure that the tool, communication box, and laptop computer were all functioning properly. During dummy runs, data was collected while an aluminum standard cylinder was fitted over the pin zone. The data was downloaded and reviewed to confirm each pin was recording the correct diameter. In this manner, typical problems such as sticking pins and poor battery connections could be identified and corrected before lowering the IST tool into the pilot hole and attempting the actual overcore test.

The elastic properties and changes in the pilot hole diameter during overcoring are used to calculate the rock stresses in the plane (horizontal) normal to the borehole axis. The stresses can be

determined using data from a minimum of three pins; however, the additional data obtained using all six pins provides a measure of redundancy and permits the evaluation of the relative accuracy of the measurement. The stress calculation software, provided by Sigra, evaluates each combination of three or more pins and calculates an error value, or goodness-of-fit measure, for those combinations with four or more pins. An exact solution can be determined for any three-pin combination; however, a least-squares curve-fit technique is required to solve the combinations involving four or more pins.

The analysis software determines values for the mean effective stress (σ_m) and the deviatoric stress (σ_d). The major (σ_1) and minor (σ_2) principal stresses in the horizontal plane are calculated as follows:

$$\sigma_1 = \sigma_m + \sigma_d \quad (\text{Eqn. 1})$$

$$\sigma_2 = \sigma_m - \sigma_d \quad (\text{Eqn. 2})$$

For gravitational loading, the vertical stress (σ_v) is based on the density of the rock and the depth of the test. The stress gradient of the overburden material was calculated assuming a specific gravity of 2.69, or 1.17 psi-per-foot of depth (psi/ft). Assuming elasticity and that the rock is laterally constrained so there is no allowable strain in the horizontal plane, the lithostatic stress, or horizontal stress due to self-loading (σ_{hsw}) caused by the Poisson's effect, is:

$$\sigma_{hsw} = \sigma_v \times \frac{\nu}{(1-\nu)} \quad (\text{Eqn. 3})$$

Recent research (Mark and Gadde 2008) into the expected levels of horizontal stress worldwide has shown the classic assumption of passive pressure (σ_{hsw}) may not be applicable and horizontal stresses are typically much higher. It was found that horizontal stresses varied linearly with depth and rock stiffness, and a constant excess stress was present, regardless of depth.

Figures 4 and 5 show plots of the best-fit stress calculations for each successful test (overcore runs 0052-070 and 0054-070). The data is considered valid and provides clear indications of stress relief and accurate relief displacements. The least-squares curve fits are generally good, and the stress orientations are consistent. Overcore 0052-070 was slightly biaxial, though overcore 0054-070 showed minimum and maximum horizontal stresses with similar magnitudes. This low biaxiality in the two overcore results means further testing may be required to determine a more accurate bearing of maximum horizontal stress at depth at the project site. The orientation of the instrument and the calculated principal stresses relative to Magnetic North are determined using the magnetometer and inclinometer data recorded by the IST. The data obtained during the measurement periods when the rods were raised were stable and are considered valid.

Using the axial modulus values derived from the laboratory tests, the average maximum horizontal stress (σ_1) is 1.2 times the minimum horizontal stress (σ_3) and 1.62 times the estimated vertical stress (σ_v). Thus, the ratio between the major maximum horizontal stress and vertical stress indicates a reasonably high horizontal stress regime. The successful tests were located at similar depths, so it was not possible to predict the variation of stress with depth. However, since the increase in vertical stress is typically linear, vertical stress levels should be expected to increase with depth. The horizontal stresses are presumed to be caused by the tectonic stresses related to the San Andreas Fault system and, therefore, not a function of the depth.

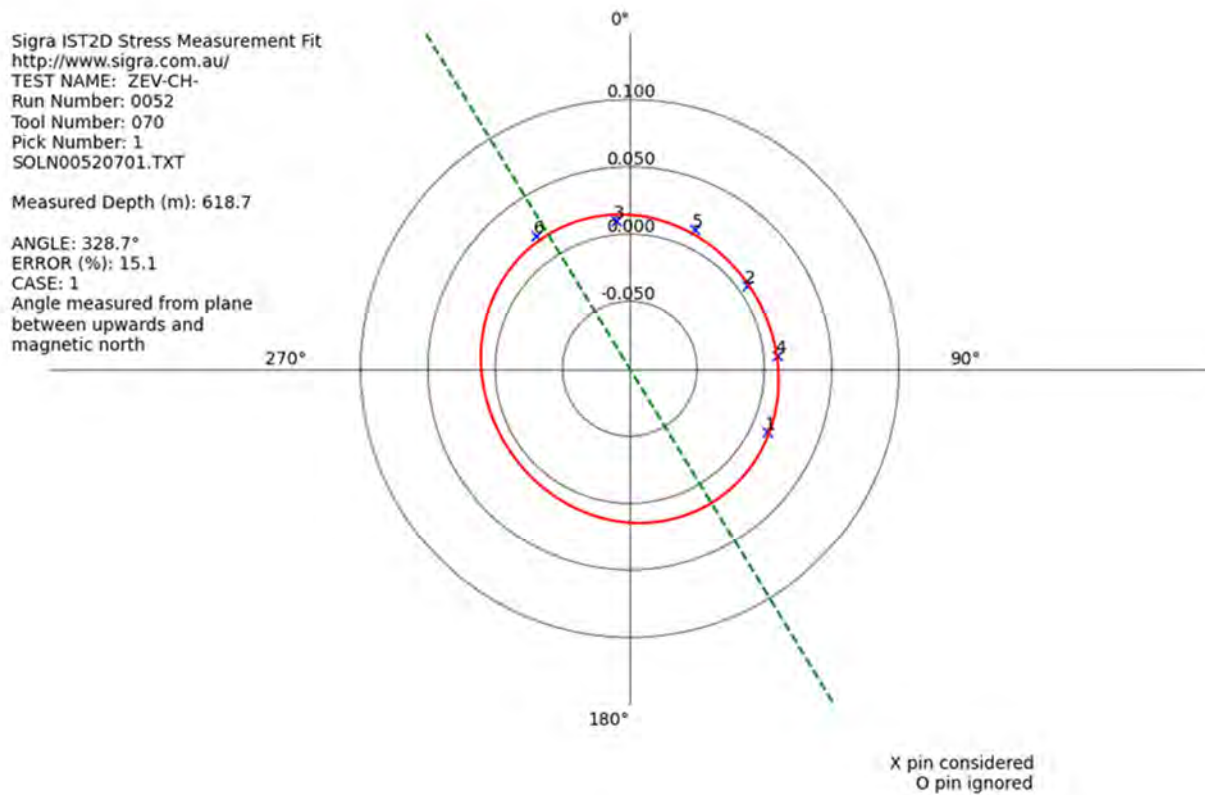


Figure 4. Best-Fit Stress Calculation for Overcore 0052-070

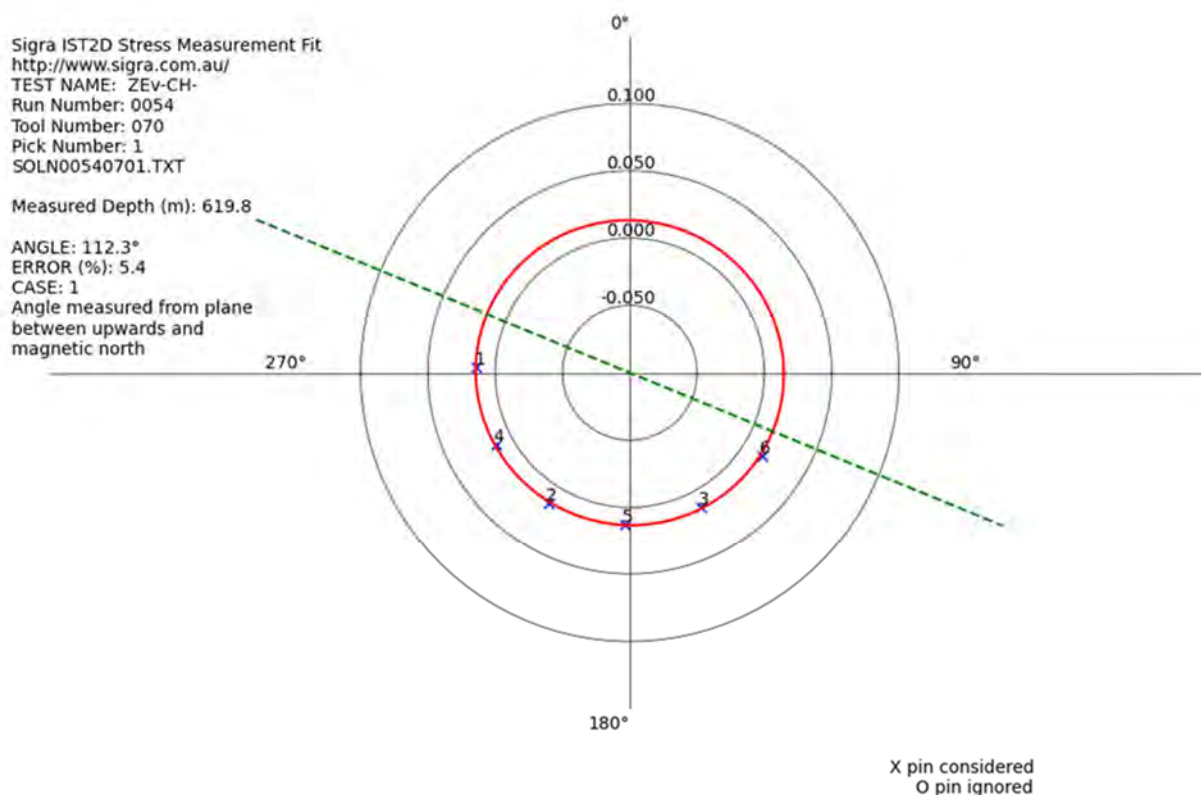


Figure 5. Best-Fit Stress Calculation for Overcore 0054-070

Table 2. Calculated Principal Stresses in ZEV-CH-03-23

Hole Name	ZEV-CH-03-23	
Overcore Number	0052-070	0054-070
Date Tested	9/5/2022	9/5/2022
Depth (ft)	2,030.0	2,033.4
Unconfined Compressive Strength (psi)	26,468	12,696
Young's Modulus ($\times 10^6$ psi)†	10.38	9.31
Poisson's Ratio †	0.32	0.19
Estimated Vertical Stress (psi)	2,368	2,373
Self-loading Horizontal Stress σ_{hsw} (psi)	1,114	557
Maximum Horizontal Stress σ_1 (psi)	4,098.8	3,611
Minimum Horizontal Stress σ_3 (psi)	3,115.4	3,446
$\sigma_1:\sigma_3$ Ratio	1.3	1.0
True Bearing of Maximum Horizontal Stress	340.38°	303.98°
Maximum Horizontal Strain ϵ_1 (microstrain)	226.8	269.8
Minimum Horizontal Strain ϵ_2 (microstrain)	101.6	248.6
psi = pounds per square inch; ° = degree; E = East		
† Tangent Calculation Method		
magnetic declination of 11.68° E		

6 DISCUSSION AND CONCLUSIONS

The testing program resulted in two successful tests out of four attempts. The first unsuccessful attempt was caused by an unforeseen incompatibility between the rock and the type of drill bit used to drill the pilot hole for the tool. This caused the pilot hole diameter to be oversized and outside of the measurable range of the tool for run 0036-076. The cause of the second failed attempt was unclear. Upon retrieval of the core run containing the deployed tool for test 0056-070, the tool was set approximately 4 inches into the pilot hole instead of the ~16 inches required for a proper deployment. The tool's pins were not placed inside the pilot hole in the rock and, therefore, recorded no usable data. The successful overcores generally produced consistent results with respect to stress levels and orientation. The variability between stress magnitudes is attributed to the typical variations associated with geological materials and environments.

The calculated magnitudes and orientation of the stresses are considered reasonable and indicate the tectonic strain of the San Andreas Fault system influences the in-situ horizontal stress of the project area.

The average maximum horizontal stress was 3,855.1 psi and the average minimum stress was 3,280.1 psi. Of the two successful tests run, 0052-070 provided an azimuth of the maximum horizontal stress of 340.4° (clockwise from North). Run 0054-070 provided maximum and minimum horizontal stresses of similar magnitude, causing lower confidence in the azimuthal data for run 0054-070.

7 REFERENCES

- Bickel, D. L. (1993), "Rock Stress Determinations from Overcoring—An Overview," U.S. Bureau of Mines Bulletin 694, 146 pp.
- Gray, I. (2000), "The Measurement and Interpretation of Stress," *Bowen Basin Symposium on Seam Gas*, pp. 321–324.
- Mark, C. and M. Gadde (2008), "Global Trends in Coal Mine Horizontal Stress Measurements," *Proceedings of the 27th International Conference on Ground Control in Mining*, Morgantown, WV, July 29–31, pp. 319–331.

APPENDIX A
LABORATORY COMPRESSION TESTING

Table A-1. Uniaxial Compressive Strength Test Results

Specimen No.	Hole ID	Depth		Rock Type	Weight (oz)	Average Diameter (inch)	Axial Length (inch)	Density (pcf)	Failure Load (lb)	UCS (psi)	Young's Modulus [†] (×10 ⁶ psi)	Poisson's Ratio [†]	Young's Modulus [‡] (×10 ⁶ psi)	Poisson's Ratio [‡]
		From (ft)	To (ft)											
Run-0052-070	ZEV-CH-03-23	2026.30	2026.75	Granite - IST Test at 2030.0 ft	37.10	2.39	5.29	168	118,940	26,468	10.38	0.32	10.64	0.17
Run-0054-070	ZEV-CH-03-23	2036.30	2036.75	Granite - IST Test at 2033.4 ft	38.37	2.40	5.49	168	57,195	12,696	9.31	0.19	8.94	0.05
[†] Tangent calculation method. [‡] Secant calculation method.														



Run-0052-070—Before



Run-0052-070—After



Run-0054-070—Before



Run-0054-070—After

Figure A-1. Before and After Photographs of Uniaxial Compressive Strength Test Specimens

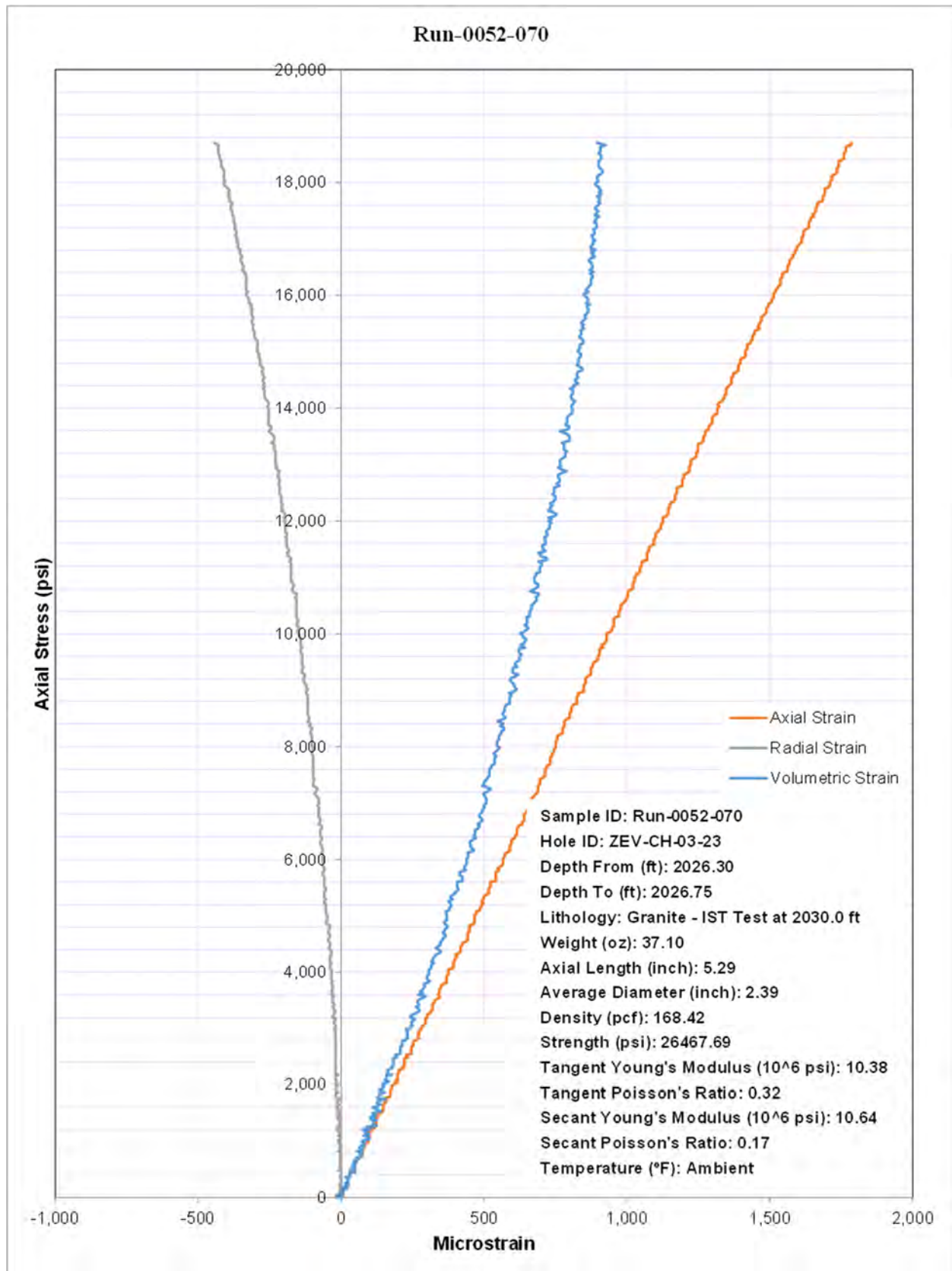


Figure A-2. Stress-Strain Plots of Uniaxial Compressive Strength Test Specimens

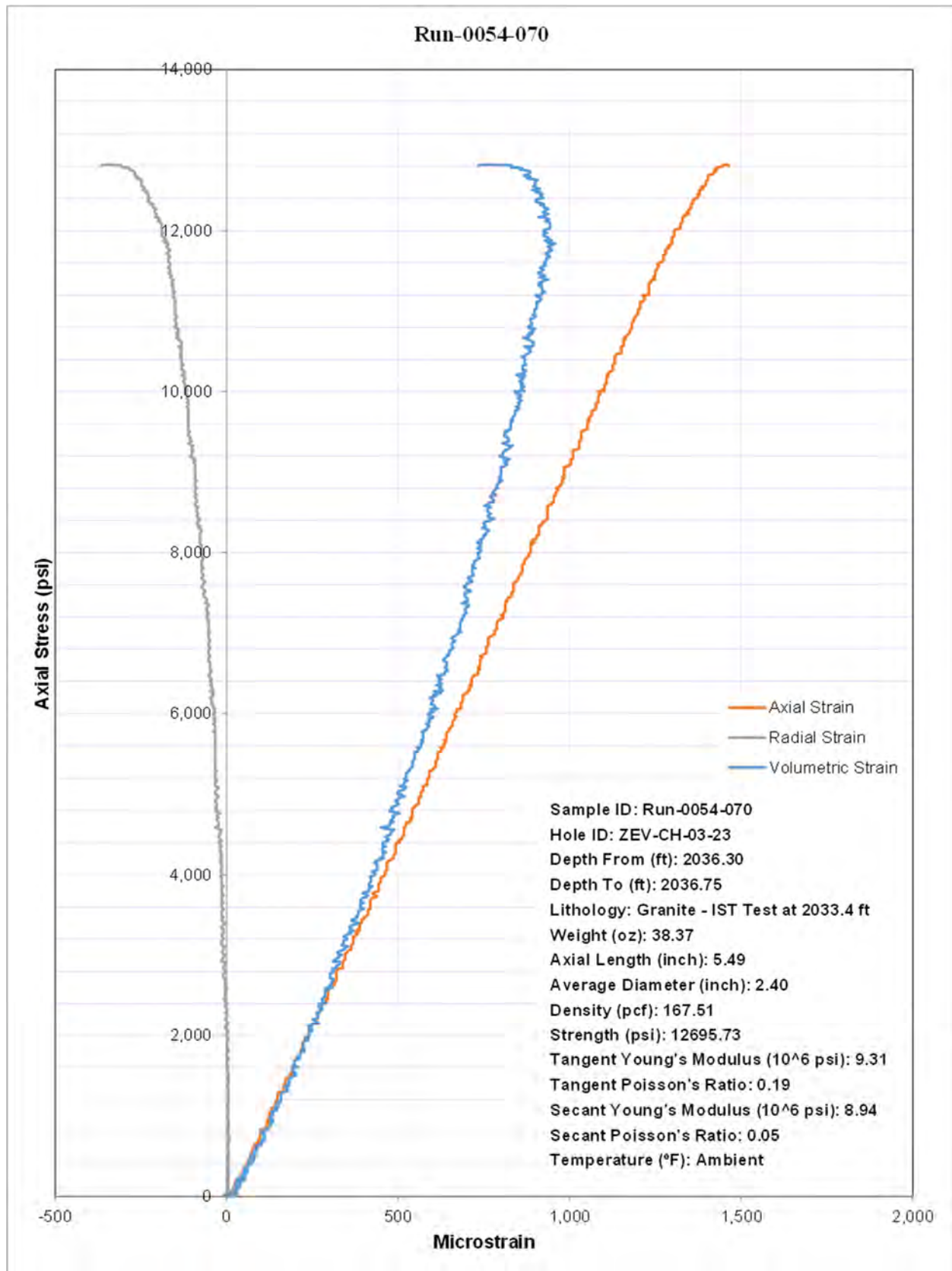


Figure A-2. Stress-Strain Plots of Uniaxial Compressive Strength Test Specimens